

SHEET 1 OF 19 SHEETS BRIDGE NO. N-18-002 (7YC)

# GENERAL NOTES

### **DESIGN:**

THE PROPOSED SUPERSTRUCTURE WAS DESIGNED IN ACCORDANCE WITH THE 2020 AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION, FOR HL-93 LOADING.

#### SURVEY BENCH MARK:

HYDRANT X CUT BOLT OVER MAIN OUTLET (1.5' A.G.)
N: 3033399.3746
E: 763539.3053
ELEVATION = 74.80'

ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

# DATE:

TO BE PLACED ON THE INSIDE FACE OF THE SOUTHEAST AND NORTHWEST HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

### SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS (A3).

#### UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE PROPOSED ENDPOSTS, AS DIRECTED BY THE ENGINEER.

### CONCRETE MIXES:

DECK, APPROACH SLAB, APPROACH SLAB SHELF, AND ABUTMENT CAP SHALL BE 4000 PSI,  $\frac{3}{4}$  In., 585 HP CEMENT CONCRETE

CT-TL2 BRIDGE RAIL AND TERMINUS SHALL BE 5000 PSI, 3 IN., 710 HP CEMENT CONCRETE

THE MINIMUM 28 DAY COMPRESSIVE STRENGTH FOR THE PRECAST CONCRETE DECK BEAMS SHALL BE 6500 PSI.

# REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

<u>MOI</u>	DIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1.	NONE	16"	19"	23"
2.	12" OF CONCRETE BELOW BAR	20"	25"	30"
3.	EPOXY COATED BARS, COVER < 3db, OR	23"	29"	34"
	CLEAR SPACING < 6db			
4.	COATED BARS, ALL OTHER CASES	18"	23"	27"
5.	CONDITION 2. AND 3.	26"	32"	39"
6.	CONDITION 2. AND 4.	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

ALL BARS SHALL BE EPOXY COATED.

# MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS - SPRAY APPLIED.

# PRECAST BEAMS:

THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING ABUTMENT DIMENSIONS PRIOR TO ORDERING ANY PRECAST ELEMENTS. ALL DIMENSIONS PROVIDED ARE BASED ON EXISTING BRIDGE PLANS AND SURVEY INFORMATION. AS-BUILT DIMENSIONS MAY VARY SLIGHTLY WHICH MAY IMPACT REQUIRED DIMENSIONS FOR PRECAST BEAM FABRICATIONS.

THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF LIFT HOOKS FOR ALL PRECAST ELEMENTS. UNDER NO CIRCUMSTANCE WILL THE REBAR ELEMENTS SHOWN ON THE PLANS BE USED TO LIFT THE PRECAST ELEMENTS. FOR ADDITIONAL REQUIREMENTS, REFER TO THE "PRECAST CONCRETE ELEMENTS" PORTION OF THE ITEM 995. IN THE SPECIAL PROVISIONS.

# TRAFFIC CONTROL:

ROADWAY SHALL BE CLOSED FOR THE DURATION OF CONSTRUCTION PER THE TEMPORARY TRAFFIC CONTROL PLAN (TTCP). MASSDOT ACCESS PERMIT FOR ROUTE 28 SHALL BE STRICTLY ADHERED TO AT ALL TIMES DURING CONSTRUCTION.

### UTILITIES:

EXISTING 10" WATER MAIN SHALL BE TEMPORARILY RELOCATED TO A TEMPORARY SUPPORT STRUCTURE (CONTRACTOR DESIGNED) TO MAINTAIN SERVICE DURING CONSTRUCTION. THE PERMANENT LINE WILL BE HUNG ON STEEL SUPPORTS ON THE WEST SIDE OF THE BRIDGE. CONTRACTOR SHALL COORDINATE WITH NORTH READING WATER DEPARTMENT FOR TEMPORARY AND PERMANENT RELOCATIONS.

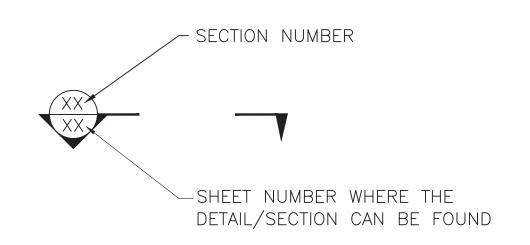
EXISTING 8" GAS MAIN OWNED BY NATIONAL GRID SHALL BE CUT AND CAP DURING CONSTRUCTION. THE PERMANENT LINE WILL BE HUNG ON STEEL SUPPORTS ON THE EAST SIDE OF THE BRIDGE. CONTRACTOR SHALL COORDINATE WITH NATIONAL GRID FOR TEMPORARY CUT AND CAP AND PERMANENT RELOCATION.

EXISTING OVERHEAD WIRES AND EXISTING TELEPHONE CONDUITS (SUPPORTED ON ADJACENT STEEL STRUCTURE)

ARE TO REMAIN IN PLACE AND ACTIVE THROUGHOUT THE DURATION OF CONSTRUCTION. CONTRACTOR SHALL USE
CAUTION WHILE WORKING AROUND THE EXISTING UTILITIES SCHEDULED TO REMAIN. CONTRACTOR SHALL UTILIZE LOW
HEIGHT HOISTING EQUIPMENT TO SET THE PRECAST ELEMENTS IN ORDER TO STAY UNDERNEATH THE EXISTING
OVERHEAD WIRES. THIS SHALL BE DESIGNED AND EXPLAINED IN THE CONTRACTOR'S ERECTION PROCEDURE.

EXISTING USGS STREAM GAUGE WILL BE REMOVED BY OTHERS PRIOR TO CONSTRUCTION.

# SECTION MARK:



		ESTIMATED QUANTITIES	
		(NOT GUARANTEED)	
ITEM	114.1	DEMOLITION OF SUPERSTRUCTURE OF BRIDGE NO. N-18-002	1 LS
ITEM	127.1	REINFORCED CONCRETE EXCAVATION	10 CY
ITEM	151.2	GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES	10 CY
ITEM	450.60	SUPERPAVE BRIDGE SURFACE COURSE (SSC-B - 9.5)	40 TON
ITEM	450.70	SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B - 9.5)	5 TON
ITEM	995.1	BRIDGE SUPERSTRUCTURE, BRIDGE NO. N-18-002 (7YC)	1 LS

# NORTH READING PARK STREET OVER MARTINS BROOK

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
MA	_	2	19	
	PROJECT FILE NO.		_	

#### **GENERAL NOTES**

TRAFFIC DATA									
	ROADWAY OVER	ROADWAY UNDER							
DESIGN YEAR	2043								
AVERAGE DAILY TRAFFIC — PRESENT	11200								
AVERAGE DAILY TRAFFIC — DESIGN YEAR	13660								
DESIGN HOURLY VOLUME	1110								
DIRECTIONAL DISTRIBUTION	62.2	X							
TRUCK PERCENTAGE — AVERAGE DAY	4.0								
TRUCK PERCENTAGE — PEAK HOUR	4.8								
DESIGN SPEED	35 MPH								
DIRECTIONAL DESIGN HOURLY VOLUME	690								

HYDRAULIC DESIGN DATA	
DRAINAGE AREA (SQ. MILES)	13.2
DESIGN FLOOD DISCHARGE (C.F.S.)	464
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	N/A
DESIGN FLOOD ELEVATION (FEET, NAVD)	69.8
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	868
BASE FLOOD ELEVATION (FEET, NAVD)	73.3
DESIGN AND CHECK SCOUR DATA	
DESIGN SCOUR FLOOD EVENT	N1 / A
RETURN FREQUENCY (YEARS)	N/A
DESIGN FLOOD ABUTMENT SCOUR DEPTH (FEET)	N/A
DESIGN FLOOD PIER SCOUR DEPTH (FEET)	N/A
CHECK SCOUR FLOOD EVENT	N/A
RETURN FREQUENCY (YEARS)	N/ A
CHECK FLOOD ABUTMENT SCOUR DEPTH (FEET)	N/A
CHECK FLOOD PIER SCOUR DEPTH (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	N/A
FREQUENCY (IF KNOWN, YEARS)	N/A
MAXIMUM ELEVATION (FEET, NAVD)	N/A
DATE (MM/YYYY)	N/A
HISTORY OF ICE FLOES	N/A
EVIDENCE OF SCOUR	N/A
AND EROSION	IN/A

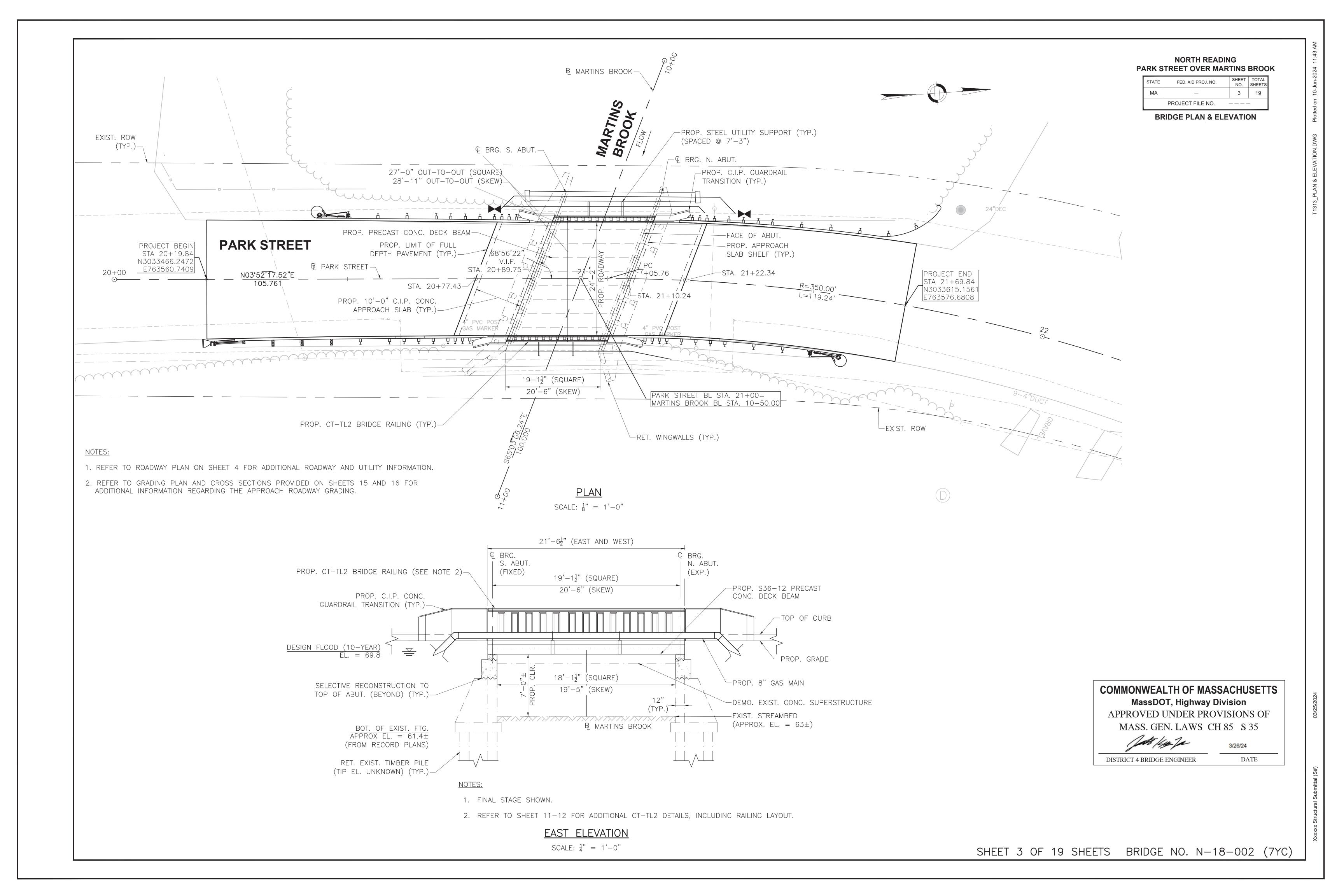
# COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division APPROVED UNDER PROVISIONS OF MASS. GEN. LAWS CH 85 S 35

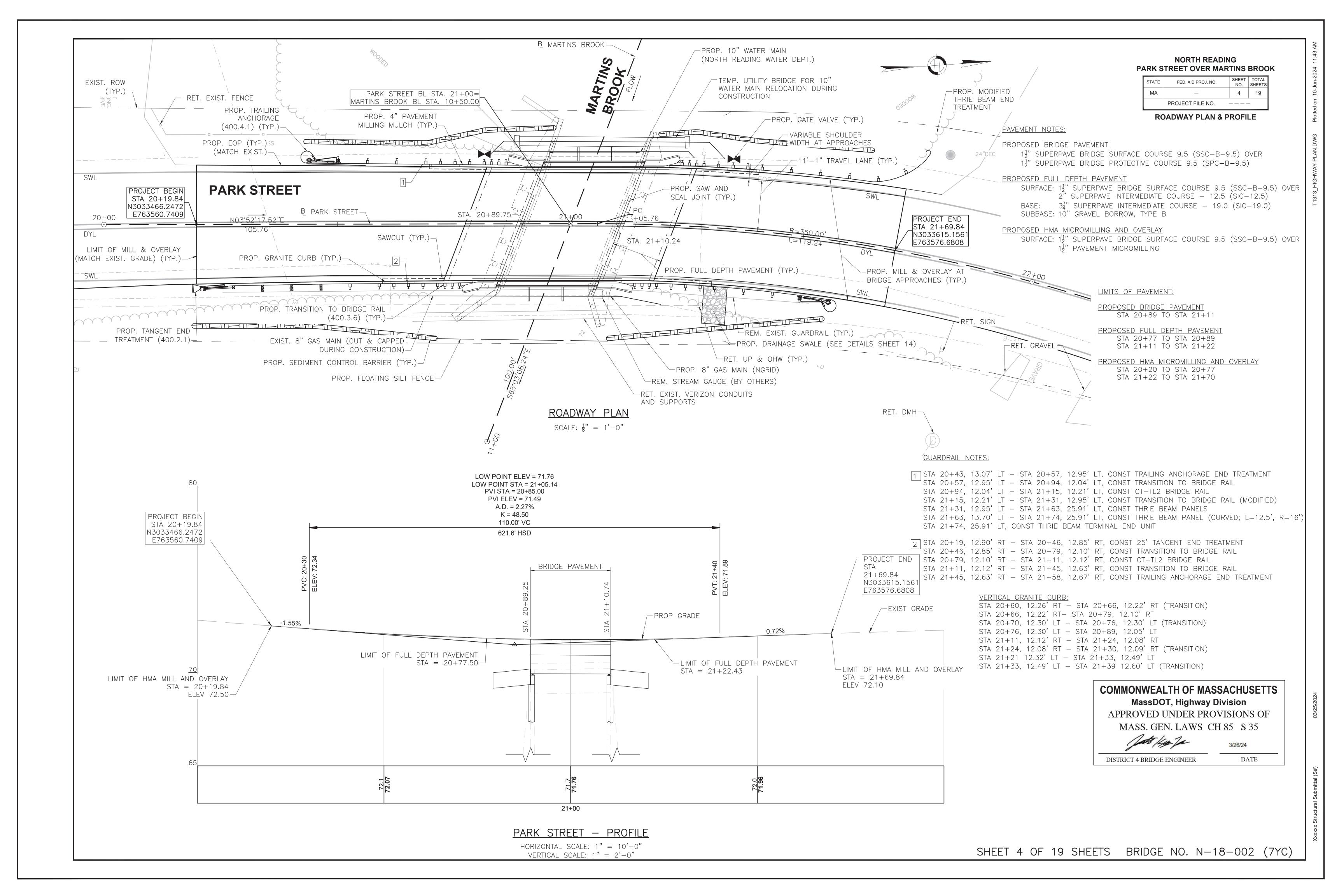
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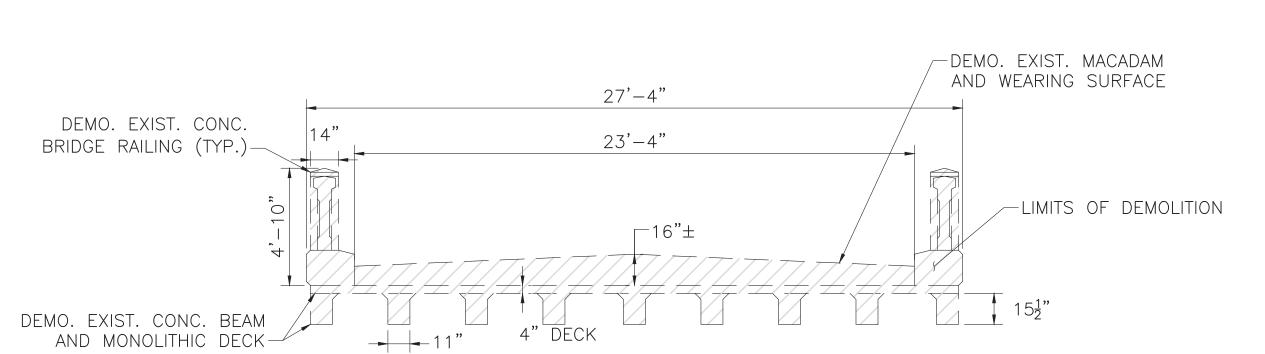
3/26/24

