

LS

PROJECT FILE NO.

N/A

<u>DESIGN:</u>

IN ACCORDANCE WITH THE 2020 (9TH EDITION) AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS FOR HL-93 LOADING.

BENCH MARK:

BENCH MARK 1: TBM#1 NAIL IN 14" OAK N=3118121.019, E=808974.433, EL.=22.17

BENCH MARK 2: TBM#2

SPIKE IN 5" CEDAR N=3118215.642, E=808713.356, EL.=18.40

ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

SURVEY:

AN INSTRUMENT FIELD SURVEY WAS PERFORMED BY BSC GROUP IN SEPTEMBER 2018. THE COORDINATES, IN FEET, ARE BASED UPON THE NORTH AMERICAN DATUM OF 1983 (NAD 83)

DATE:

SCALES:

TO BE PLACED ON THE INSIDE FACE OF THE NORTHEASTERLY AND SOUTHWESTERLY HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING THE SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF THE CONTRACT COMPLETION AS OF THE DATE THE BRIDGE IS CONSTRUCTED. BOTH ENDS OF THE BRIDGE SHALL FEATURE THE SAME DATE. SEE SHEET 6 OF 20.

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZED PRINTS (A3).

FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

ANCHOR BOLTS:

ALL ANCHOR BOLTS SHALL BE SET BY A TEMPLATE BEFORE THE CONCRETE IS PLACED. (SEE SHEETS 19 & 20)

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWING, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MO	DIFICATION CONDITION	<u>#4 BARS</u>	#5 BARS	#6 BARS	#7 BARS	#8 BARS
1.	NONE	16"	19"	23"	33"	38"
2.	12" OF CONCRETE BELOW BAR	20"	25"	30"	43"	49"
3.	COATED BARS, COVER < 3dь, OR	23"	29"	34"	50"	57"
	CLEAR SPACING < 6db					
4.	COATED BARS, ALL OTHER CASES	18"	23"	27"	40"	46"
5.	CONDITION 2. AND 3.	26"	32"	39"	56"	64"
6.	CONDITION 2. AND 4.	24"	30"	36"	52"	59"
4. 5.	CLEAR SPACING < 6db COATED BARS, ALL OTHER CASES CONDITION 2. AND 3.	18" 26"	23" 32"	27" 39"	40" 56"	46" 64"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

ALL REINFORCEMENT SHALL BE EPOXY COATED.

MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING (SPRAY-APPLIED) FOR NEW BRIDGE DECKS.

<u>CONCRETE:</u>

CONCRETE TYPE

CAST IN PLACE 4000 PSI, 1.5 INCH, 565 CEMENT CONCRETE LOWER INTEGRAL ABUTMENTS & WINGWALLS 4000 PSI, \frac{3}{4} INCH, 585 HP CEMENT CONCRETE DECK SLAB, UPPER INTEGRAL ABUTMENTS & WINGWALLS 4000 PSI, \(\frac{3}{4} \) INCH, 610 CEMENT CONCRETE BEAM PEDESTALS

APPLICATION

5000 PSI, \(\frac{3}{4}\) INCH, 685 HP CEMENT CONCRETE SAFETY CURB & SIDEWALK

5000 PSI, \(\frac{3}{4} \) INCH, 685 HP CEMENT CONCRETE GUARDRAIL TRANSITIONS

4000 PSI, 1.5 INCH, 565 CEMENT CONCRETE APPROACH SLABS

* SEE SHEET 11 OF 20 FOR PRESTRESSED B36-24 BEAM DETAILS.

STRUCTURAL STEEL:

PILES SHALL CONFORM TO AASHTO M270 GRADE 50.

EXISTING CONDITIONS:

ALL DIMENSIONS AND DETAILS SHOWN FOR THE EXISTING STRUCTURE ARE NOT GUARANTEED TO BE CORRECT. MASSDOT, THE CITY OF NEWBURYPORT AND TOWN OF WEST NEWBURY DO NOT HAVE ANY EXISTING PLANS OF THE STRUCTURE. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND DETAILS NECESSARY FOR THE COMPLETION OF ALL WORK BY FIELD MEASUREMENT AND SURVEY.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURACY AND ADEQUACY THEREOF AND SHALL NOT COMMENCE ANY FABRICATION UNTIL THEY HAVE MADE THE REQUIRED MEASUREMENTS ON THE ACTUAL STRUCTURE AND THE SUBMITTED SHOP DRAWINGS HAVE BEEN APPROVED BY THE ENGINEER. SHOP DRAWINGS SHALL STATE THAT THE EXISTING DIMENSIONS, ANGLES, ELEVATIONS AND FIELD CONDITIONS HAVE BEEN FIELD VERIFIED BY THE CONTRACTOR.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS REQUIRED FOR THE PROPER PERFORMANCE OF THE WORK. FIELD CONDITIONS MAY EXIST, WHICH DEVIATE FROM THE TYPICAL WORK AND THEORETICAL DIMENSION SHOWN ON THE PLANS. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR FABRICATION AND FIT OF THEIR WORK.

TRAFFIC NOTES:

THE BRIDGE IS CURRENTLY CLOSED WITH A DETOUR IN PLACE. THE BRIDGE WILL REMAIN CLOSED FOR THE DURATION OF BRIDGE CONSTRUCTION.

UTILITIES:

THE CONTRACTOR SHALL PROTECT THE RESERVOIR FROM DAMAGE/CONTAMINATION. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE WATER DEPARTMENT. THERE ARE NO KNOWN UTILITIES ABOVE GROUND OR BELOW AT THE SITE LOCATION. THE CONTRACTOR SHALL CONTACT DIG-SAFE BEFORE WORK BEGINS.

CONSTRUCTION:

ANY PERMIT MODIFICATIONS REQUIRED DUE TO THE CONTRACTOR'S MEANS AND METHODS WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL MODIFY ALL REQUIRED PERMITS AND LICENSES AND PAY ALL CHARGES AND FEES INCURRED. THE CONTRACTOR SHALL GIVE ALL NOTICES NECESSARY AND INCIDENT TO THE DUE AND LAWFUL PROSECUTION OF THE WORK, AND SHALL COMPLY WITH ALL LAWS, ORDINANCES, RULES, AND REGULATIONS OF THE FEDERAL GOVERNMENT, THE STATE, THE TOWN/CITY, AND OTHER BODIES HAVING JURISDICTION OVER THE WORK AND ENCOMPASSED BY THE CONTRACT. THE COMPLETION DATE WILL REMAIN AS STATED IN THE CONTRACT DOCUMENTS.

AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.

CONSTRUCTION JOINTS:

CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT THE APPROVAL OF BSC GROUP.

	ESTIMATED QUANTITIES			
	(NOT GUARANTEED)			
ITEM NO.	DESCRIPTION		<u>QUANTITY</u>	<u>UNIT</u>
115.1	DEMOLITION OF BRIDGE NO. N-11-007 = W-20-001		1	LS
140.	BRIDGE EXCAVATION		570	CY
143.	CHANNEL EXCAVATION	390 /1	> 85	CY
144.	CLASS B ROCK EXCAVATION		90	CY
151.1	GRAVEL BORROW FOR BRIDGE FOUNDATION		290	CY
151.2	GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES		35	CY
153.1	CONTROLLED DENSITY FILL - NON-EXCAVATABLE	14	CY	
156.13	CRUSHED STONE FOR INTEGRAL ABUTMENT PILES	35	TON	
156.5	CRUSHED STONE FOR FILTER BLANKET	D 85	CY	
450.60	SUPERPAVE BRIDGE SURFACE COURSE - 9.5 (SSC-B-9.5)		15	TON
450.70	SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 (SPC-B-9.5)		20	TON
698.4	GEOTEXTILE FABRIC FOR PERMANENT EROSION CONTROL	390 /1	345	SY
942.124	STEEL PILE HP 12 X 84	380 /1	380	FT
948.41	DYNAMIC LOAD TEST BY CONTRACTOR		2	EA
948.5	PILE SHOES		10	EA
983.12	RIPRAP WITH GRAVEL PACKED VOIDS	440 1	> 500	TON
983.521	STREAMBED/BANK RESTORATION	86 1	DK(CY
991.1	CONTROL OF WATER - STRUCTURE NO. N-11-001 = W-20-001		1	LS
994.01	TEMPORARY PROTECTIVE SHIELDING BRIDGE NO. N-11-001 = W-	-20-001	1	LS
225	DDIDOE OFDIGOE DDIDOE NO. N. 44 OOA			

GENERAL NOTE:

THE CONTRACTOR MUST COORDINATE ALL WORK WITH THE CITY OF NEWBURYPORT, THE TOWN OF WEST NEWBURY, THE WATER DEPARTMENT, THE ENGINEER, AND ANY AFFECTED ABUTTERS. WORK SHALL NOT PROCEED WITHOUT WRITTEN APPROVAL FROM THE CITY OF NEWBURYPORT AND THE TOWN OF WEST NEWBURY.