DISTRICT 4 BRIDGE ENGINEER

DATE







EXISTING UTILITIES NOT SHOWN FOR CLARITY. WATER MAIN WILL BE TEMPORARILY RELOCATED AND GAS MAIN WILL BE PERMANENTLY RELOCATED PRIOR TO DEMOLITION.

# **EXISTING BRIDGE SECTION**

SCALE: NOT TO SCALE

## **DEMOLITION NOTES:**

- 1. EXISTING BRIDGE GEOMETRY SHOWN THROUGHOUT THIS SHEET IS BASED ON EXISTING BRIDGE PLANS. CONTRACTOR SHALL VERIFY AS-BUILT BRIDGE DIMENSIONS PRIOR TO DEMOLITION.
- 2. BRIDGE DEMOLITION SHALL NOT BEGIN UNTIL EXISTING UTILITIES ARE RELOCATED TO THEIR TEMPORARY OR PERMANENT LOCATION, THE DETOUR IS IN PLACE, AND DETOUR SIGNAGE IS APPROVED BY ENGINEER.
- 3. PRIOR TO START OF BRIDGE DEMOLITION, A DEMOLITION AND TEMPORARY SHIELDING PROCEDURE STAMPED BY A REGISTERED ENGINEER IN THE COMMONWEALTH OF MASSACHUSETTS SHALL BE APPROVED BY THE ENGINEER.
- 4. TEMPORARY SHIELDING SHALL BE INSTALLED BENEATH THE BRIDGE PRIOR TO BEGINNING ANY BRIDGE DEMOLITION.

# LEGEND:

LIMITS OF DEMOLITION

# COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division APPROVED UNDER PROVISIONS OF MASS. GEN. LAWS CH 85 S 35

DISTRICT 4 BRIDGE ENGINEER

DATE

3/26/24

# LIMITS OF DEMOLITION EXIST. BEAM SEAT (SOUTH) EL. = 68.84EXIST. BEAM SEAT (NORTH) EL. = 68.75BOT. OF EXIST. CONC. BEAM PROP. LIMIT OF DEMO (N & S) EL. = 67.71 12" 12"

# SECTION 1 SCALE: NOT TO SCALE

# <u>ABUTMENT DEMOLITION LIMITS — ELEVATION</u>

SCALE: NTS

B MARTINS BROOK-

PC +05.7/6

**DEMOLITION PLAN** 

SCALE: NOT TO SCALE

PARK STREET

-RET. EXIST.

WINGWALL (TYP.)

EXIST. BRIDGE N-18-002-

₽ PARK STREET —

21'-6"-

LIMIT OF DEMO

EL. 67.71

S. ABUT. (SKEW)

N. ABUT. (SKEW)

S. ABUT. (SKEW)

N. ABUT. (SKEW)

S. ABUT. (SQUARE)

N. ABUT. (SQUARE)

S. ABUT. (SQUARE)

N. ABUT. (SQUARE)

DIMENSIONS

 $15'-4\frac{1}{2}"$ 

14'-4"

14'-5"

13'-6"

14'-5"

13'-6"

 $15'-1\frac{3}{4}$ "

 $14' - 1\frac{3}{4}$ "

DEMO. EXIST. CONC. BRIDGE RAILING (TYP.)

N3°52'18"E

RET. EXIST.

TIMBER PILE (TYP.)

-DEMO. EXIST. CONC. BEAMS

-LIMIT OF SELECTIVE

-RET. EXIST.

WINGWALL (TYP.)

ABUT. DEMO.

AND MONOLITHIC DECK

 $R = 350.00^{\circ}$ L=119.24

-SELECTIVE DEMO. TO EXIST. CONC. SUBSTRUCTURE (TYP.)

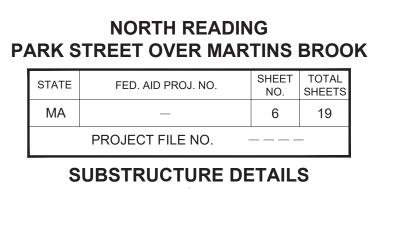
EXIST. COUNTERFORT (TYP.)

DETAIL INTENDED TO ILLUSTRATE ABUTMENT DEMOLITION

RELATED TO SUPERSTRUCTURE DEMOLITION LIMITS.

LIMITS. SUPERSTRUCTURE NOT SHOWN FOR CLARITY. REFER TO DETAILS ON THIS SHEET FOR ADDITIONAL INFORMATION

SHEET 5 OF 19 SHEETS BRIDGE NO. N-18-002 (7YC)



-1"X5" NEOPRENE PAD,

60 DUROMETER (TYP.)

-€ BEAM (TYP.)

PROVIDE  $\frac{1}{8}$ "/FT. SLOPE BETWEEN BEARINGS.

LAYOUT OF BEARINGS

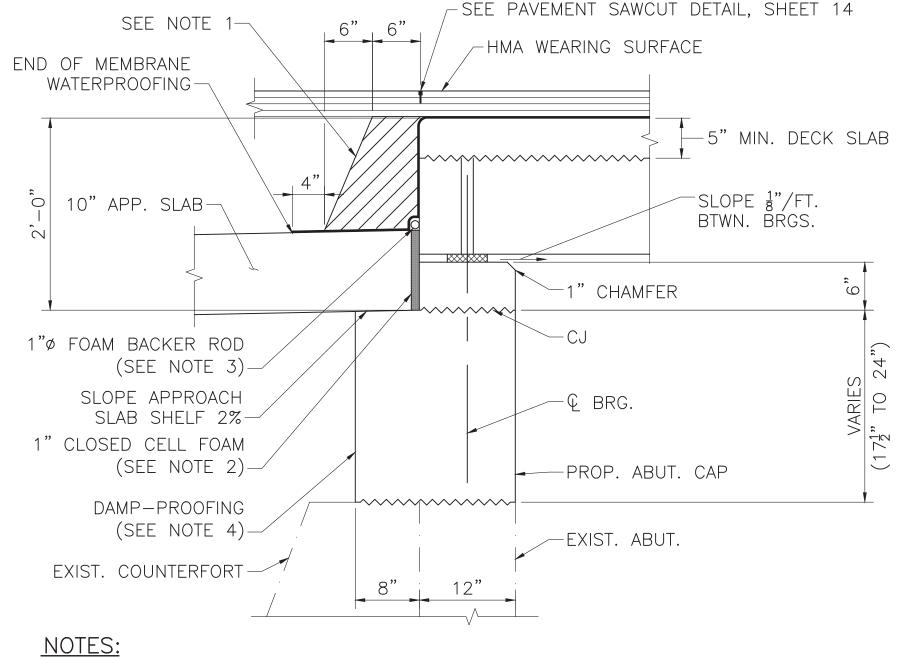
SCALE:  $\frac{1}{4}$ " = 1'-0"

SEE NOTE

—́Q BRGS.

NOTE:

 $1\frac{1}{2}$ "Ø SLEEVE (TYP.).



# 1. PROTECTIVE COURSE TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5),

2. ATTACH CLOSED CELL FOAM TO BACK OF ABUTMENT CAP AND DECK BEAM WITH ADHESIVE.

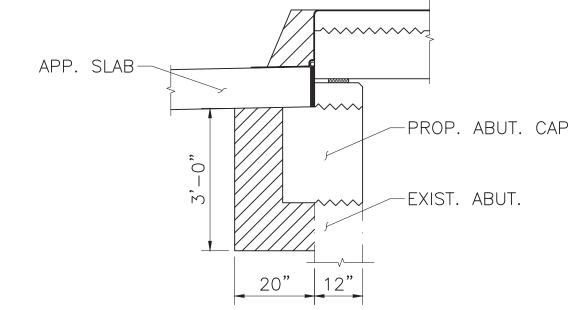
PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER WITHIN

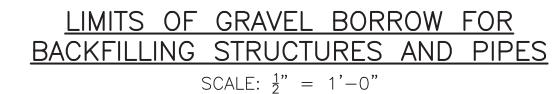
3. SPRAY MEMBRANE WATERPROOFING OVER CLOSED CELL FOAM BACKER ROD.

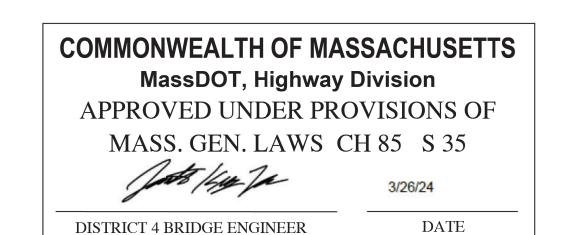
12 HOURS AFTER PLACING MEMBRANE WATERPROOFING.

- 4. APPLY DAMP PROOFING TO THE ENTIRE BACKSIDE OF THE NEW ABUTMENT CAP (INCLUDING 6" BELOW DEMO LINE) AND ANY HORIZONTAL FACE OF THE DEMOLISHED COUNTERFORTS.
- 5. REFER TO SECTIONS 2 AND 3 ON THIS SHEET FOR ABUTMENT CAP REINFORCING.









# SOUTH ABUTMENT ELEVATION

28'-11"

(SKEW)

NORTH ABUTMENT ELEVATION

SCALE:  $\frac{1}{4}$ " = 1'-0"

28'-11"

(SKEW)

PARK STREET

14'-5"

(SKEW)

14'-6"

(SKEW)

APP. SLAB SHELF/CJ-

NW BM SEAT EL. 70.21

EL. 69.71

LIMIT OF DEMO/

TOP OF EXIST. DECK

(APP. EL. 70.38±)-

SE BM SEAT EL. 69.67

EL. 69.17

PROP. ABUT. CAP-

B PARK STREET

14'-6"

(SKEW)

-APP. SLAB SHELF/CJ

14'-5"

(SKEW)

-EXIST. COUNTERFORT (TYP.)

-TOP OF EXIST. DECK

(APP. EL. 70.38±)

NE BM SEAT

L. 69.67

EL. 69.17

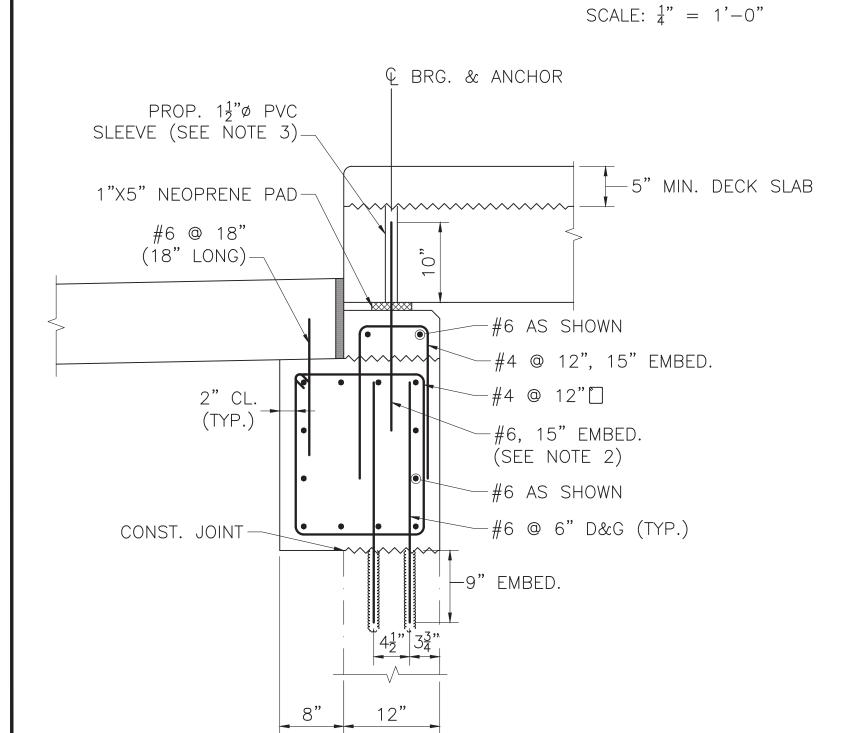
-PROP. ABUT. CAP

SW BM SEAT

EL. 69.71

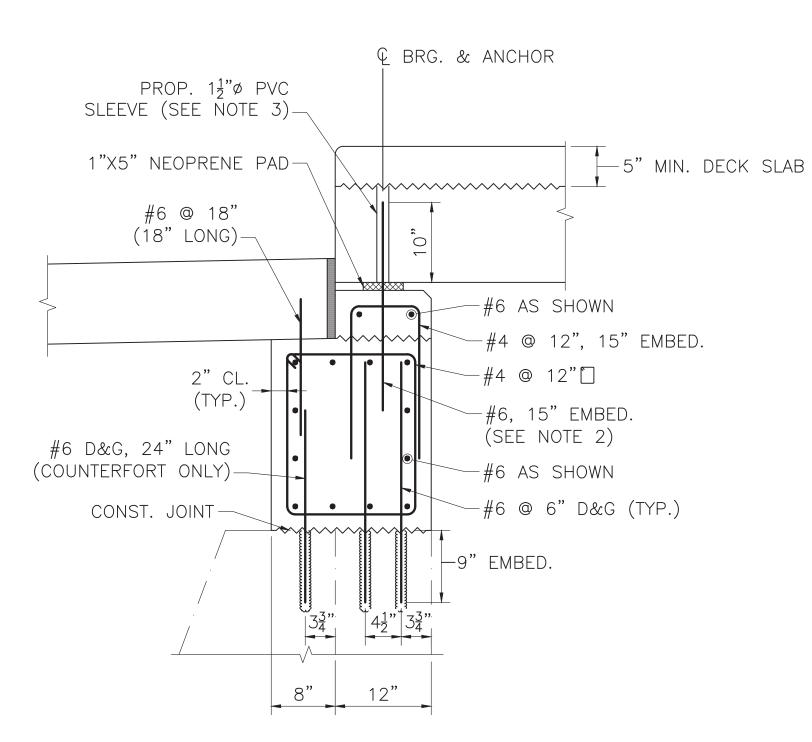
EL. 67.71

LIMIT OF DEMO



EXIST. COUNTERFORT (TYP.)-

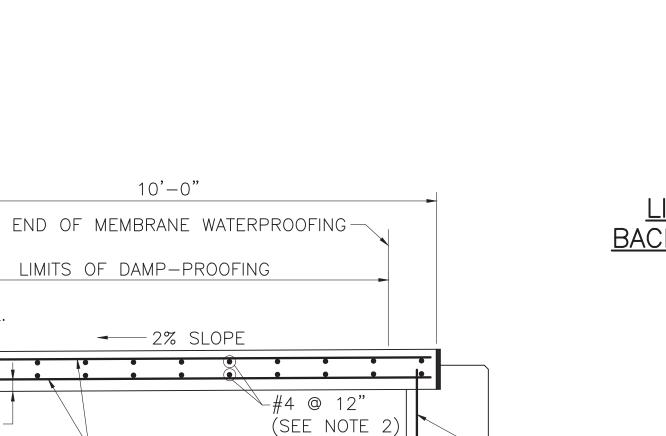
SECTION 2 - ABUTMENT CAP DETAIL SCALE: 1" = 1'-0"



<u>SECTION 3 — ABUTMENT CAP DETAIL AT COUNTERFORT</u> SCALE: 1" = 1'-0"

# NOTES:

- 1. ALL REBAR SCHEDULED TO BE DRILLED AND GROUTED SHALL BE GROUTED WITH HILTI HIT-RE 500 V3 EPOXY, OR APPROVED EQUIVALENT.
- 2. BARS SHALL BE CAST INTO ABUTMENT CAP. SEE FRAMING PLAN (SHEET 7) FOR BAR LAYOUT & SPACING.
- 3. FILL SLEEVE WITH GROUT AT FIXED END (SOUTH) AND WITH EXPANDED POLYSTYRENE FILLER AT EXPANSION END (NORTH).



-#6 @ 18"



┌2" CL.

3" CL. -

1. APPROACH SLAB TO BE 4000 PSI,  $\frac{3}{4}$  IN, 585 HP CEMENT CONCRETE.

10'-0"

**→** 2% SLOPE

LIMITS OF DAMP-PROOFING

<sup>\_</sup>#7 @ 6"

(SEE NOTE 2)

2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO CENTERLINE OF CONSTRUCTION. PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.

# APPROACH SLAB DETAILS SCALE: $\frac{1}{2}$ " = 1'-0"

### NORTH READING PARK STREET OVER MARTINS BROOK FED. AID PROJ. NO. 7 19 MA PROJECT FILE NO.

### FRAMING PLAN

**CONSTRUCTION SEQUENCE NOTES:** 

TRANSVERSE TIE TO 5 KIPS.

DEBRIS UNTIL THEY ARE FILLED.

1. AFTER ALL BEAMS HAVE BEEN ERECTED, TENSION EACH

2. FILL ALL KEYWAYS AND ANCHOR SLEEVES AT SOUTH

3. AFTER THE MORTAR HAS CURED (24 HOURS MINIMUM),

TRANSVERSE TIES HAVE BEEN FULLY TENSIONED.

CURED PER THE STANDARD SPECIFICATIONS.

TENSION EACH TRANSVERSE TIE TO 44 KIPS.

ABUTMENT WITH MORTAR (M4.04.0). IF THE KEYWAYS AND

ANCHOR SLEEVES ARE NOT FILLED WITHIN FIVE (5) DAYS

AFTER THE BEAMS ARE ERECTED, THE CONTRACTOR SHALL

COVER AND PROTECT THE KEYWAYS FROM WEATHER AND

4. CONCRETE FOR DECK SLAB SHALL BE 4000 PSI,  $\frac{3}{4}$  IN, 585

5. NO TRAFFIC OR HEAVY EQUIPMENT WILL BE PERMITTED ON

THE BRIDGE UNTIL ALL TRANSVERSE TIES HAVE BEEN

HP CEMENT CONCRETE AND SHALL BE PLACED AFTER THE

PROPERLY TENSIONED AND THE DECK HAS BEEN CAST AND

#### INT. BEAM EXT. BEAM DIRECTION DIRECTION (BM. 2-8)(BM. 1 & 9) AT TRANSFER UP UP UP AT ERECTION UP AFTER UP CONSTRUCTION

# NOTES:

- 1. TRANSFER DEFLECTIONS INCLUDE BEAM SELF WEIGHT AND BEAM CAMBER.
- SUPERIMPOSED DEAD LOADS, AND BEAM CAMBER WITH PCI ERECTION MULTIPLIERS APPLIED TO THE BEAM SELF WEIGHT

LOCATION	LEFT EDGE OF DECK SLAB	PROFILE GRADE LINE	RIGHT EDGE OF DECK SLAB
Ç OF BRGS. ◎ S. ABUT.	6"	6 <mark>1</mark> "	5 <u>1</u> "
MIDSPAN	5 <mark>7</mark> "	5 <mark>7</mark> "	5 <u>1</u> "
Q OF BRGS. @ N. ABUT.	6"	5 <mark>7</mark> "	5"

# NOTES:

- 1. THIS TABLE INDICATES THE THEORETICAL THICKNESS OF THE DECK SLAB IN INCHES BASED UPON ASSUMED BEAM CHAMBERS AT ERECTION.
- 2. TABLE IS PROVIDED TO ASSIST IN ESTIMATING THE REQUIRED CONCRETE VOLUME.
- 3. THE ACTUAL DECK THICKNESS WILL BE AS REQUIRED TO MEET THE PROFILE GRADES.

# THEORETICAL DECK SLAB THICKNESS TABLE

LOCATION	BEAM SEAT EL.	TOP OF CONC. DECK EL.	STATION	OFFSET
WP1	70.21	71.79	20+94.95	LT 13.50
WP2	69.94	71.53	20+89.75	0.00
WP3	69.67	71.22	20+84.55	RT 13.50
WP4*	70.21	71.79	21+15.09	LT 13.63
WP5*	69.94	71.51	21+10.24	0.00
WP6*	69.67	71.17	21+05.04	RT 13.50

CONTRACTOR TO NOTE PRESENCE OF PC ON BRIDGE.

BEAM SEAT ELEVATIONS

21'-7"

 $9'-2\frac{1}{4}"$ 

BM 1

(TYP.)

LEFT EDGE OF

DECK SLAB-

PC +05.76

WP5

BM 2

BM 3

BM 4

BM 6

BM 7

(TYP.)

SEE STANDARD SPECIFICATIONS FOR BEAM ERECTION AND LAYOUT.

RIGHT EDGE OF DECK SLAB—

DECK PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

FRAMING PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

BM 8

BM 9

₽ BRG. S. ABUT.—

TYP. THIS END)\_

68°56'22"

(CONTRACTOR /

V.I.F.)

18" (TYP.)

NOTE:

₽ BRG. S. ABUT.—

WP2-

WP3

 $1\frac{1}{2}$ " SLEEVE W/  $\frac{3}{4}$ "  $\phi$ 

ANCHOR ROD (FIXED,

STA. 20+89.75-

20°57'34"

(CONTRACTOR

V.I.F.)

 $9'-2\frac{1}{4}"$ 

PC +05.76

-SPACING OF Q's OF

3" Ø SLEEVES FOR TRANSVERSE TIES

∕—Ç BRG. N. ABUT.

 $/-1\frac{1}{2}$ " SLEEVE W/  $\frac{3}{4}$ "  $\phi$ 

TYP. THIS END)

/─B PARK STREET

─STA. 21+10.24

12" (TYP.)

-FACE OF ABUT. (TYP.)

-END OF BEAM (TYP.)

-PROP. STEEL UTILITY SUPPORT (TYP.)

−Ç BRG. N. ABUT.

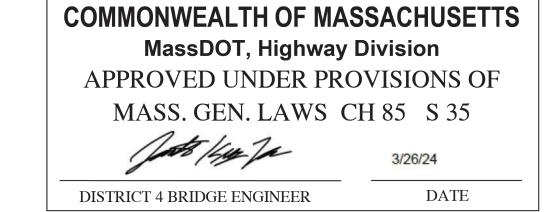
/─B PARK STREET

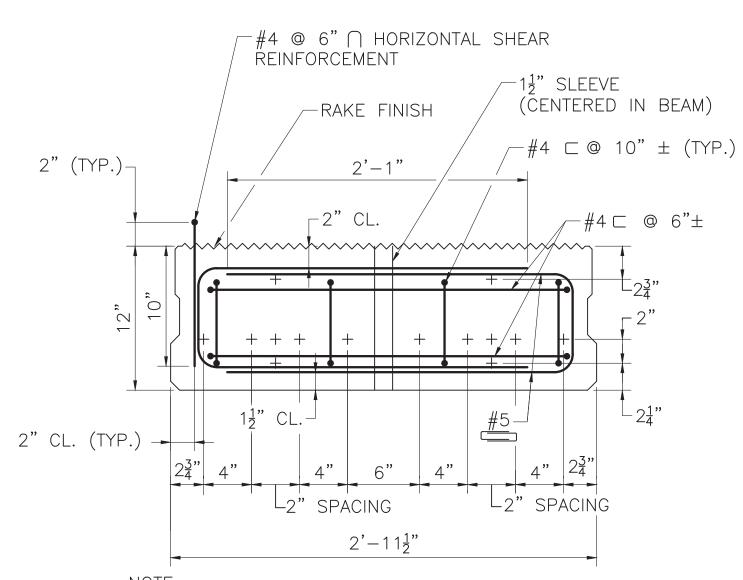
ANCHOR ROD (EXPANSION,

- 2. ERECTION DEFLECTIONS INCLUDE SELF WEIGHT, DECK, AND CAMBER.
- 3. AFTER CONSTRUCTION DEFLECTIONS INCLUDE SELF WEIGHT, DECK, SUPERIMPOSED DEAD LOADS, AND BEAM CAMBER WITH PCI FINAL MULTIPLIERS APPLIED TO ALL LOADS.

# ESTIMATED INITIAL DEFLECTIONS AT MIDSPAN

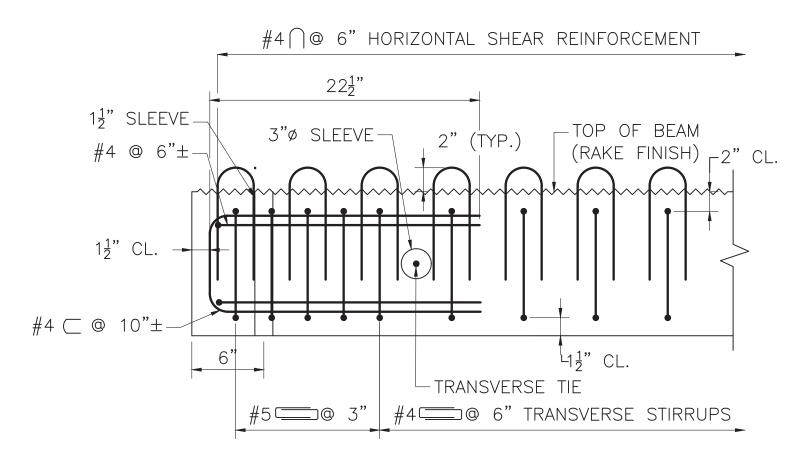
LOCATION	LEFT EDGE OF DECK SLAB	PROFILE GRADE LINE	RIGHT EDGE OF DECK SLAB			
€ OF BRGS.	6"	6 <mark>1</mark> "	5 <del>1</del> "			
MIDSPAN	5 <u>7</u> "	5 <del>7</del> "	5 <u>1</u> "			
€ OF BRGS.	6"	5 <sup>7</sup> <sub>8</sub> "	5"			





END OF BEAM SECTIONS SHOWN AT THE CENTERLINE OF BEARING. SEE BELOW DETAIL FOR TRANSVERSE TIE LOCATIONS AT BEAM ENDS.