995.01 BRIDGE STRUCTURE, BRIDGE NO. N-11-001 = W-20-001

SECTION MARK:

DIRECTION OF SECTION VIEW-

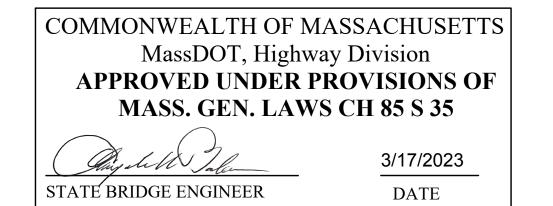
SHEET NUMBER WHERE THE SECTION CAN BE FOUND, NO SHEET NUMBER IF IT IS ON THE SAME SHEET. INSTEAD "-" IS DENOTED.

-SECTION #

TRAFF	TC DATA	
	ROADWAY OVER	ROADWAY UNDER
ESIGN YEAR	2039	
/ERAGE DAILY TRAFFIC — PRESENT	920	
/ERAGE DAILY TRAFFIC — DESIGN YEAR	1120	
ESIGN HOURLY VOLUME	125	
RECTIONAL DISTRIBUTION	53% WB	X
RUCK PERCENTAGE — AVERAGE DAY	2.9%	
RUCK PERCENTAGE — PEAK HOUR	1.6%	
ESIGN SPEED	30 MPH	
RECTIONAL DESIGN HOURLY VOLUME	65	

SEISMIC DESIGN CRITERIA										
DESIGN RETURN PERIOD:	1000 YEARS									
DESIGN SPECTRA										
As	0.149									
SD_S	0.290									
SD1	0.101									
SITE CLASS	D									
SEISMIC DESIGN CATEGORY (SDC)	А									

HYDRAULIC	DESIGN DATA			
DRAINAGE AREA:	5.60	SQ. MILES		
DESIGN FLOOD DISCHARGE:	80.0	C.F.S.		
DESIGN FLOOD FREQUENCY:	10	YEARS		
DESIGN FLOOD VELOCITY:	0.32	F.P.S.		
DESIGN FLOOD ELEVATION:	12.60	FEET, NAVD		
BASE (100-YE	AR) FLOOD DATA	1		
BASE FLOOD DISCHARGE:	240	C.F.S.		
BASE FLOOD ELEVATION:	13.00	FEET, NAVD		
DESIGN AND CH	IECK SCOUR DAT	A		
DESIGN SCOUR FLOOD EVENT RETURN FREQUENCY:	25	YEARS		
DESIGN FLOOD ABUTMENT SCOUR DEPTH:	0.00	FEET		
CHECK SCOUR FLOOD EVENT RETURN FREQUENCY:	50	YEARS		
CHECK FLOOD ABUTMENT SCOUR DEPTH:	0.00	FEET		
FLOOD (F RECORD			
DISCHARGE:	N/A	C.F.S.		
FREQUENCY:	N/A	YEARS		
MAXIMUM ELEVATION:	N/A	FEET, NAVD		
DATE:	N/A	MONTH, YEAR		
HISTORY OF ICE FLOES:	N/A	N.A.		
EVIDENCE OF SCOUR AND EROSION:	N/A	N.A.		
TEMPORARY WATER	Control design	I DATA		
DESIGN FLOOD DISCHARGE:	50	C.F.S.		
DESIGN FLOOD FREQUENCY:	5	YEARS		
DESIGN FLOOD VELOCITY:	0.93	(F.P.S.)		
DESIGN FLOOD ELEVATION:	12.63	FEET, NAVD		



ISSUED FOR CONSTRUCTION MONTH DD, YYYY DESCRIPTION DATE USE ONLY PRINTS OF LATEST DATE

SHEET 2 OF 20 SHEETS BRIDGE NO. N-11-007 = W-20-001 (CBR)

ATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
1A	-	21	42
	PROJECT FILE NO.	N/A	

BORING LOG BB-1

GEOSCIENCES TESTING AND RESEARCH, INC.

55 Middlesex Street, Suite 225, North Chelmsford, MA. Phone: (978) 251-9395 www.gtrinc.net

Newburyport, MA

Sample Data

Boring No. BB-1
Page: 1 of 2
GTR Job #: 22.219
GTR Rep: C. George
Reviewer: C. George

Stratum

Car-Dee Corporation Steve DeSimone Helper(s): Joe/Frank Depth (ft) Casing | Sampler Equipment 2/5/2019 Date | Time | Water | Casing | Hole 2/4/2019 End Date: Type ~ 17.7' Gnd Surface Elev (ft): 1.75" 2/5 | 7am | 5.5' | 45' 2.16" Size I.D. 300 lb 140 lb 808857.91 E , 3118163.09 N Hammer Wt. Location: Truck mounted Deidrich D50 with Automatic Hammer 30 in 30 in Hammer Fall

	Ŧ	8				Jui	Julian	Additional	à		
	Dept	Case	No.	Pen/ Rcvy	Depth (ft)	Blows per 6in	Field Test	Description and Classification	Description	Data	Note
			SS-1A	24/16	0.7-1.7	59-20		S-1A,Dry, brown, dense, fine SAND, little	ASPHALT		1,2
			SS-1B		1.7-2.7	14-8		Silt, trace Gravel	8"		
<u> </u>								S-1B, Dry, brown, Hard, Clayey SILT,			
					1			trace fine Sand			
	-5										
	· ·		S-2	24/15	5-7	2-3		Wet, Brown, very loose, fine to medium			
)			· /		1	1 for 12"		SAND, little Clayey Silt, trace Gravel			
		-	-					-			
	10								FILL		
	10000		SS-3	24/14	10-12	3-2		Wet, Brown, very loose, fine to medium			
						1 for 12"		SAND, trace Silt, trace Gravel			
					3						
	15		SS-4	24/12	15-17	20-7		Wet, Brown, loose, fine to medium SAND,			
		_	33.4	24/12	1517	3-7		some Gravel			
<u> </u>											
<u> </u>									18.5'		
	20		SS-5	24/20	20-22	3-2		Gray, medium stiff, CLAY & SILT			
						3-3			CLAY		
0											
	25								23.5'		
	25		SS-6	24/12	25-27	4-4		Gray, loose, fine to medium SAND, little			
00						5-7		Silt, trace Gravel			
									SAND		
								- 1			
	NOTE	200	2. 03		L	— CONTII	NUATION	*#		4)	
	1. W	ater le	vel checke	d prior to	starting the	second day	of drilling	g.			
	2. Th	e upp	er 15 feet	of the bori	ing was com	pleted with	4-inch ho	ollow stem augers.			
	Orde	er of Sa	ample Des	cription (Modified Burn	nister)		PENETRATION RESIST	TANCE (N) GUID	E	
			Content: Dry					Cohesionless Soils (Sands)		sive Soils (Clays)	
	2. Sc	oil Relati	ve Density of	r Consistency	/			Relative Density / Blows per Foot	Consiste	ncy / Blows per Foot	