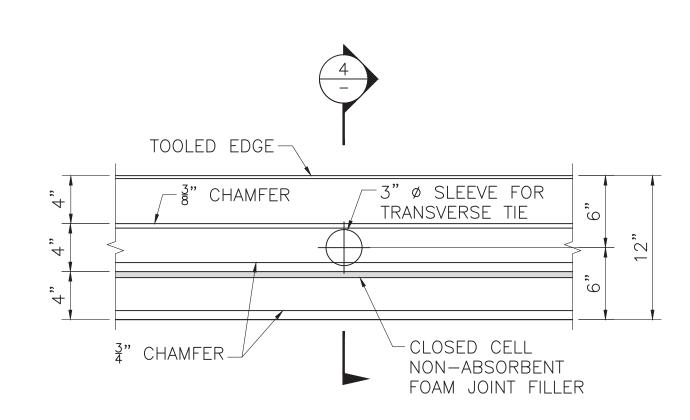
# END OF BEAM SECTION SCALE: $1\frac{1}{2}$ " = 1'-0"



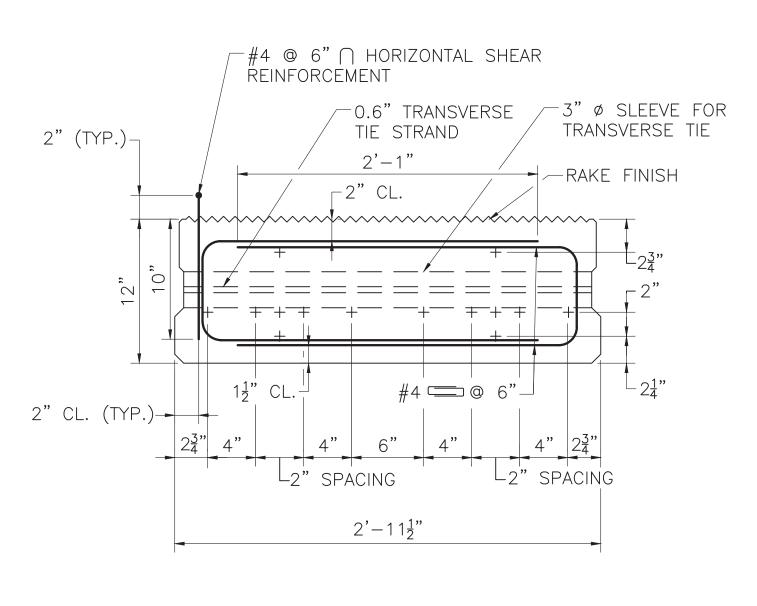
NOIE: THE STRANDS ARE NOT SHOWN FOR CLARITY.

# LONGITUDINAL SECTION

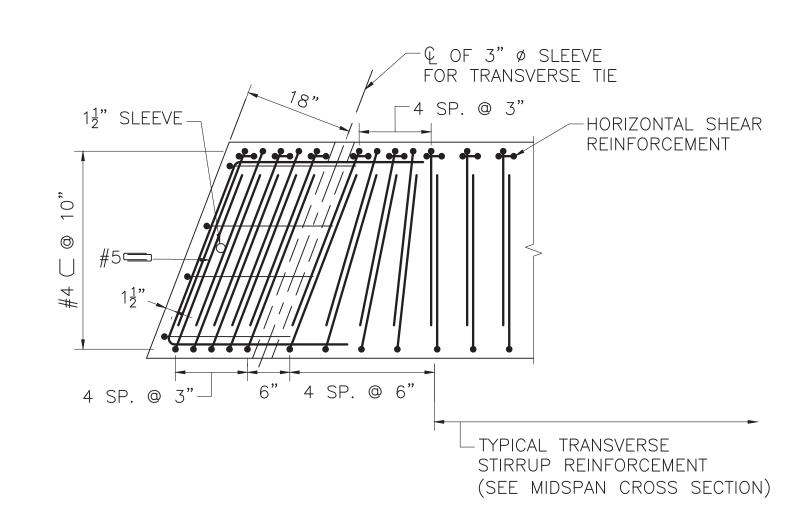
SCALE:  $1\frac{1}{2}$ " = 1'-0"



TYPICAL BEAM ELEVATION AT TRANSVERSE TIE LOCATIONS SCALE:  $1\frac{1}{2}$ " = 1'-0"



# MIDSPAN SECTION SCALE: $1\frac{1}{2}$ " = 1'-0"



# END OF BEAM PLAN SCALE: $\frac{3}{4}$ " = 1'-0"

-MORTAR (M4.04.0)

TIE STRAND

-CLOSED CELL

NON-ABSORBENT

FOAM JOINT FILLER

\_\_0.6" TRANSVERSE

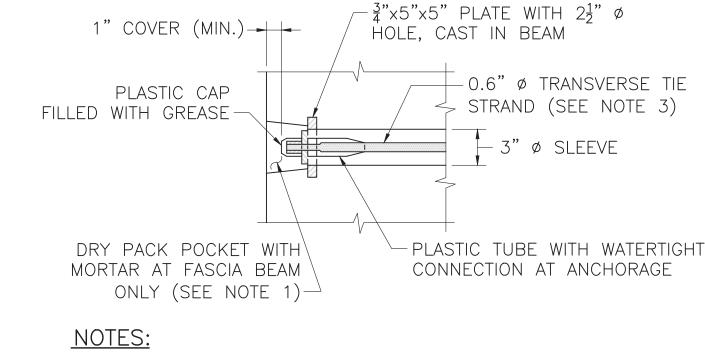
TOP OF BEAM

SECTION 4

SCALE:  $1\frac{1}{2}$ " = 1'-0"

3" Ø SLEEVE-

(RAKE FINISH)-



- 1. MORTAR FOR EXTERIOR POCKETS SHALL CONFORM TO M4.02.15 AND SHALL BE THE SAME COLOR AND TEXTURE AS THE BEAM CONCRETE.
- 2. OTHER ANCHORAGE SYSTEMS MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. ALTERNATE ANCHORAGE SYSTEMS SHALL BE WATERTIGHT AND CORROSION PROOF.
- 3. TRANSVERSE TIES SHALL BE COVERED BY A SEAMLESS POLYPROPYLENE SHEATH (WITH CORROSION INHIBITING GREASE BETWEEN THE STRAND AND SHEATH) FOR THE FULL LENGTH OF THE STRAND, EXCEPT AT THE ANCHORAGE LOCATION.

TRANSVERSE TIE ANCHORAGE SCALE:  $1\frac{1}{2}$ " = 1'-0"



STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	_	8	19
	PROJECT FILE NO.		

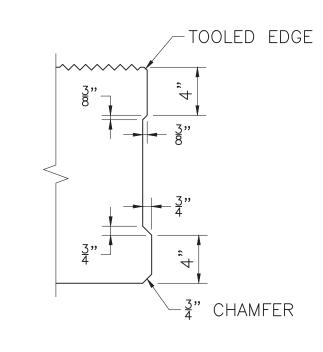
**BEAM DETAILS** 

## **BEAM SECTION NOTES:**

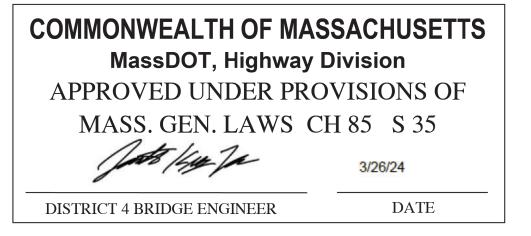
- 1. + DENOTES STRAIGHT STRANDS.
- 2. SEE END OF BEAM PLAN FOR STIRRUP SPACING.
- 3. MAINTAIN ALL CLEARANCES AS SHOWN ON THE PLANS.
- 4. OMIT SHEAR KEYS FROM FASCIA SIDES OF EXTERIOR BEAMS (BEAMS 1 AND 9).
- 5. SEE UTILITY ANCHOR NOTES ON SHEET 9.
- 6. SEE SECTION THROUGH SAFETY CURB DETAIL ON SHEET 12 FOR ADDITIONAL REBAR TO BE CAST INTO BEAMS 1 & 9.

# PRESTRESS NOTES:

- 1. ALL PRETENSIONING ELEMENTS SHALL BE 0.6" Ø, UNCOATED, SEVEN-WIRE, LOW RELAXATION STEEL STRANDS AND SHALL CONFORM TO AASHTO M 203
- 2. THE TENSILE STRENGTH OF THE PRETENSIONING SHALL BE 270 KSI.
- 3. THE INITIAL TENSION PER 0.6" Ø STRAND SHALL BE 44 KIPS.
- 4. THE TOP STRANDS SHALL BE PRETENSIONED TO 2 KIPS EACH.
- 5. THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 6500 PSI.
- 6. NO PRESTRESS SHALL BE TRANSFERRED TO THE CONCRETE UNTIL IT HAS ATTAINED A COMPRESSIVE STRENGTH, AS SHOWN BY CYLINDER TEST, OF AT LEAST 5200 PSI.
- 7. THE TOP OF ALL BEAMS SHALL BE GIVEN A RAKE FINISH  $\binom{1}{4}$ " AMPLITUDE) ACROSS THE WIDTH (PERPENDICULAR TO THE BEAM'S AXIS).
- 8. THE FABRICATOR IS FULLY RESPONSIBLE FOR THE DESIGN OF THE LIFTING DEVICES WHICH SHALL BE ADEQUATE FOR THE SAFETY FACTORS REQUIRED BY THE ERECTION PROCEDURE.



SHEAR KEY DETAIL SCALE:  $1\frac{1}{2}$ " = 1'-0"



TEMP. RELOCATED 10"ø

WATER LINE DURING

-ANGLE WITH SUPPORT

BRACKETS, ALL HOLES

AND SLOTTED 1"

SHALL BE PRE-DRILLED

CONSTRUCTION

\_\_W14 (TYP.)

# MEMBRANE WATERPROOFING (SPRAY APPLIED)-#5 @ 9"*-*-3" HMA WEARING SURFACE 5" MIN. DECK SLAB 3'-0"

# NOTES:

- 1. ROADWAY DECK SLAB SHALL BE 4000 PSI,  $\frac{3}{4}$  IN, 585 HP CEMENT CONCRETE.
- 2. LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE  $\mathbb Q$  OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE  $\mathbb Q$  OF CONSTRUCTION.
- 3. ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE EPOXY COATED.

TYPICAL DECK REINFORCEMENT

SCALE:  $\frac{3}{4}$ " = 1'-0"

4. THE FINISHED SURFACE OF BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.

# NOTES:

- 1. THIS DETAIL IS CONCEPTUAL ONLY, ALL TEMPORARY BRIDGE ELEMENTS SHALL BE CONTRACTOR DESIGNED. THIS DETAIL IS PROVIDED FOR CONTINGENCY MEASURES ONLY, AS THE GAS MAIN IS SCHEDULED FOR A CUT AND CAP CURING CONSTRUCTION FOLLOWED BY A PERMANENT RELOCATION. SEE SPECIAL PROVISIONS ITEM 992.321 FOR MORE INFORMATION ON CONTINGENCY MEASURES FOR TEMPORARILY SUPPORTING THE GAS MAIN.
- 2. THE TEMPORARY UTILITY BRIDGE SHALL BE DESIGNED TO ENSURE THAT THE MAXIMUM OUT-TO-OUT WIDTH DOES NOT RESULT IN THE BRIDGE BEING LOCATED BEYOND THE EXISTING R.O.W.
- 3. NATIONAL GRID GAS WILL PROVIDE AND INSTALL THE TEMPORARY GAS PIPE. NATIONAL GRID WILL PROVIDE THE ROLLER ASSEMBLIES.
- 4. THE ASSUMED LOADING OF THE STEEL GAS PIPE AND ROLLER SUPPORTS SHALL BE CONSIDERED TO BE 14 PLF. AT A MINIMUM, THE SUPERSTRUCTURE AND SUBSTRUCTURE SHALL BE DESIGNED TO SUPPORT THESE SUPERIMPOSED LOADS.
- 5. UTILITY RELOCATIONS SHALL OCCUR PRIOR TO DEMOLITION OF THE SUPERSTRUCTURE.
- 6. ONCE UTILITIES ARE MOVED TO PERMANENT LOCATIONS, THE TEMPORARY SUPPORTS SHALL BE REMOVED AND ANCHOR RODS SHALL BE CUT.

# CONTINGENCY TEMPORARY GAS SUPPORT SCHEMATIC

SEE NOTE 6

TEMP. 4"Ø GAS LINE (BY OTHERS)

-W-BEAM (CONTRACTOR DESIGNED)

DURING CONSTRUCTION

-PROP. GROUT PAD

TO PLACING GROUT PAD

─8" (TYP.)

(NON-SHRINK GROUT)

-ROUGHEN SURFACE PRIOR

NATIONAL GRID TO INSTALL GAS

DRILL AND GROUT ROD

INTO TOP OF WINGWALL-

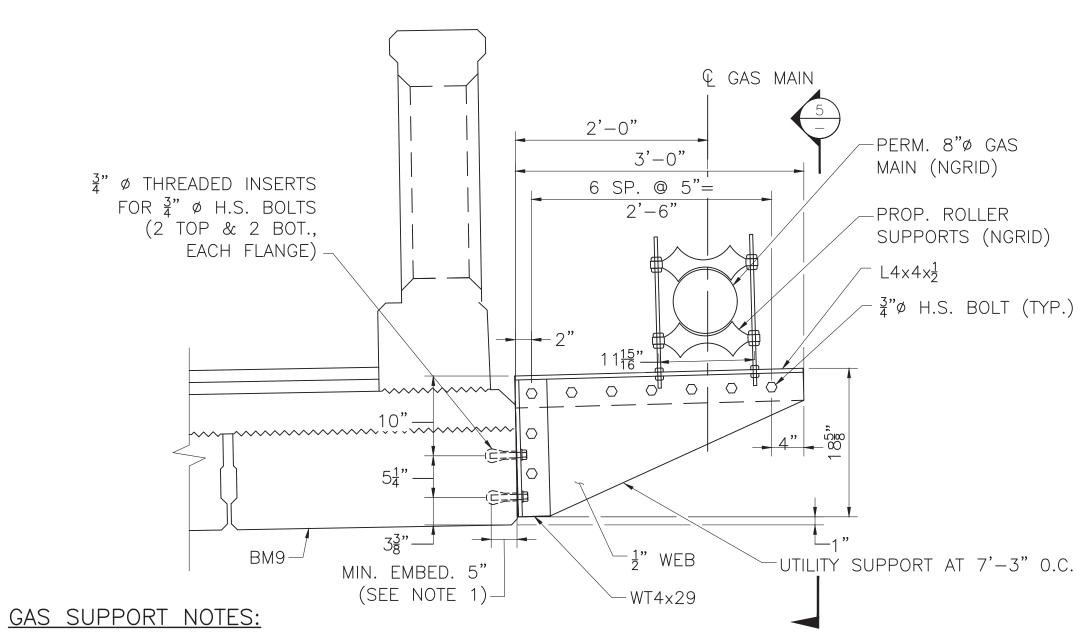
EXIST. ABUTMENT —

LINE AND ROLLER SUPPORTS-

PRE-DRILL 3/4" HOLES

FOR § ROD (TYP.)

2" MIN.-



- 1. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING LOCATIONS OF THREADED INSERTS IN PRECAST ELEMENTS PRIOR TO FABRICATION.
- 2. STRUCTURAL STEEL IN UTILITY SUPPORTS SHALL CONFORM TO AASHTO M270, GRADE 36. ALL STRUCTURAL STEEL AND FASTENERS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111 AND M232.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ALL STEEL FOR UTILITY SUPPORTS, AS DETAILED ABOVE.

PERMANENT GAS SUPPORT DETAIL

SCALE: 1" = 1'-0"

-1" / L4x4x $\frac{1}{2}$  <

SECTION 5

SCALE: 1" = 1'-0"

-PROP. ROLLER SUPPORTS (NGRID)

-2 SP. @ 5" = 10"

THE  $\frac{3}{4}$ "Ø THREADED INSERTED FOR  $\frac{3}{4}$ "Ø BOLTS SHALL BE CAST INTO THE PRECAST BEAMS BY THE FABRICATOR AND SHALL PROVIDE A MINIMUM NOMINAL TENSILE RESISTANCE Of 6.0 KIPS AND A MINIMUM NOMINAL SHEAR RESISTANCE OF 6.0 KIPS IN 3000 PSI CONCRETE.

8"ø GAS MAIN—

1" WEB

WT4x29----

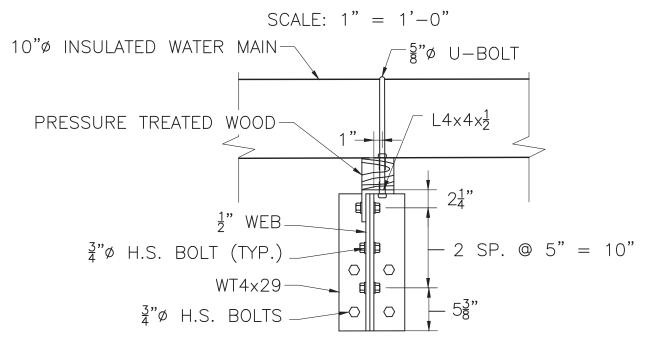
¾"ø H.S. BOLT (TYP.) \_

3"ø H.S. BOLTS

# WATER SUPPORT NOTES: 1. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING LOCATIONS OF THREADED INSERTS IN PRECAST ELEMENTS PRIOR TO FABRICATION.

- 3. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ALL STEEL FOR UTILITY SUPPORTS, AS DETAILED ABOVE.
- 4. THE  $\frac{3}{4}$ "Ø THREADED INSERTED FOR  $\frac{3}{4}$ "Ø BOLTS SHALL BE CAST INTO THE PRECAST BEAMS BY THE FABRICATOR AND SHALL PROVIDE A MINIMUM NOMINAL TENSILE RESISTANCE Of 6.0 KIPS AND A MINIMUM

# PERMANENT WATER SUPPORT DETAIL

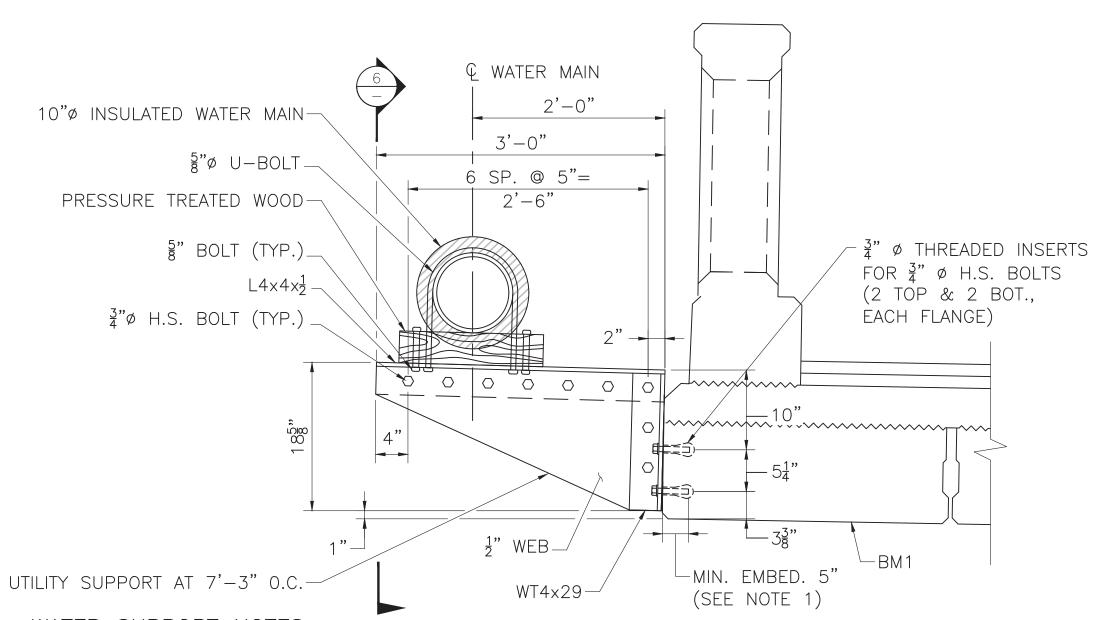


SECTION 6 SCALE: 1" = 1'-0"

# **COMMONWEALTH OF MASSACHUSETTS MassDOT**, Highway Division APPROVED UNDER PROVISIONS OF MASS. GEN. LAWS CH 85 S 35 3/26/24

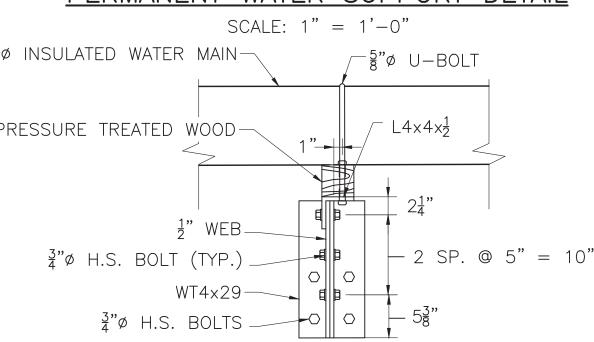
4 GIRDER

SCALE: N.T.S.



2. STRUCTURAL STEEL IN UTILITY SUPPORTS SHALL CONFORM TO AASHTO M270, GRADE 36. ALL STRUCTURAL STEEL AND FASTENERS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111 AND M232.

NOMINAL SHEAR RESISTANCE OF 6.0 KIPS IN 3000 PSI CONCRETE.



SHEET 9 OF 19 SHEETS BRIDGE NO. N-18-002 (7YC)

DISTRICT 4 BRIDGE ENGINEER

**GIRDER** 

BEYOND THE EXISTING R.O.W.

SUPERSTRUCTURE.

NOTES:

2'-3"

(MIN.)

SEE NOTE 2

PROTECTED FROM FLOATING DEBRIS AND DEBRIS DURING DEMOLITION.

2. THE TEMPORARY UTILITY BRIDGE SHALL BE DESIGNED TO ENSURE THAT THE

1. THIS DETAIL IS CONCEPTUAL ONLY, ALL BRIDGE ELEMENTS (SUPERSTRUCTURE AND SUBSTRUCTURE) FOR THE TEMPORARY UTILITY BRIDGE SHALL BE CONTRACTOR DESIGNED. THE DESIGN SHALL ENSURE THAT THE TEMPORARY WATER LINE IS