CONTINUATION — ¬

Order of Sample Description (Modified Burmister)

Minor Component: "and" - 35% to 50% minor grain size

"some" - 20% to 35% minor grain size

"little" - 10% to 20% minor grain size

"trace" - < 10% of minor grain size

Moisture Content: Dry, Moist, Wet

2. Soil Relative Density or Consistency

4. Major Component: Should be capitalized

ţ	BPF		Sample Data						Stratum Additional		
Depth	Case	No.	Pen/ Rcvy	Depth (ft)	per 6in	Field Test Results	Description and Classification	Description	Data	Notes	
30		SS-7A	24/14	30-31	10-7		SS-7A, Gray, medium dense, fine to	SAND	Ï		1
30		SS-7B		31-32	17-12		medium SAND, little Gravel, trace Silt	31'			
							SS-7B, Gray, medium dense, fine to				
							medium SAND, some Clayey Silt, some				
							Gravel				
35	-	S-8	24/12	35-37	17-24		Gray, very dense, GRAVEL, some fine to				
	-	3-0	24/12	33-37	36-31		coarse Sand, trace Clayey Silt				
	-	-		1	30-31		l coarse sand, trace clayey sin	GLACIAL TILL			
			7			Š.		OLACIAL HEL			
40											
40		SS-9	24/10	40-42	37-37		Gray, very dense, fine to medium SAND,				
					45-51		little Gravel, little Silt				
										-	APP
45		55.10	2/0	45.45.2	100 f 2"					3	PILE EL.
	-	SS-10	2/0	45-45.2	100 for 2"		No Recovery	44'			
	-	C-1	60/51	45.2-50.2	5:45 min 6:30 min		Gray, slightly to moderately weathered,				
	-			 	6:45 min		moderately to highly fractured, medium grained, moderately hard, TONALITE/				
	_			1	6:15 min		GRANODIORITE				
50	-				5:45 min		CHANODIONITE	BEDROCK			
		C-2	60/54	50.2-55.2	4:45 min		Gray, slightly to moderately weathered,				
					4:00 min	-	moderately to highly fractured, medium				
					6:00 min		grained, moderately hard, TONALITE/				
55					6:30 min		GRANODIORITE				
22					6:30 min						
							Bottom of boring at 55.2 feet below	55.2'			
			1				ground surface with 10 foot rock core.				

Soil Relative Density or Consistency
 Color

4. Major Component: Should be capitalized
5. Minor Component: "and" - 35% to 50% minor grain size
"some" - 20% to 35% minor grain size
"little" - 10% to 20% minor grain size

PENETRATION RESISTANCE (IN) GUIDE											
Cohesionless Soils (Sands)	Cohesive Soils (Clays)										
Relative Density / Blows per Foot	Consistency / Blows per Foot	•									
Very Loose >> 0 - 4	Very Soft >> Below 2	•									
Loose >> 4-10	Soft >> 2 - 4	•									
Medium Dense >> 10 - 30	Medium Stiff >> 4 - 8										
Dense >> 30 - 50	Stiff >> 8 - 15										
Very Dense >> Over 50	Very Stiff >> 15 - 30	•									
	Hard >> Over 30	•									

NOTES

"trace" - < 10% of minor grain size

- 1. LOCATION OF BORINGS FROM FEBRUARY 2019 ARE SHOWN ON THE PLAN THUS: BB-# ♠. LOCATION OF BORINGS FROM JULY 2022 ARE SHOWN ON THE PLANS AS THUS: RW-# ♠. LOCATIONS OF PROBES FROM JULY 2022 ARE SHOWN ON THE PLANS AS THUS: P# ●.
- 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. WATER LEVELS SHOWN ON THE BORINGS LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 13 I.D. SPLIT SPOON SAMPLER 6 USING A 140 POUND WEIGHT FALLING 30".

OBSERVED GROUND
WATER EL. 12.2±
FEBRUARY 5, 2019

—BOT. OF WEST INTEGRAL ABUT.

EL. 6.50

- 5. ALL BORINGS WERE MADE IN FEBRUARY, 2019 & JULY, 2022.
- 6. FEBRUARY 2019 & JULY 2022 BORINGS WERE MADE BY CAR-DEE TEST BORING & CONSTRUCTION, LOCATED AT 37 LINDEN ST., MEDFORD, MA 02155.
- 7. ALL PROBES FROM JULY 2022 WHERE MADE BY GEOSCIENCES TESTING AND RESEARCH INC. LOCATED AT 55 MIDDLESEX ST. NORTH CHELMSFORD, MA 01863
- 8. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.
- 9. BORING LOG FOR BORING BB-2B IS NOT AVAILABLE. BORING BB-2B ENCOUNTERED OBSTRUCTION AT 10 FEET BELOW GROUND SURFACE AND RELOCATED TO BB-2C.

COMMONWEALTH OF MASSACHUSETTS
MassDOT, Highway Division
APPROVED UNDER PROVISIONS OF
MASS. GEN. LAWS CH 85 S 35

Cohesive Soils (Clays)

Consistency / Blows per Foot

Very Soft >> Below 2

Medium Stiff >> 4 - 8

STATE BRIDGE ENGINEER

Soft >> 2 - 4

Stiff >> 8 - 15

Hard >> Over 30

Very Stiff >> 15 - 30

MONTH DD, YYYY ISSUED FOR CONSTRUCTION
DATE DESCRIPTION

USE ONLY PRINTS OF LATEST DATE

3/17/2023

DATE

BORING LOG BB-1

SCALE: 1/4" = 1'-0"

SHEET 3 OF 20 SHEETS BRIDGE NO. N-11-007 = W-20-001 (CBR)

PENETRATION RESISTANCE (N) GUIDE

Cohesionless Soils (Sands)

Relative Density / Blows per Foot

Very Loose >> 0 - 4

Medium Dense >> 10 - 30

Very Dense >> Over 50

Loose >> 4 - 10

Dense >> 30 - 50

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	-	22	42
	PROJECT FILE NO.	N/A	

BORING LOG BB-2A

GEOSCIENCES TESTING AND RESEARCH, INC.

55 Middlesex Street, Suite 225, North Chelmsford, MA. Phone: (978) 251-9395 www.gtrinc.net

1 of 2 Page: Plummer Spring Road 22.219 GTR Job #: Newburyport, MA GTR Rep: C. George Location: Reviewer: C. George

BB-2A

Boring No.

Drilling Co. Car-Dee Corporation Steve DeSimone Helper(s): Equipment Casing Sampler Core Groundwater Depth (ft) Frank 2/5/2019 End Date: 2/6/2019 HW Start Date: SS Type Date | Time | Water | Casing | Hole ~ 17.9' 1.75" 4" Size I.D. 2/6 | 7am | Gnd Surface Elev (ft): 25" 300 lb 140 lb 808900.72 E, 3118159.80 N Hammer Wt. Location: 30 in 30 in Truck mounted Deidrich D50 with Automatic Hammer Hammer Fall

	th	BPF	Sample Data					Data	Stratum	Stratum Additional		
	Depth	Case	No.	Pen/ Rcvy	Depth (ft)	Blows per 6in	Field Test	Description and Classification	Description	Data	Notes	
1			SS-1A	18/10	0.7-2.2	21-17		Dry, brown, dense, fine to medium SAND,	ASPHALT	4	1,2	
			SS-1B			39		little Silt, little Gravel	8"		24	
5.00	-		;	5								
	5		S-2	24/0	5-7	1-1		No Recovery				
						WOH for 12	M.S				_	
0.00	_		S-3	24/0	7-9	WOH for		No Recovery				OBSERVED GRO
				:-		24"		4	ru.			WATER EL. 11.
	10	-	SS-4	15/6	10-11.3	15-18		Wet, Brown, very dense, GRAVEL, little	FILL			FEBRUARY 6, 2
	ł		55.9	13/0	10 11.3	50 for 3"		Sand, trace Silt			3	1
00				1				Salita, trace site			858	
	Ī											BOT. OF EAST
	15		0.00]				INTEGRAL ABUT EL. 6.74
			SS-5	24/10	15-17	6-18		Wet, Brown, medium dense, fine to				LL. 0.7 1
, , ,						9-9		medium SAND, some Gravel, trace Clayey				
0.00	- 1			-		-		Silt, trace Wood Fibers	18'		4	
	888	-						⊣ :	PEAT		(65).	
	20		SS-6	24/6	20-22	7-10		Gray, medium dense, GRAVEL, some fine	20'			
	l					12-16		Sand, trace Silt	(1365)			
5.00	_ [
								_	GLACIAL TILL			
	25	-	SS-7	24/7	25-27	46-10		Gray madium dance GRAVEL and fine to				
			33-7	24//	25-27	14-14		Gray, medium dense, GRAVEL and fine to medium Sand, trace Clayey Silt			5	
10.00						14-14		Bottom of boring at 27 feet below ground	27'		3	
	- I							surface due to battered casing.	2/			

- 1. Water level checked prior to starting the second day of drilling.
- 2. The upper 20 feet of the boring was completed with 4-inch hollow stem augers.
- 3. Boulder/Cobbles encountered at 10 to 15 feet below ground surface
- 4. Peat/Organic Silt observed on the augers at approximately 18 to 20 feet below ground surface.
- 5. Due to obstructions encountered casing was battered to much to continue drilling. Casing removed and rig moved ahead for BB-2B Order of Sample Description (Modified Burmister) PENETRATION RESISTANCE (N) GUIDE

- Moisture Content: Dry, Moist, Wet
 Soil Relative Density or Consistency

- Major Component: Should be capitalized
 Minor Component: "and" 35% to 50% minor grain size

"some"	- 20% to 35% minor grain size	
"little"	- 10% to 20% minor grain size	
"trace"	- < 10% of minor grain size	
		•

Cohesionless Soils (Sands)	Cohesive Soils (Clays)				
Relative Density / Blows per Foot	Consistency / Blows per Foot				
Very Loose >> 0 - 4	Very Soft >> Below 2				
toose >> 4-10	Soft >> 2 - 4				
Medium Dense >> 10 - 30	Medium Stiff >> 4 - 8				
Dense >> 30 - 50	Stiff >> 8 - 15				
Very Dense >> Over 50	Very Stiff >> 15 - 30				
= = = = = = = = = = = = = = = = = = = =	Hard >> Over 30				

NOTE:

FOR BORING NOTES SEE SHEET 3 OF 20.

SEE BORING LOG NOTE 5 (THIS SHEET) FOR INFORMATION ON BORING BB-2B.

BORING LOG BB-2A SCALE: 1/4" = 1'-0"

MassDOT, Highway Division APPROVED UNDER PROVISIONS OF MASS. GEN. LAWS CH 85 S 35 3/17/2023 STATE BRIDGE ENGINEER DATE

ISSUED FOR CONSTRUCTION MONTH DD, YYYY DESCRIPTION
USE ONLY PRINTS OF LATEST DATE DATE

COMMONWEALTH OF MASSACHUSETTS

BORING LOG BB-2C

GEOSCIENCES TESTING AND RESEARCH, INC.

55 Middlesex Street, Suite 225, North Chelmsford, MA. Phone: (978) 251-9395 www.gtrinc.net

> 1 of 1 GTR Job #: Plummer Spring Road 22.219 Newburyport, MA GTR Rep: C. George Reviewer: C. George

Boring No.

BB-2C

Car-Dee Corporation Drilling Co. Steve DeSimone Helper(s): Casing Sampler Core Groundwater Equipment Depth (ft) 2/6/2019 End Date: 2/6/2019 Type HW Date Time Water Casing Hole SS ~ 17.9' 1.75" Gnd Surface Elev (ft): Size I.D. 4" 2/6 7am 6' 25' 300 lb 140 lb 808897.04 E, 3118156.24 E Hammer Wt Results from boring BB-2A 30 in Truck mounted Deidrich D50 with Automatic Hammer Hammer Fall 30 in

Sample Data Stratum Additional Blows Data Description and Classification Description Depth (ft) per 6in Test ASPHALT 1,2 15.00 10.00 FILL ₹ 00.00 PEAT -5.00GLACIAL TILL -10.00

L — — — CONTINUATION

1. Stratum description from boring BB-2A. Boring BB-2B encountered obstruction at 10 feet below ground surface and relocated to BB-2C.

2. The upper 10 feet of the boring was completed with 4-inch hollow stem augers.

3. Boulder/Cobbles encountered at 5 to 15 feet below ground surface

Order of Sample Description (Modified Burmister) PENETRATION RESISTANCE (N) GUIDE 1. Moisture Content: Dry, Moist, Wet Cohesionless Soils (Sands) Cohesive Soils (Clays) 2. Soil Relative Density or Consistency Consistency / Blows per Foot Relative Density / Blows per Foot Very Soft >> Below 2 Very Loose >> 0 - 4 4. Major Component: Should be capitalized Loose >> 4-10 Soft >> 2 - 4 5. Minor Component: "and" - 35% to 50% minor grain size Medium Stiff >> 4 - 8 Medium Dense >> 10 - 30 "some" - 20% to 35% minor grain size Stiff >> 8-15 Dense >> 30 - 50 "little" - 10% to 20% minor grain size Very Dense >> Over 50 Very Stiff >> 15 - 30 "trace" -< 10% of minor grain size Hard >> Over 30

CONTINUATION -

5. Minor Component; "and" - 35% to 50% minor grain size

"some" - 20% to 35% minor grain size

"little" - 10% to 20% minor grain size

"trace" - < 10% of minor grain size

		th	BPF	BPF		Sa	mple D	ata	Stratum	Additional	tes	
		Depth	Case	No.	Pen/ Rcvy	Depth (ft)	Blows per 6in	Field Test Results	Description and Classification	Description	Data	Notes
		30		SS-8	24/10	30-32	15-14		Gray, dense, GRAVEL and fine to coarse Sand, trace Clayey Silt			ic.
		(35)					17-19					
_	-15.00					1						4
TION (FEET)	-13.00	1										
									Gray, very dense, GRAVEL, some fine to coarse Sand, trace Clayey Silt	GLACIAL TILL		
		35		S-9	24/8	35-37	27-36					
				Ü			71-45					
	-20.00	1										
				Ĺ								
		40		CC 10	24/10	40.42	27 27		Gray, very dense, fine to medium SAND, little Gravel, little Clayey Silt			
		3		SS-10	24/10	40-42	37-37 45-51					
	-25.00	. 1				1	45-51		Bottom of boring at 42 feet below ground	42'		
	20.00	1		00					surface with no refusal encountered.	7.4		
		45										
				Ţ			Į.					
	-30.00	- 1										
						- 1						
		50							-			
						1	-					
_	-35.00	}			8	9 9		10				
					-							
					S	3						
		55										
_	-40.00	1										
				ĺ								
		NOTE	-									
		3. Ba	sed or	n drilling a	ction a cob	ble/boulder	was encou	intered at a	pproximately 32 to 33 feet below ground sur	face.		
		NAME OF THE PARTY										
							1000	r		_		
						Modified Burm	ister)		PENETRATION RESIST	1		
		Moisture Content: Dry, Moist, Wet Soil Relative Density or Consistency Color							Cohesionless Soils (Sands) Relative Density / Blows per Foot	Cohesive Soils (Clays) Consistency / Blows per Foot		
									Very Loose >> 0 - 4	Very Soft	>> Below 2	
		4. Major Component: Should be capitalized 5. Minor Component: "and" - 35% to 50% minor grain size							Loose >> 4 - 10 Medium Dense >> 10 - 30	Soft >> 2 - 4		

NOTE:

FOR BORING NOTES SEE SHEET 3 OF 20. SEE BORING LOG NOTE 1 (THIS SHEET) FOR INFORMATION ON BORING BB-2B.