MAXIMUM OUT-TO-OUT WIDTH DOES NOT RESULT IN THE BRIDGE BEING LOCATED

3. THE ASSUMED UNFACTORED LOADING OF THE TEMPORARY WATER PIPE SHALL BE

CONSIDERED TO BE 85 PLF. AT A MINIMUM, THE TEMPORARY SUPERSTRUCTURE

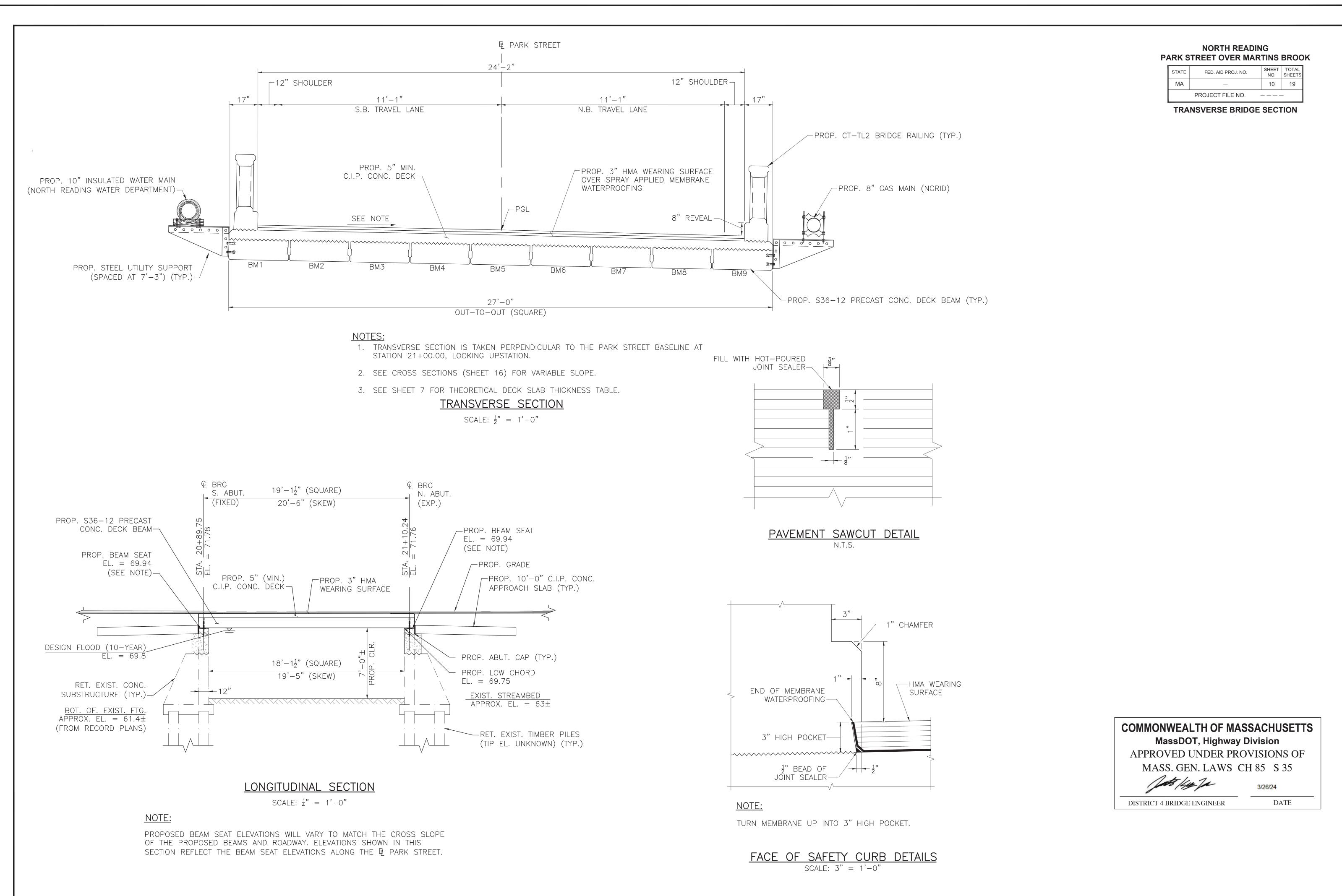
AND SUBSTRUCTURE SHALL BE DESIGNED TO SUPPORT THESE SUPERIMPOSED

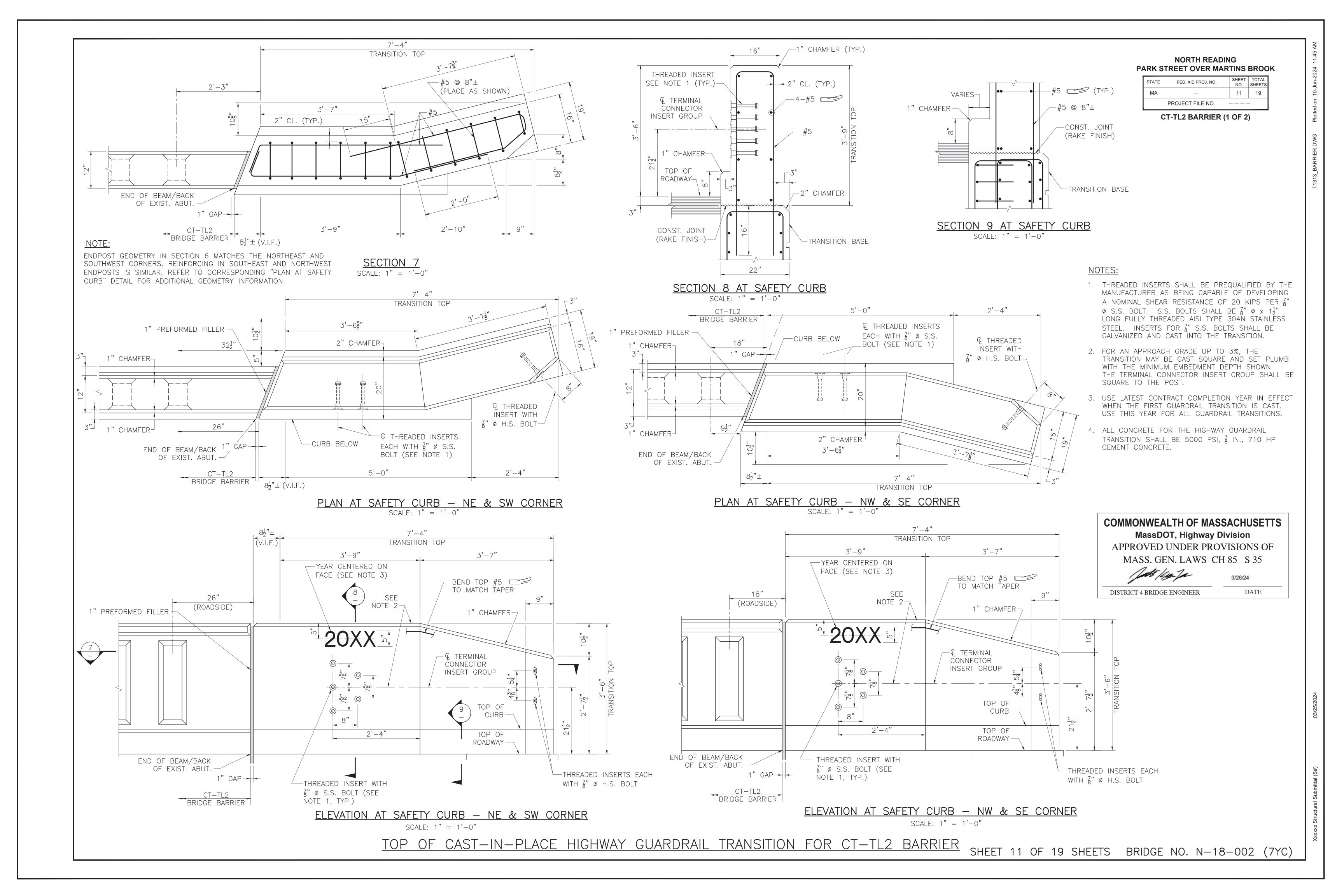
4. TEMPORARY UTILITY RELOCATIONS SHALL OCCUR PRIOR TO DEMOLITION OF THE

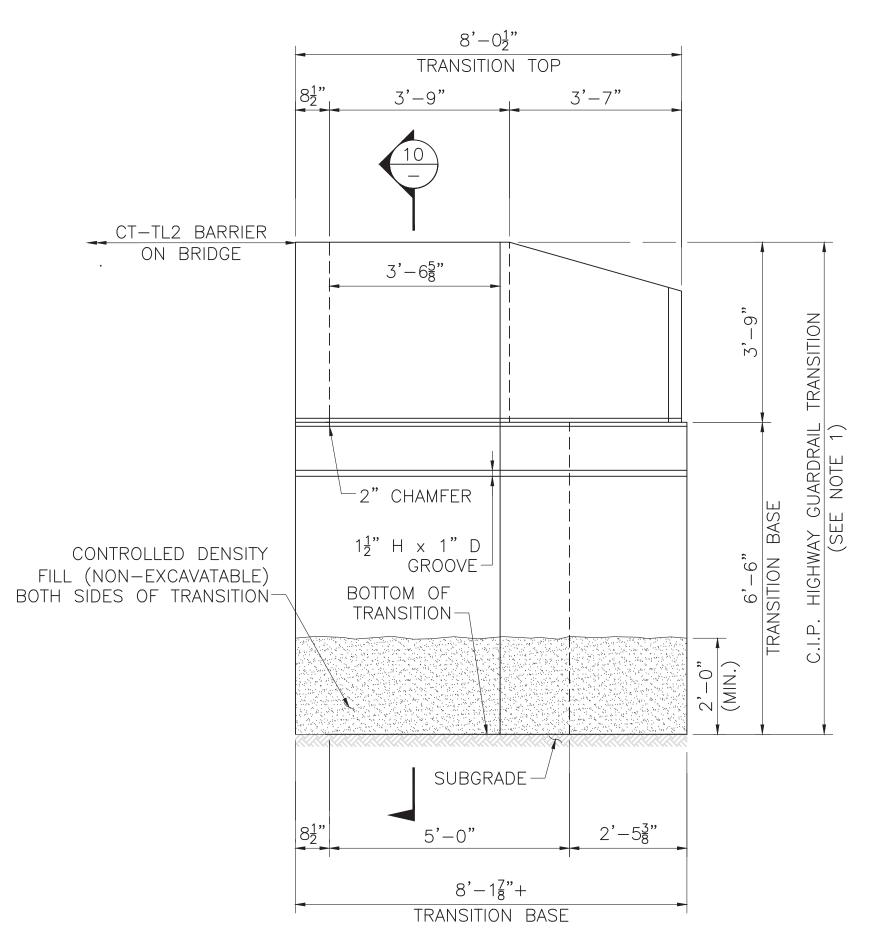
TEMPORARY UTILITY BRIDGE SCHEMATIC

SCALE: N.T.S.

DATE





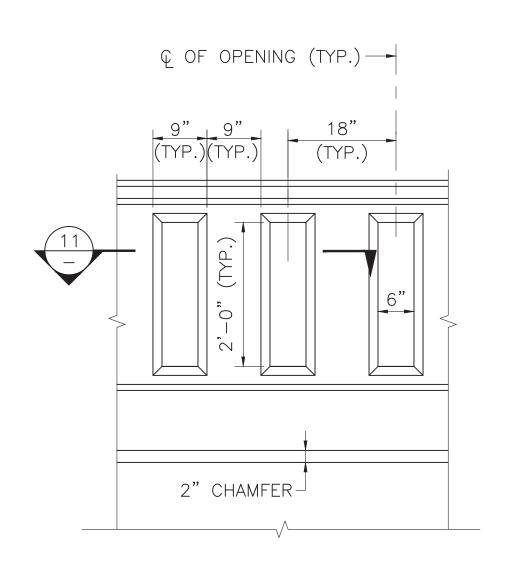


# NOTES:

- 1. CAST-IN-PLACE GUARDRAIL TRANSITION SHALL BE 5000 PSI, 3 IN, 710 HP CEMENT CONCRETE.
- 2. GRAVEL BORROW SHALL BE PLACED AND THOROUGHLY COMPACTED TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE. WHERE NO GRAVEL BORROW IS REQUIRED BELOW THE BASE, IT SHALL BE PLACED ON UNDISTURBED SOIL.
- 3. CONTRACTOR SHALL CAST THE CAST—IN—PLACE GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKILL CAST—IN—PLACE GUARDRAIL TRANSITION WITH CONTROLLED DENSITY FILL (NON—EXCAVATABLE) TO THE ELEVATION SHOWN.

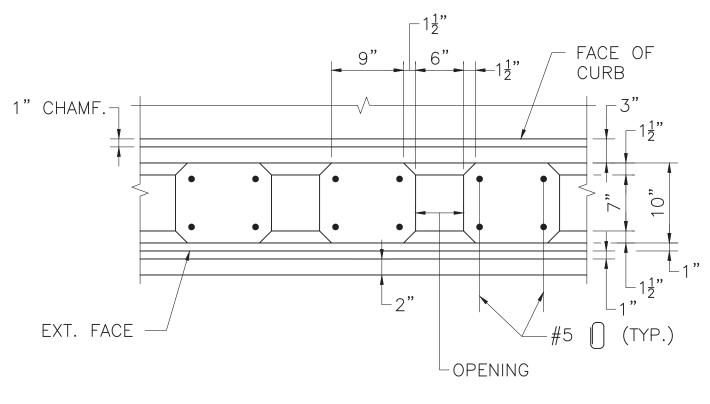
# CAST-IN-PLACE GUARDRAIL TRANSITION ELEVATION

SCALE:  $\frac{1}{2}$ " = 1'-0"

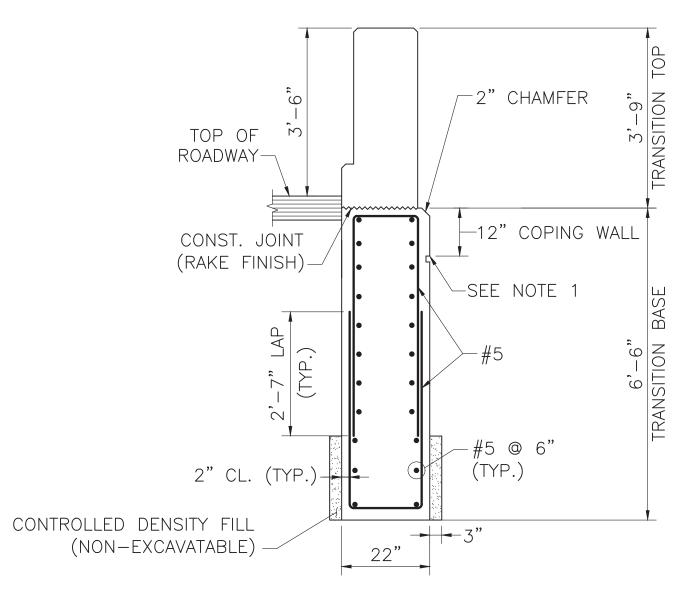


EXTERIOR BARRIER ELEVATION

SCALE: \(\frac{3}{4}\)" = 1'-0"



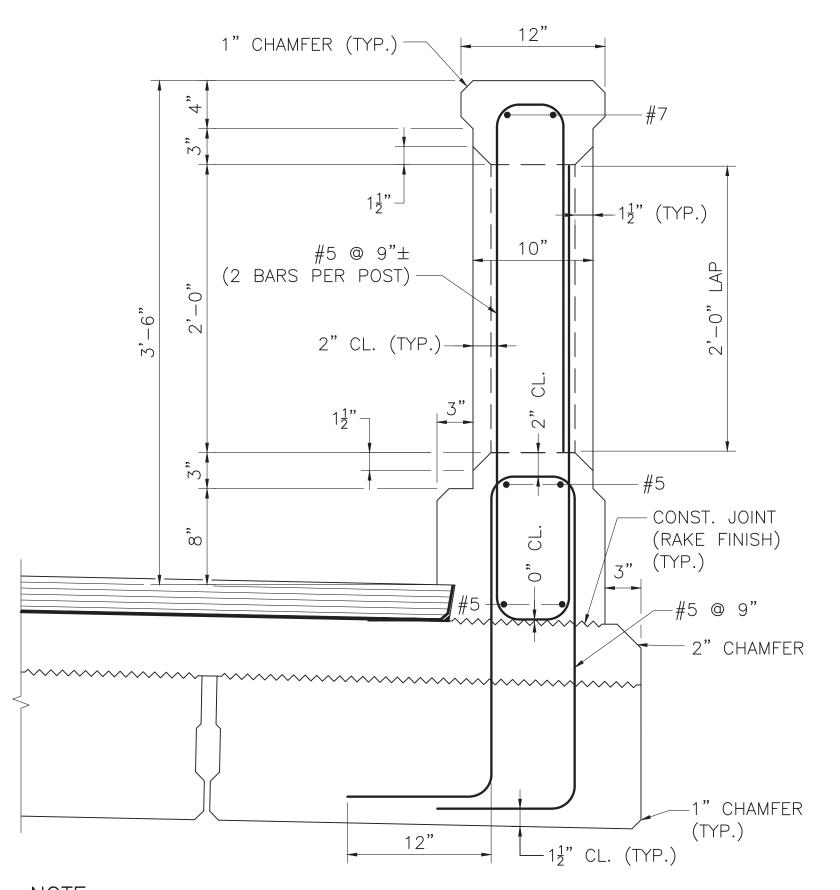
<u>SECTION 11</u> SCALE: 1" = 1'-0"



# NOTES:

- 1.  $1\frac{1}{2}$ " H x 1" D GROOVE.
- 2. REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY.

# SECTION 10 SCALE: $\frac{1}{2}$ " = 1'-0"



NOTE:

DECK AND BEAM REINFORCEMENT NOT SHOWN FOR CLARITY.

SECTION THRU SAFETY CURB SCALE:  $1\frac{1}{2}$ " = 1'-0" COMMONWEALTH OF MASSACHUSETTS

MassDOT, Highway Division

APPROVED UNDER PROVISIONS OF

MASS. GEN. LAWS CH 85 S 35

DISTRICT 4 BRIDGE ENGINEER DATE

SHEET 12 OF 19 SHEETS BRIDGE NO. N-18-002 (7YC)

TUBES CAN BE

PLAN VIEW

TUBES MAY BE

PLACED ON THE

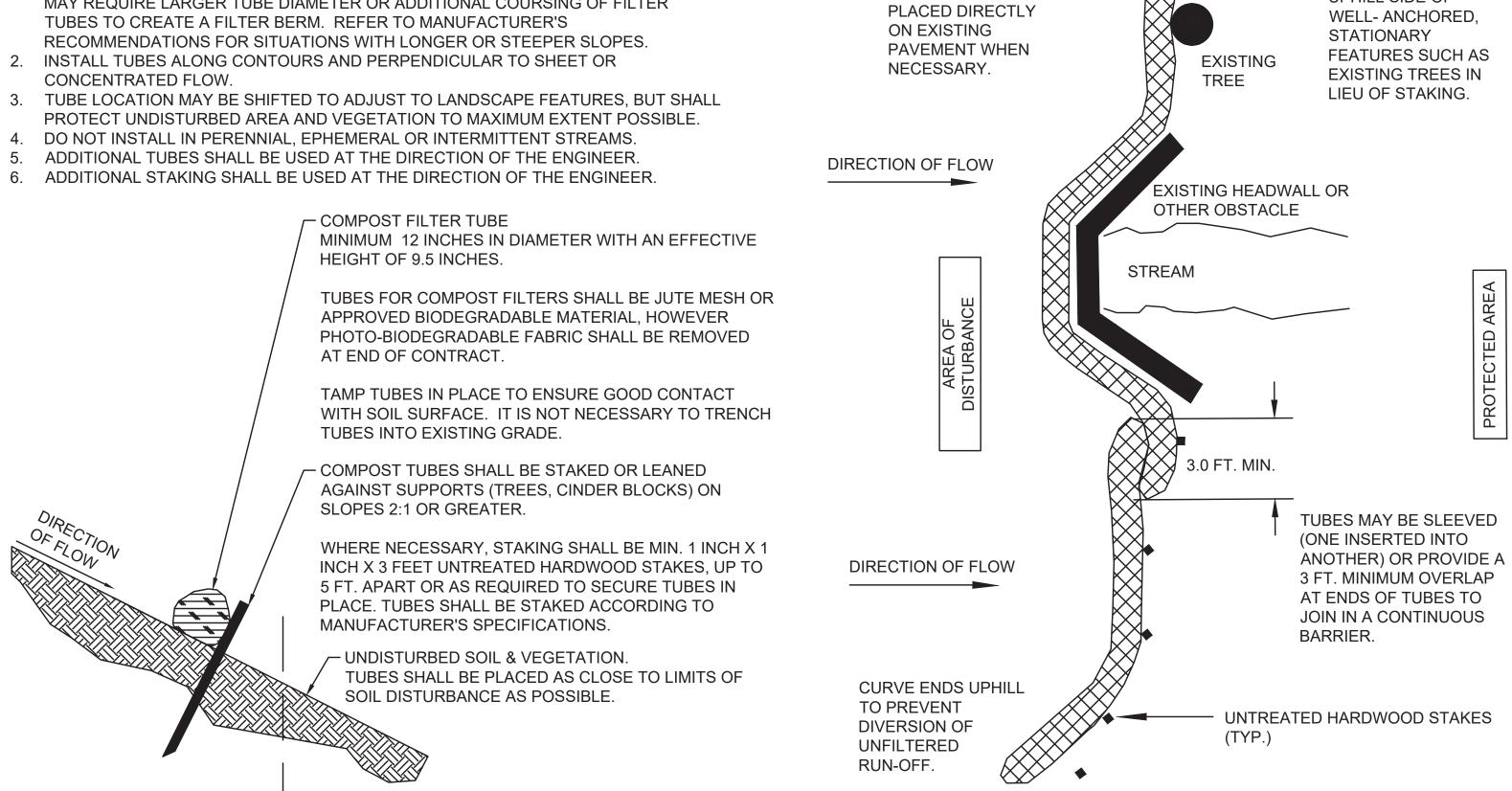
UPHILL SIDE OF

# NOTES:

- 1. PROVIDE A MINIMUM TUBE DIAMETER OF 12 INCHES FOR SLOPES UP TO 50 FEET IN LENGTH WITH A SLOPE RATIO OF 3H:1V OR STEEPER. LONGER SLOPES OF 3H:1V MAY REQUIRE LARGER TUBE DIAMETER OR ADDITIONAL COURSING OF FILTER TUBES TO CREATE A FILTER BERM. REFER TO MANUFACTURER'S RECOMMENDATIONS FOR SITUATIONS WITH LONGER OR STEEPER SLOPES.
- CONCENTRATED FLOW.
- PROTECT UNDISTURBED AREA AND VEGETATION TO MAXIMUM EXTENT POSSIBLE.