BORING LOG BB-2C SCALE: 1/4" = 1'-0"

OBSERVED GROUND WATER EL. 11.9±

FEBRUARY 6, 2019

BOT. OF EAST

EL. 6.74

INTEGRAL ABUT.

COMMONWEALTH OF MASSACHUSETTS MassDOT, Highway Division APPROVED UNDER PROVISIONS OF MASS. GEN. LAWS CH 85 S 35

STATE BRIDGE ENGINEER

Stiff >> 8 - 15

Hard >> Over 30

Very Stiff >> 15 - 30

Medium Stiff >> 4 - 8

3/17/2023 DATE

APPROX.

 $\overline{\text{EL.}} - 27.00 (\pm)$

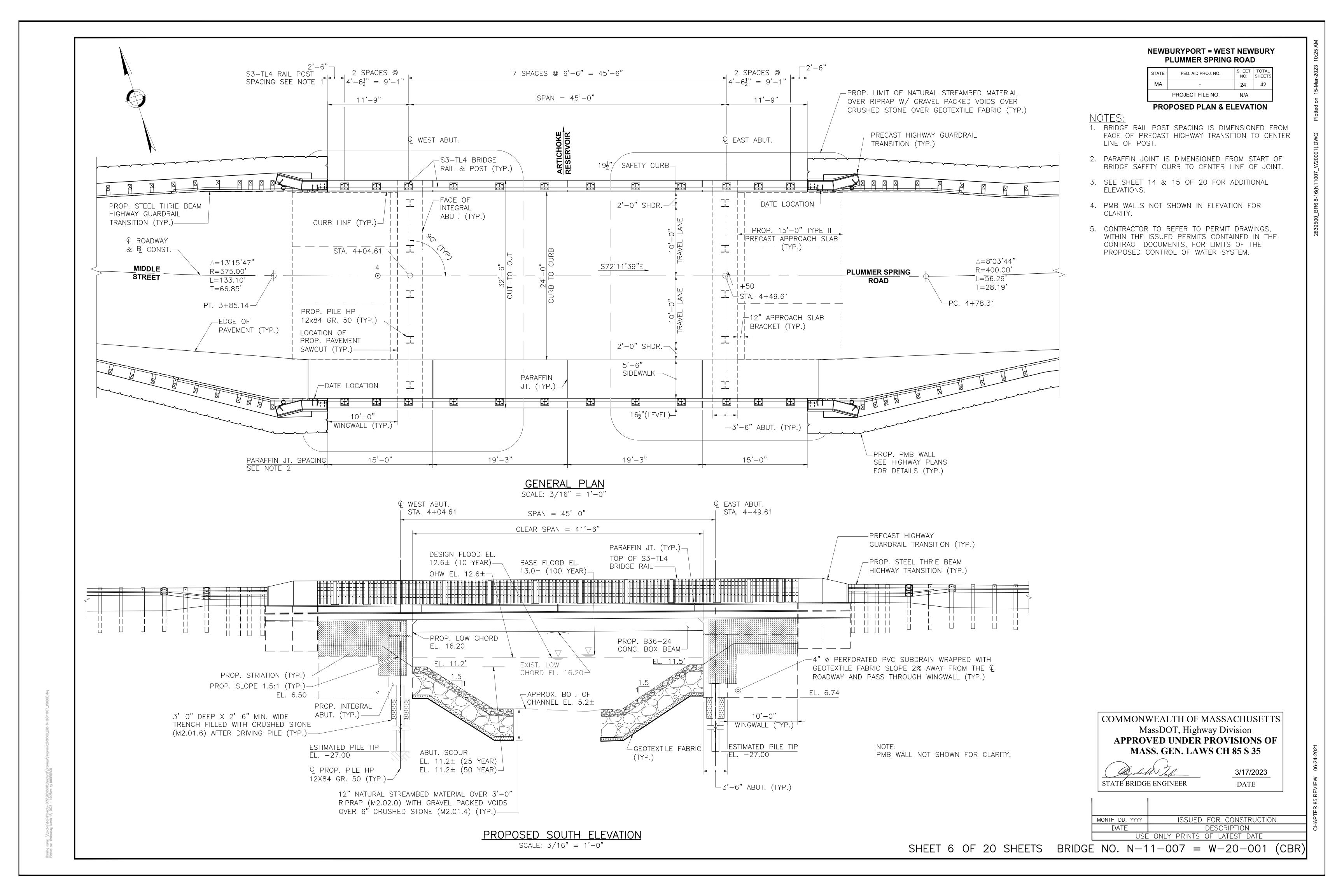
ISSUED FOR CONSTRUCTION MONTH DD, YYYY DESCRIPTION DATE USE ONLY PRINTS OF LATEST DATE

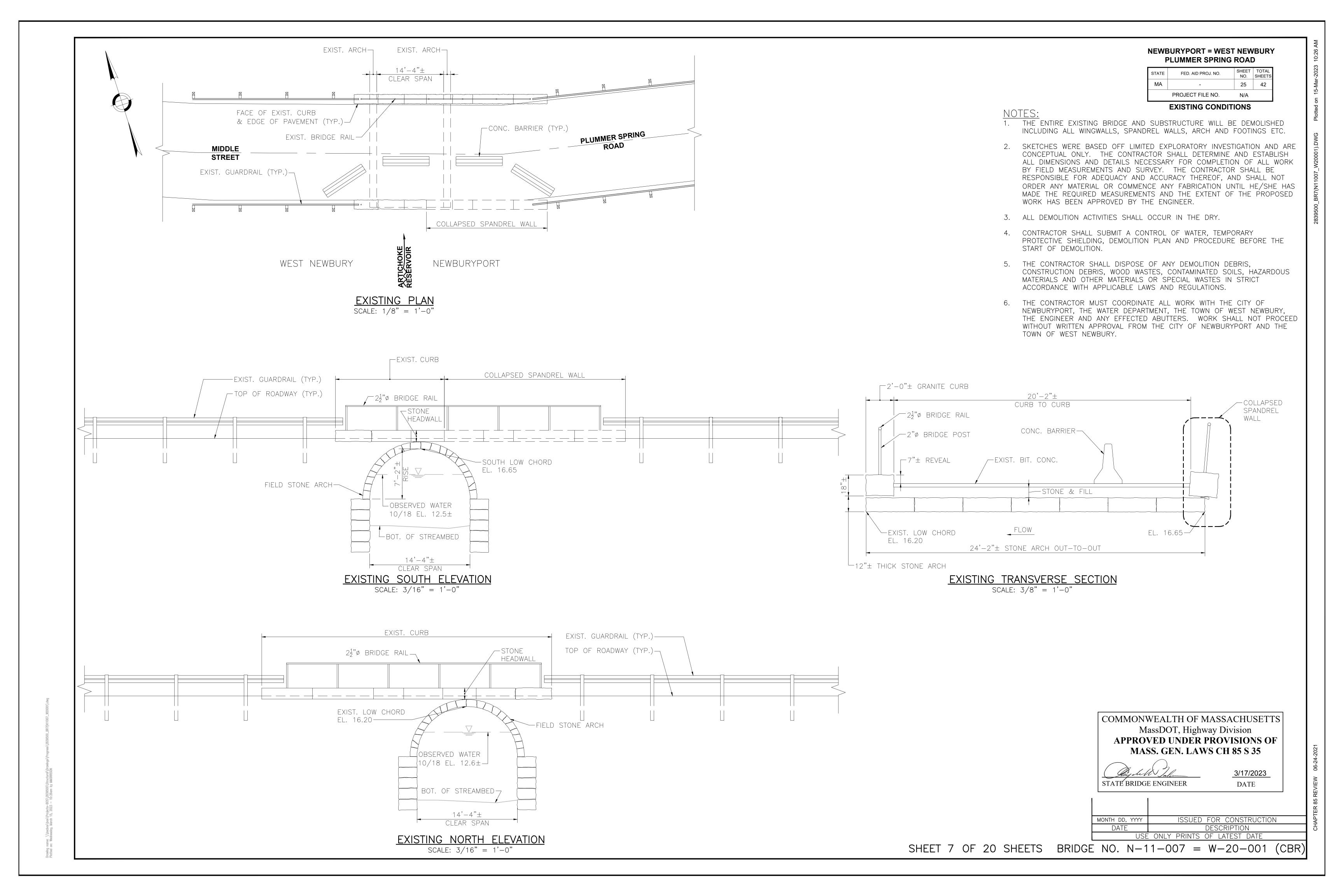
SHEET 5 OF 20 SHEETS BRIDGE NO. N-11-007 = W-20-001 (CBR)