LIMIT OF

WORK



COMPOST FILTER TUBE

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS				
MA	_	13	19				
PROJECT FILE NO. ————							

MISC DETAILS

GRANITE CURB IN FULL DEPTH PAVEMENT

SCALE: N.T.S.

**ROADWAY DETAILS** 

# GRANITE CURB IN HMA MILL AND OVERLAY

─8" (MIN.)

SCALE: N.T.S.

SAWCUT-

SURFACE-

(SEE NOTES)-

GRAVEL BORROW-

PROP. HMA OVERLAY-

MILLED PAVEMENT

CEMENT CONCRETE

PROP. GRANITE CURB -TYPE VA4 TAPER PROP. PAVEMENT -PROP. GRANITE MILLING UNDER GUARDRAIL TRANSITION CURB GUTTER LINE — 6'-0" (MIN.) TRANSITION - PAVEMENT MILLING UNDER GUARDRAIL

**GRANITE CURB TRANSITION** 

SCALE: N.T.S.

# NOTES:

TOP OF CURB

-GUTTER LINE

-PROP. MILLING

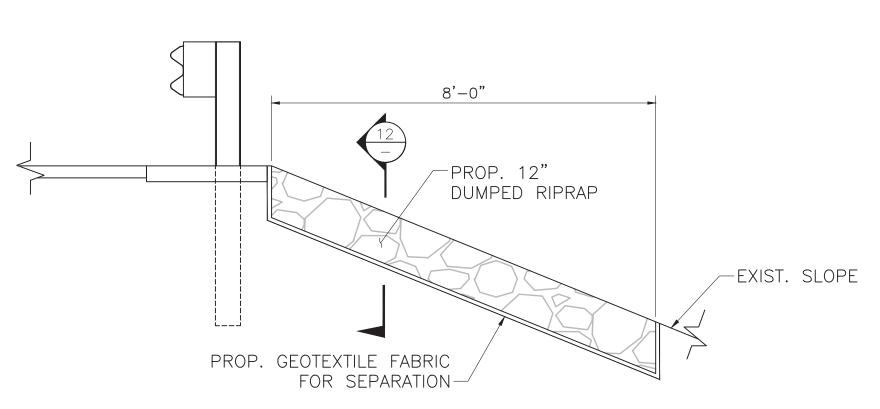
MULCH

-PROP. GRANITE

CURB-TYPE VA4

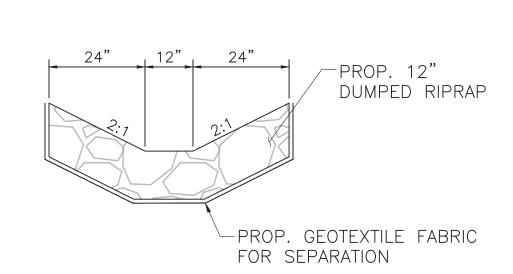
1. CONCRETE SHALL BE INCLUDED IN PRICE BID FOR GRANITE CURB.

- 2. FOR MILL AND OVERLAY DETAIL, SAWCUT 6" AWAY FROM CURBLINE AND REMOVE EXISTING PAVEMENT AND GRAVEL. REPLACE WITH CEMENT CONCRETE.
- 3. ANY DESIGNATED CEMENT CONCRETE THAT IS ACCEPTABLE UNDER SECTION M4 OF THE STANDARD SPECIFICATIONS MAY BE USED. ALL TEST REQUIREMENTS ARE WAIVED. HOT MIX ASPHALT SHALL NOT BE USED AS A SUBSTITUTE.



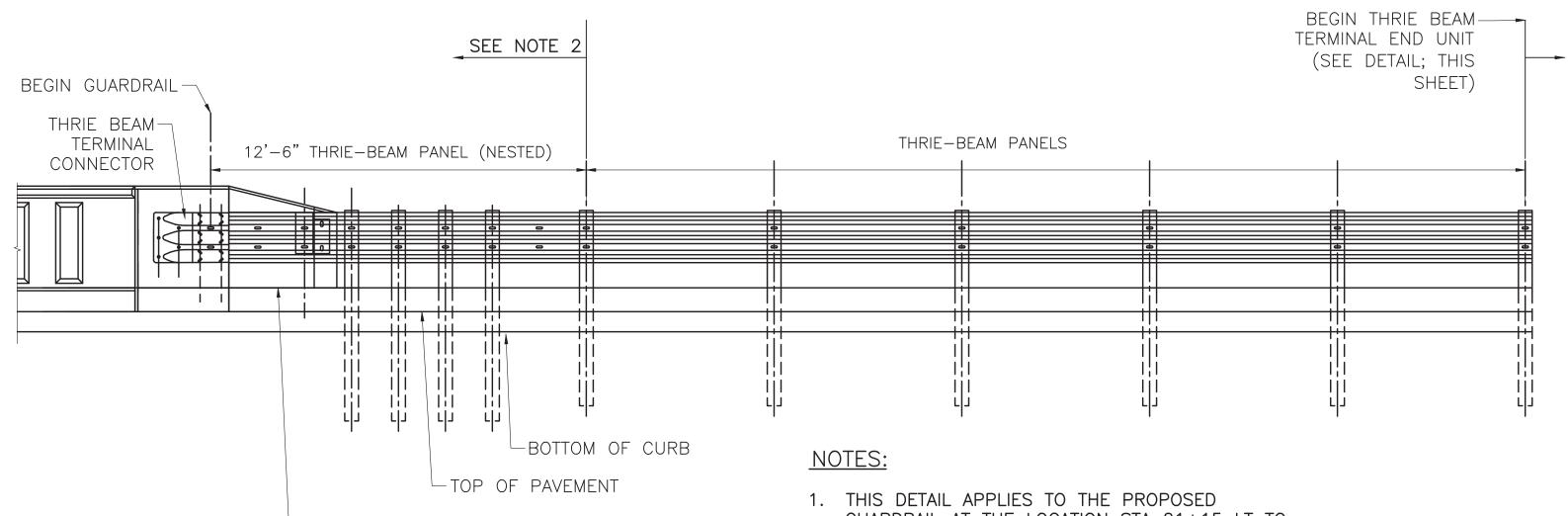
DRAINAGE SWALE DETAIL

SCALE:  $\frac{1}{2}$ " = 1'-0"



SECTION 12

SCALE:  $\frac{1}{2}$ " = 1'-0"

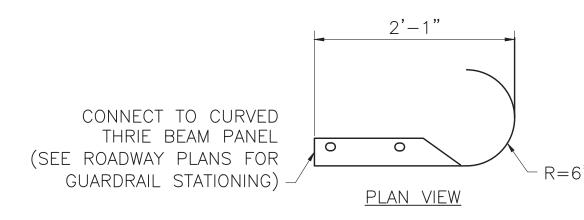


GUARDRAIL AT THE LOCATION STA 21+15 LT TO STA 21+31 LT.

2. REFER TO MASSDOT CONSTRUCTION STANDARD DETAIL 400.3.6 FOR CONSTRUCTION OF NESTED THRIE BEAM, THRIE BEAM TERMINAL CONNECTOR AND ALL HARDWARE ASSOCIATED THEREWITH.

TRANSITION TO BRIDGE RAIL (MODIFIED)

SCALE: N.T.S.



# NOTES:

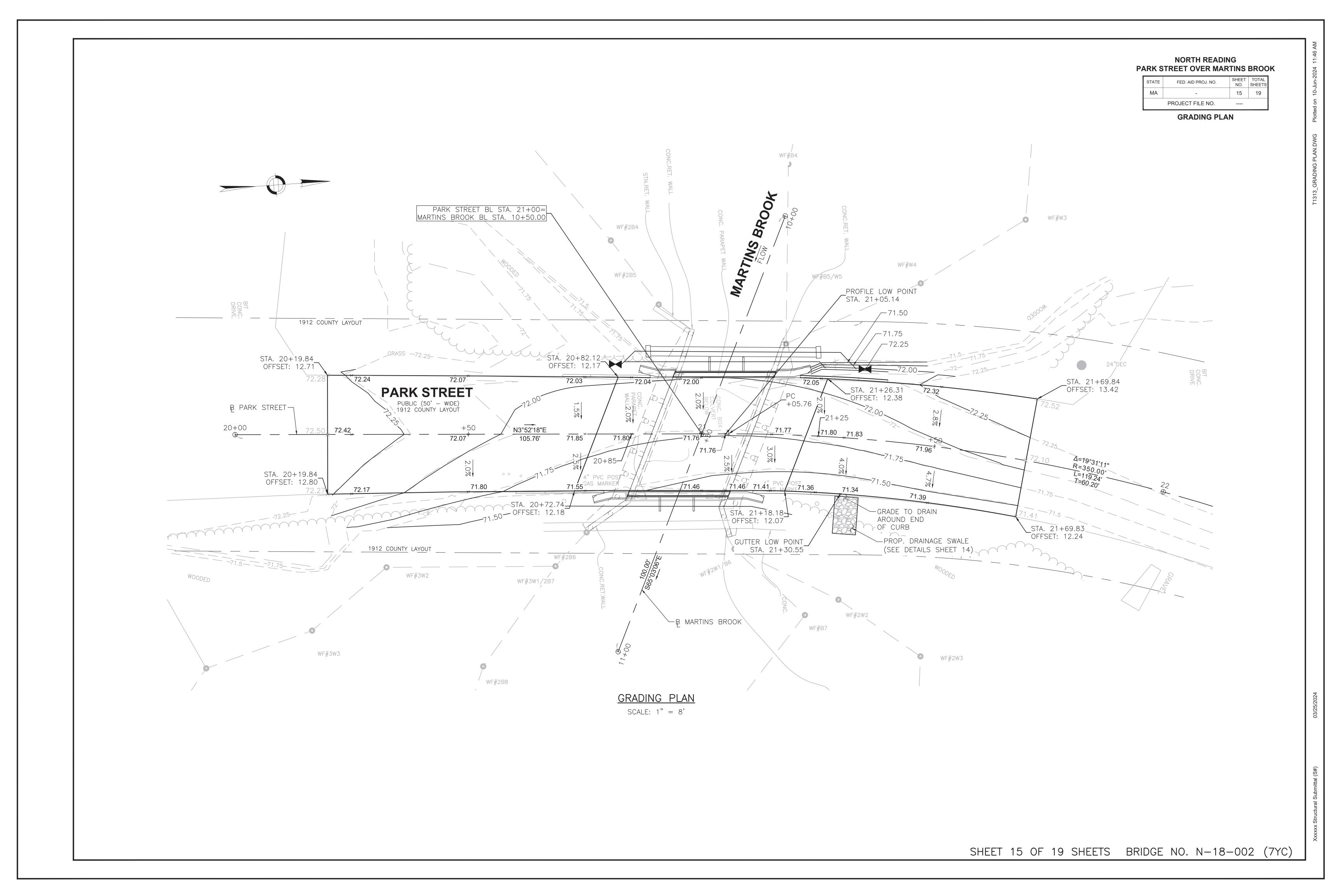
1. SEE MASSDOT CONSTRUCTION STANDARD DETAIL 400.1.3 FOR THRIE BEAM PANEL SECTION.

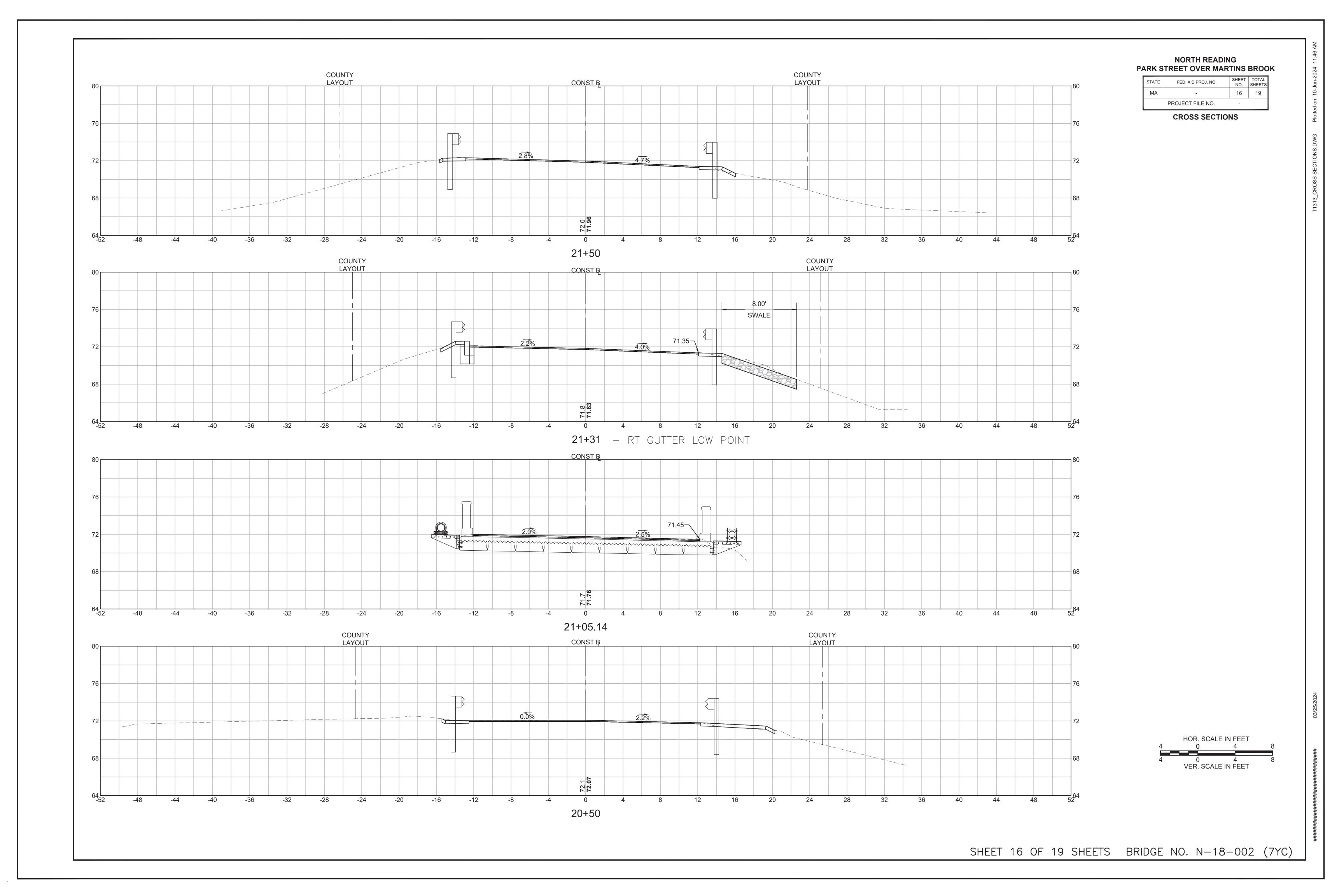
2. THRIE BEAM TERMINAL END UNIT SHALL BE INSTALLED PER MANUFACTURER'S GUIDELINES.