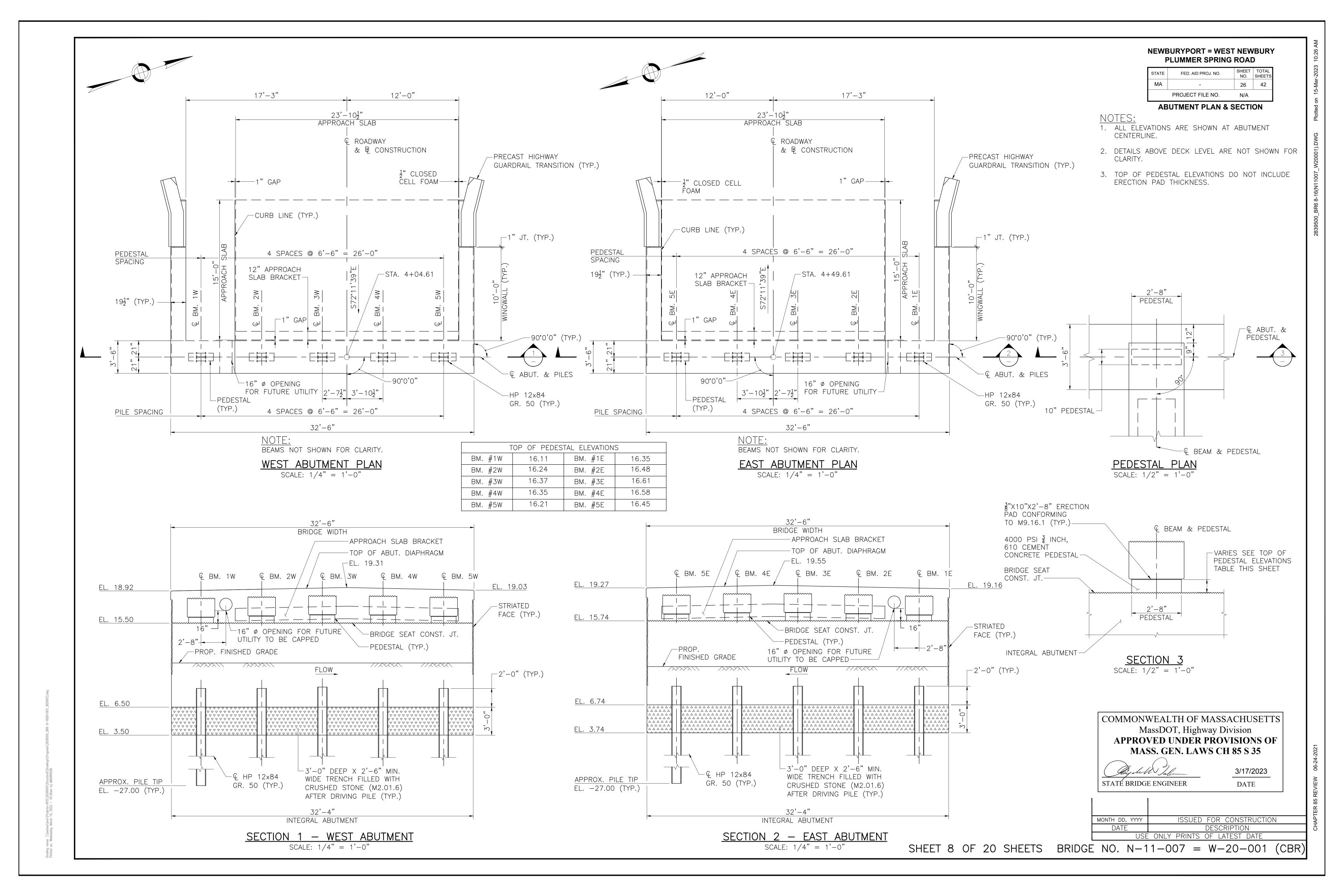
Dense >> 30 - 50

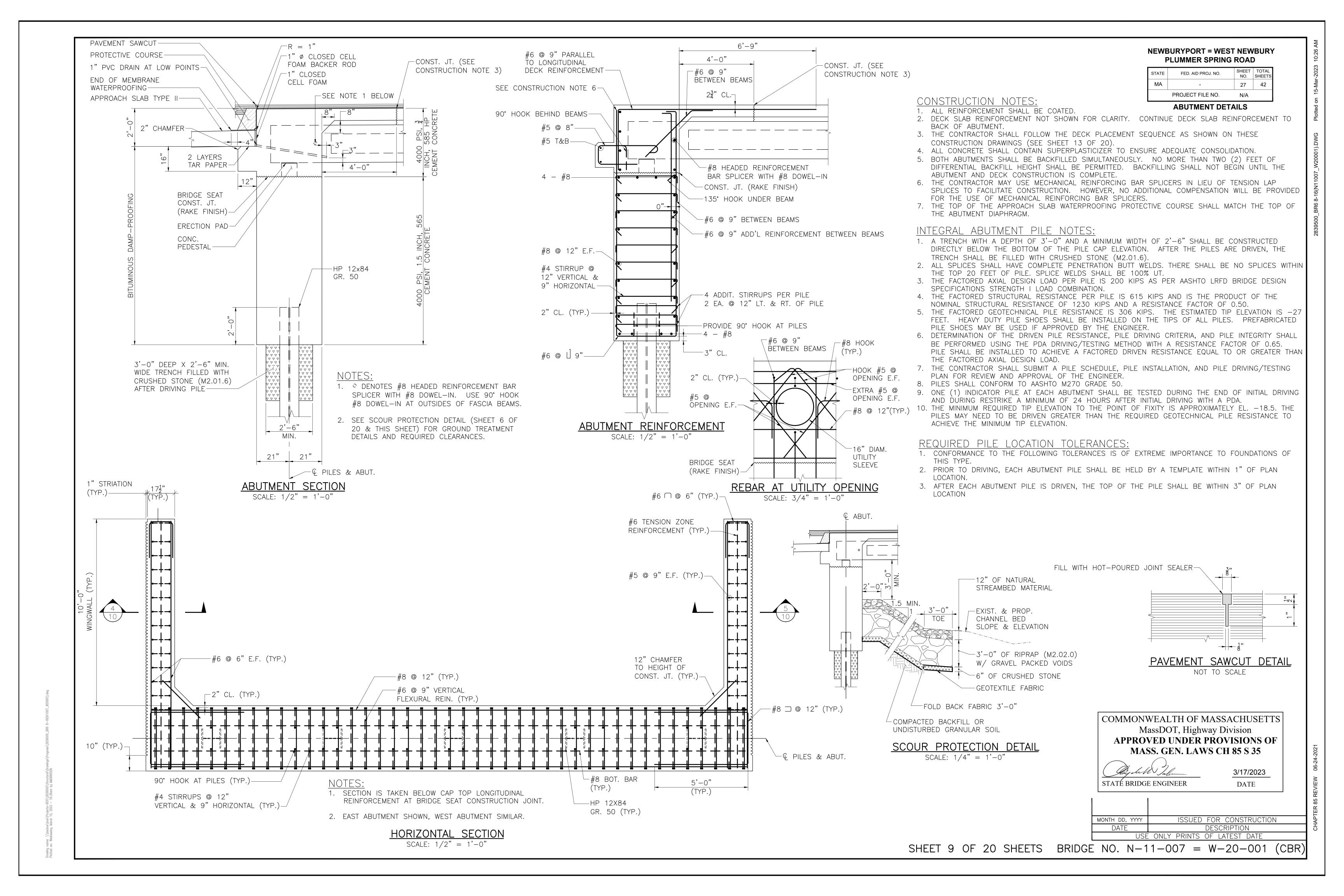
Very Dense >> Over 50

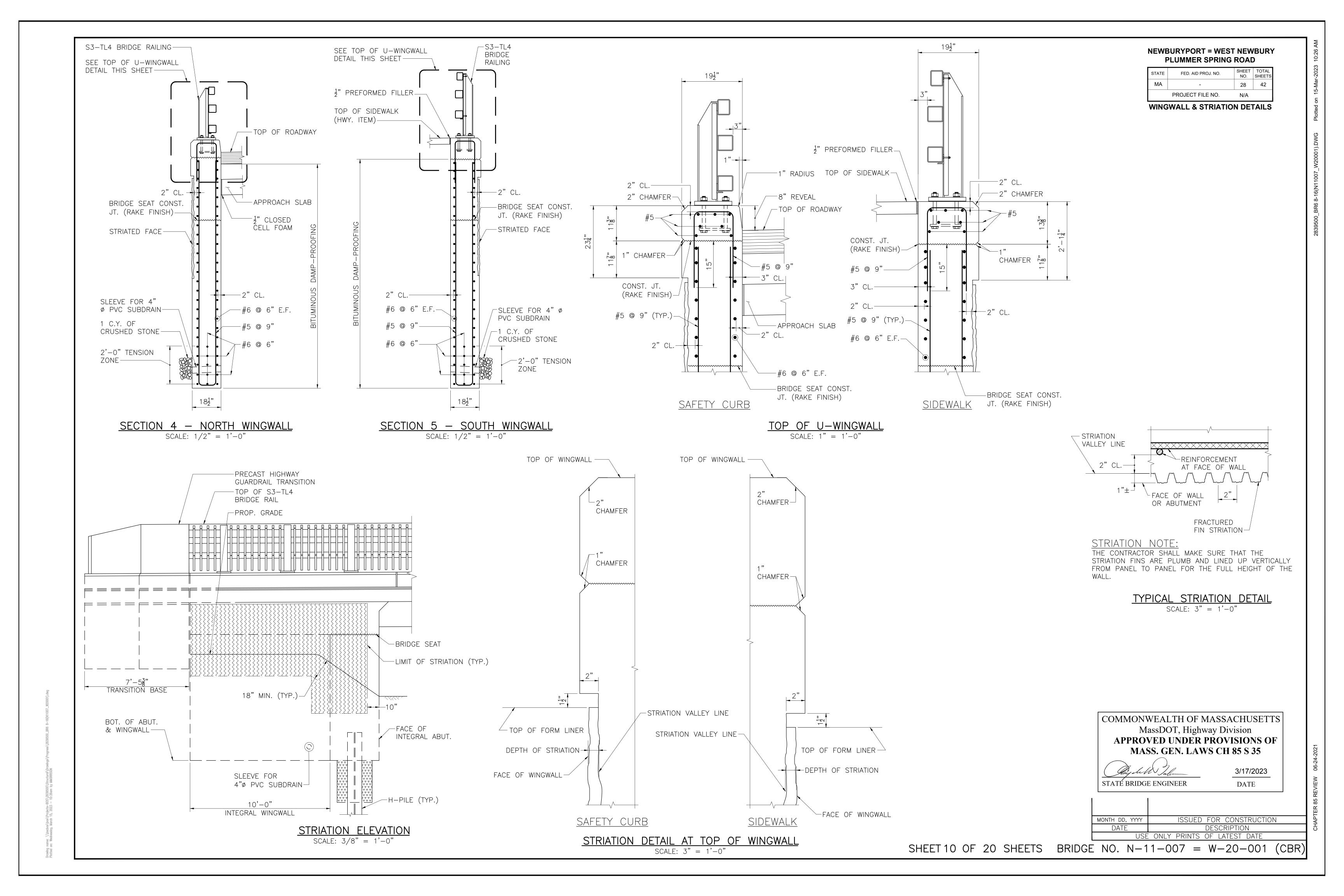
Medium Dense >> 10 - 30

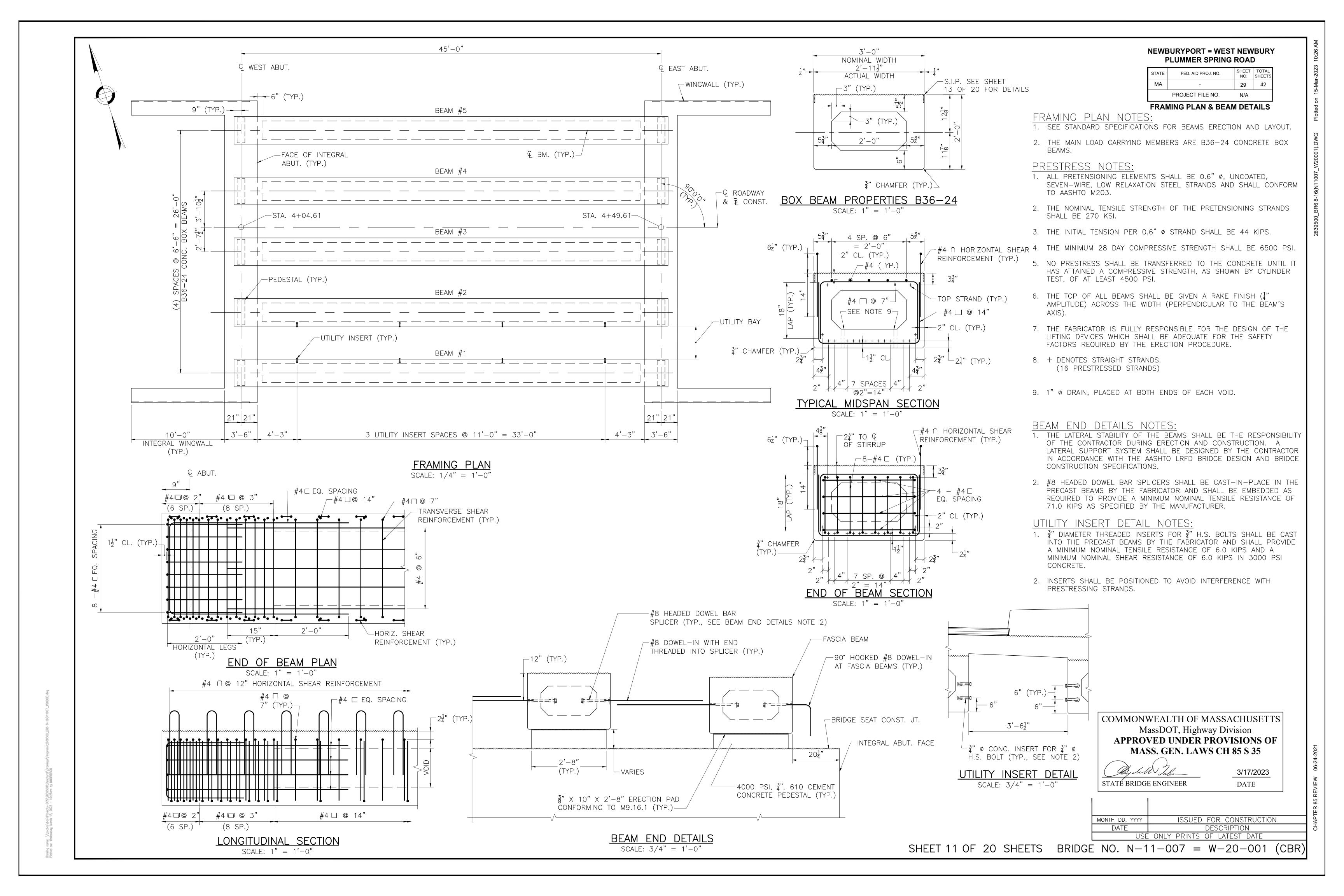


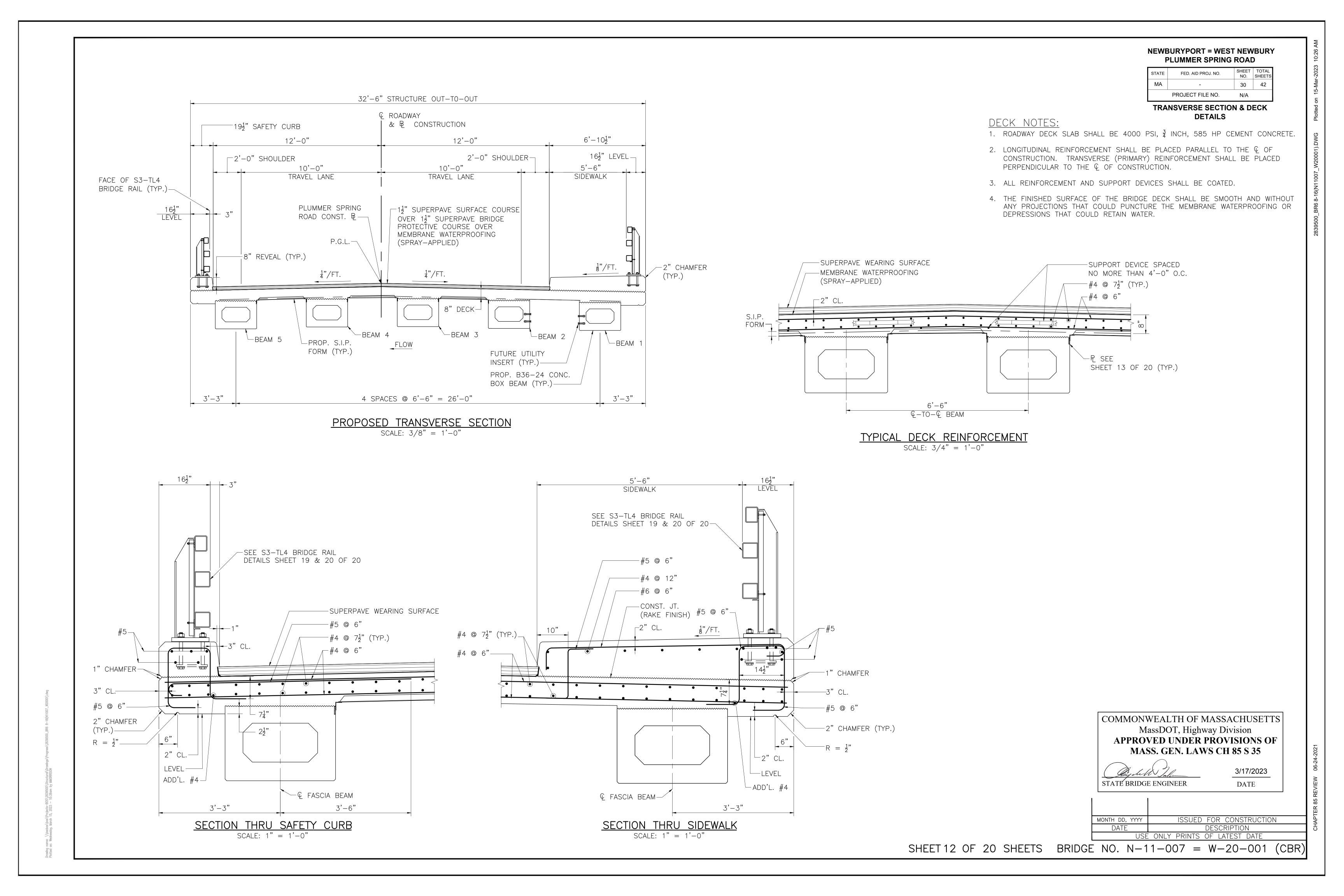












PLUMMER SPRING ROAD STATE FED. AID PROJ. NO. 31 42 PROJECT FILE NO. N/A

DECK DETAILS

STAY-IN-PLACE FORM NOTES:

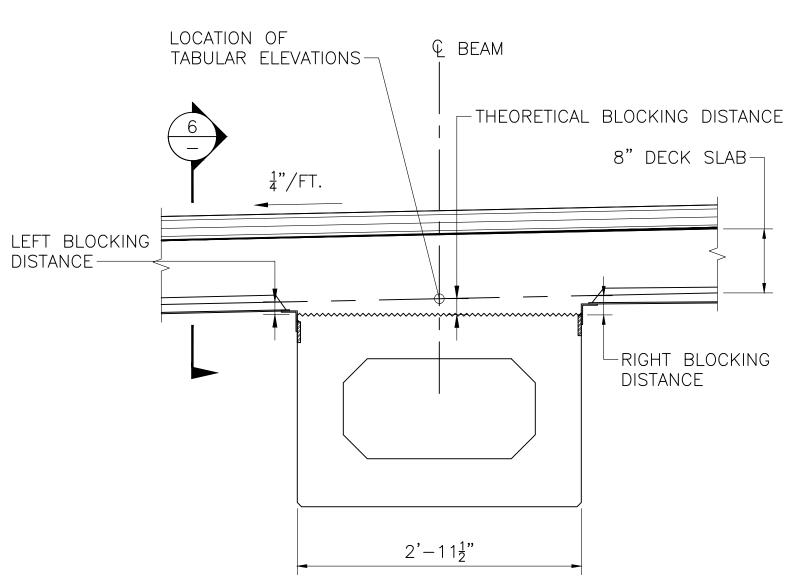