SPLICE BOLT SLOT (TYP.)

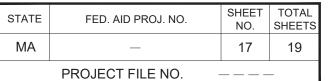
THRIE BEAM TERMINAL END UNIT

SCALE: N.T.S.

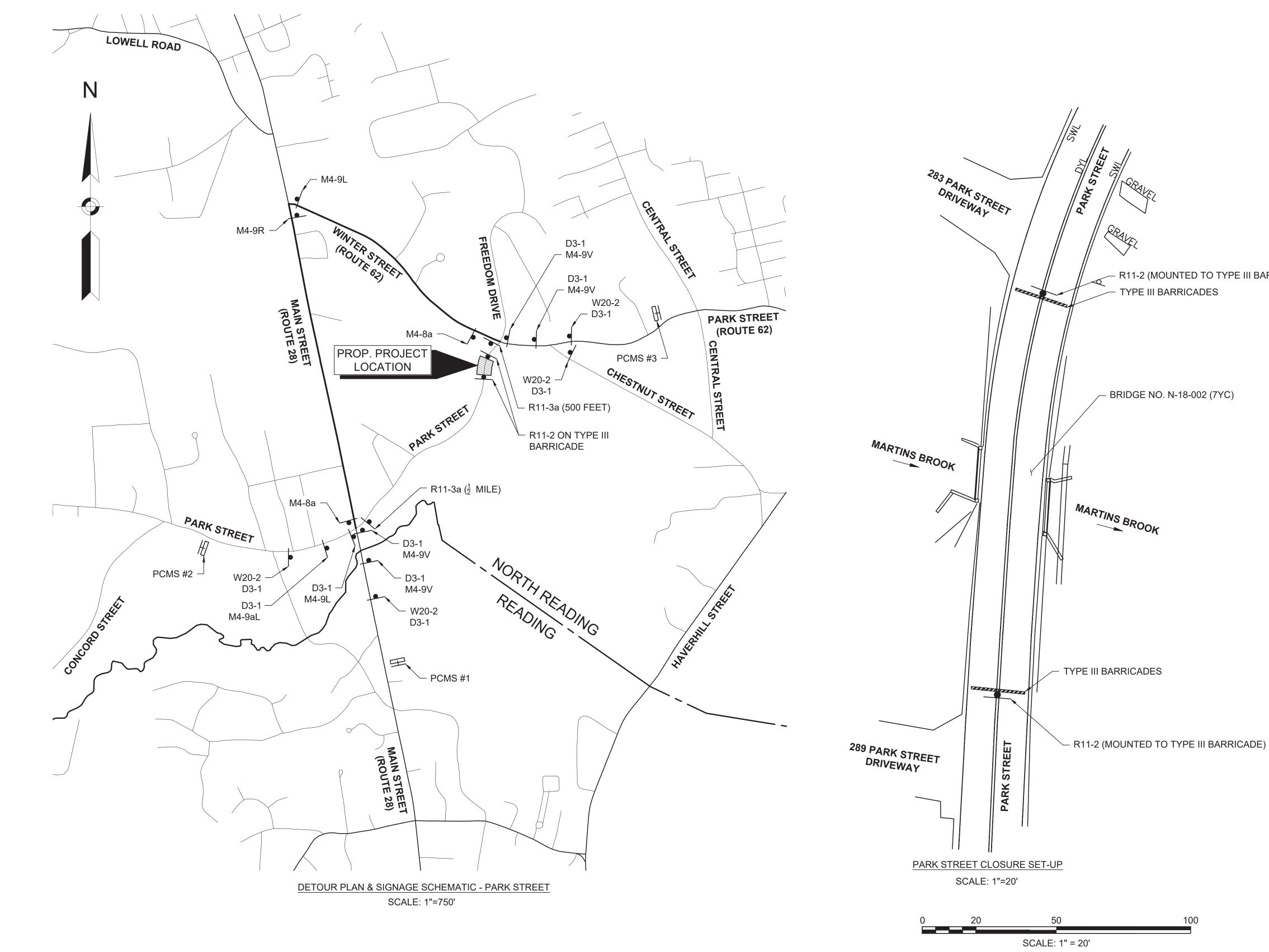








TEMPORARY TRAFFIC CONTROL PLANS SHEET 1 OF 3



# **GENERAL NOTES:**

— R11-2 (MOUNTED TO TYPE III BARRICADE)

TYPE III BARRICADES

- 1. ALL WORK ZONES AND DETOURS ARE ESTABLISHED FOR 24-HOURS A DAY. TEMPORARY CONSTRUCTION SIGNING, BARRICADES, AND ALL OTHER NECESSARY WORK ZONE TRAFFIC CONTROL DEVICES SHALL BE REMOVED FROM THE HIGHWAY OR COVERED WHEN THEY ARE NOT REQUIRED FOR CONTROL OF TRAFFIC.
- 2. ALL TEMPORARY TRAFFIC CONTROL WORK SHALL CONFORM WITH THE 2009 MANUAL ON UNIFORM TRAFFIC CONTROL (M.U.T.C.D.) AND ALL REVISIONS, UNLESS SUPERCEDED BY THESE PLANS.
- 3. ALL SIGN LEGENDS, BORDERS, AND MOUNTING SHALL BE IN ACCORDANCE WITH THE M.U.T.C.D.
- 4. TEMPORARY CONSTRUCTION SIGNING AND ALL OTHER TRAFFIC CONTROL DEVICES SHALL BE IN PLACE PRIOR TO THE START OF ANY WORK.
- 5. SIGNS AND SIGN SUPPORTS LOCATED ON OR NEAR THE TRAVELED WAY MUST PASS THE CRITERIA SET FORTH IN NCHRP REPORT 350, "RECOMMENDED PROCEDURES FOR THE SAFETY PERFORMANCE EVALUATION OF HIGHWAY FEATURES" AND/OR "MANUAL FOR ASSESSING SAFETY HARDWARE" (MASH).
- 6. DISTANCES ARE A GUIDE AND MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER.
- 7. ALL SIGNS SHALL BE MOUNTED ON THEIR OWN STANDARD SIGN SUPPORTS AT THE DISCRETION OF THE ENGINEER.

# TEMPORARY TRAFFIC CONTROL LEGEND

**WORK ZONE** 

TYPE III BARRICADES

PORTABLE CHANGEABLE MESSAGE BOARD

STARTING 14 DAYS PRIOR TO DETOU	JR
	$\downarrow$

PARK ST BRIDGE WORK

STARTS XX-XX FOLLOW DETOUR SIGNS

DURING ALL TIME PERIOD OF DETOURS

PORTABLE CHANGE MESSAGE SIGNS #1, #2 & #3

# NOTES

1. CONTRACTOR TO FURNISH SIGNS CONSISTENT WITH 2009 MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS (OR LATEST EDITION). SEE MANUAL FOR TEXT AND LEGEND DIMENSIONS.

					Т	RAF	FIC SIGN	SUMMARY						
IDENTIFICATION	SIZE OF SIC	GN (INCHES)		TEXT DI	MENSION	NS (IN	CHES)	NUMBER OF		COLOR		NUMBER OF	UNIT	AREA IN
NUMBER	WIDTH	HEIGHT	LEGEND	LETTER HEIGHT	VERTION SPACI		ARROW RTE. MKR.	SIGNS REQUIRED	BACKGROUND	LEGEND	BORDER	SUPPORTS REQUIRED	AREA (S.F.)	SQUARE FEET
R11-2	48	30	ROAD CLOSED		1		2	WHITE	BLACK	BLACK	0 ON TYPE III BARRICADE	10.00	20.00	
R11-3a( 500 FT)	60	30	ROAD CLOSED 500 FT AHEAD LOCAL TRAFFIC ONLY					1	WHITE	BLACK	BLACK	1	12.50	12.50
R11-3a(½ MI)	60	30	ROAD CLOSED  1 MILE AHEAD LOCAL TRAFFIC ONLY					1	WHITE	BLACK	BLACK	1	12.50	12.50
W20-2	36	36	DETOUR AHEAD					4	FL. ORANGE	BLACK	BLACK	0 W/ D3-1	9.00	36.00
M4-8a	24	18	END DETOUR					2	FL. ORANGE	BLACK	BLACK	2	3.00	6.00
M4-9L	30	24	DETOUR					2	WHITE	BLACK	BLACK	1 1 W/ D3-1	5.00	10.00
M4-9R	30	24	DETOUR					1	WHITE	BLACK	BLACK	1	5.00	5.00
M4-9aL	30	24	DETOUR					1	WHITE	BLACK	BLACK	0 W/ D3-1	5.00	5.00
M4-9V	30	24	DETOUR 1		V			4	WHITE	BLACK	BLACK	0 W/ D3-1	5.00	20.00
D3-1	35	12	Park st	6D / 4D	2.75 3.25	5	NA	10	FL. ORANGE	BLACK	BLACK	10	2.92	29.17

<b>)</b>	ARK S	NORTH READIN TREET OVER MAR	_	BROC	K
	STATE	FED AID PROJ NO	SHEET	TOTAL	

MA         —         19         19           PROJECT FILE NO.         ————	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
PROJECT FILE NO. ————	MA	_	19	19	
		PROJECT FILE NO.		_	

TEMPORARY TRAFFIC CONTROL PLANS
SHEET 3 OF 3

																								1
SEQUENCE AND	TIMIN	IG CH	ART F	OR F	ULLY-	ACTU	ATEC	TRAF	FICS	SIGNA	L CON	ITROL	. [ISOL	ATEC	) INTE	ERSE	CTION]							
		Ø1			Ø2			Ø3			Ø4			Ø5			Ø6		Ø7			Ø8		
MAIN STREET (ROUTE 28) AT PARK STREET (NORTH READING, MASSACHUSETTS)			Y		HI H	Y				PM		Y	NO	T USI	ED		The second secon			Y	7		Y PM	
APPROACH DIRECTION HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17 18	19	20	21	22	23	24	FLASHING OPERATION
				1.0					Ī							1.0								T
MINIMUM INTERVAL	5			10						8						10		5			6			
VEHICLE EXTENSION	3			3						3						3		3			3			
MAXIMUM 1	15			35						30						35		20			10			] =
MAXIMUM 2	10			30						40						30		25			10			Ō
YELLOW CLEARANCE		3.5			4.5						4.0						4.5		4.0			4.0		O O
RED CLEARANCE			2.0			2.0						3.0					2.0			3.0			3.0	L L
WALK							8.0																	I R
PEDESTRIAN CLEARANCE								12.0																EMERGENCY ONLY
RECALL		OFF			MIN			OFF			OFF						MIN	OFF			OFF			
DETECTOR	NO	DN-LO	CK	NC	N-LO	CK		LOCK		NC	N-LO	CK				NC	N-LOCK	NC	DN-LO	CK	NC	N-LO	CK	

	SEQU	JENCE AND TI	MING	CHAF	RT FO	R FUL	LY-A	CTUAT	ΓED T	RAFF	IC SIG	SNAL (	CONTI	ROL [I	NACT	IVATE	D CO	ORDI	VATIC	N]							
				Ø1			Ø2			Ø3			Ø4			Ø5			Ø6			Ø7			Ø8		
MAIN STREET (ROUTE 28) A STREET (ROUTE 6 (NORTH READING, MASSAC	62)	N			<u> </u>										NC	)T USI	ΞD		I PA		NC	OT US	ED	NO	OT US	ED	
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASHING OPERATIO
MINIMUM INTERVAL			6			6						6						6									
VEHICLE EXTENSION			2			2						2						2									
MAXIMUM 1			10			30						30						30								ONLY	
MAXIMUM 2			20			40						20						40									0
YELLOW CLEARANCE				4.0			5.0						3.0						5.0								
RED CLEARANCE					2.0			1.0						1.0						1.0							
WALK									5.0																		
PEDESTRIAN CLEARANCE										25.0																	EMERGENC
RECALL				OFF			MIN			OFF			OFF						MIN								] "
DETECTOR			NO	ON-LO	CK	NC	N-LO	CK		LOCK		NC	N-LO	CK				NO	N-LO	CK							

SEQUENCE A	ND TIMII	NG CHAF	RT FO	R FU	JLLY-A	ACTU.	ATED	TRAF	FIC S	IGNA	_ CON	TROL	[ISOL	ATEC	INTE	RSEC	CTION	]							
		Ø1			Ø2			Ø3			Ø4			Ø5			Ø6			Ø7			Ø8		
WINTER STREET (ROUTE 62) AT PARK STREET (ROUTE 62) / FREEDOM DRIVE (NORTH READING, MASSACHUSETTS)		FOLA		PM			NC	T US	ED		PN		NO	T USI	ED	<u> </u>		PM	NOT	USI	ED		PM		
APPROACH DIRECTION HOUSIN	G 1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASHING OPERATION
MINIMUM INTERVAL	5			6						5						6						5			
VEHICLE EXTENSION	2			3						2						3						2			
MAXIMUM 1	22			35						10						35						10			ONLY
MAXIMUM 2	22			35						10						35						10			Ō
YELLOW CLEARANCE		4.0			4.0						3.0						4.0						3.0		0
RED CLEARANCE			1.0			1.0						1.0						1.0						1.0	
WALK	6.0																								1 BB
	16.0	4.0	1.0																						EMERGENC
PEDESTRIAN CLEARANCE									-																1 "
PEDESTRIAN CLEARANCE RECALL		OFF			MIN						OFF						MIN						OFF		

# SEQUENCE & TIMING NOTES:

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1. AUTOMATIC FLASHING OPERATION PER M.U.T.C.D. SECTION 4D.12.

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5. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT

6. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE

7. THE RIGHT-OF-WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF

8. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT-OF-WAY SHALL BE IN

DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT

2. PEDESTRIAN PHASE UPON PUSH BUTTON ACTIVATION ONLY.

WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.

TRAFFIC MOVEMENT WILL NOT CHANGE DURING

9. DEACTIVATE DUAL ENTRY FOR PHASES 4 AND 8

3. PM = PERMITTED, Y = YIELD, OL = OVERLAP

4. MAXIMUM 1 = M-F 6:00 AM TO 10:00 AM MAXIMUM 2 = FREE OPERATION

THE CLEARANCE INTERVAL.

NON-CONFLICTING PHASES.

- 2. PEDESTRIAN PHASE UPON PUSH BUTTON ACTIVATION ONLY.
- 3. PM = PERMITTED
- 4. MAXIMUM 1 = FREE OPERATION
- MAXIMUM 2 = M-F 3:00 PM TO 7:00 PM
- 5. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
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