- 1. FOR 2" S.I.P. FORM, SET BOTTOM OF FORM 1" BELOW ELEVATION GIVEN IN TABLE FOR 3" S.I.P. FORM, SET BOTTOM OF FORM $1\frac{1}{2}$ " BELOW TABLE ELEVATIONS.
- 2. FORM ENDS SHALL BE CRIMPED CLOSED IN A TAPERED MANNER. SEPARATE END CLOSURE PIECES WILL NOT BE ALLOWED.
- 3. SUPPORT ANGLES SHALL BE PLACED IN THE "LEG DOWN" POSITION WHERE POSSIBLE. WHERE "LEG UP" POSITION IS NECESSARY, THE UPPER MOST PORTION OF THE ANGLE SHALL NOT PROJECT MORE THAN 1" ABOVE THE TOP FLANGE. THE CONTRACTOR SHALL HAVE AN ASSORTMENT OF ANGLES OF VARIOUS SIZES AVAILABLE ON THE SITE TO CONFORM TO THIS REQUIREMENT.
- 4. ALL MAIN STEEL REINFORCEMENT IN THE LOWER MAT SHALL BE CENTERED OVER THE VALLEY OF THE S.I.P. FORM.
- 5. THE CONTRACTOR SHALL DESIGN AND DETAIL ALL ELEMENTS OF THE FORMING SYSTEM AND SHALL SUBMIT TO THE ENGINEER FOR APPROVAL.
- 6. IN CASES WHERE STANDARD 2" OR 3" DEEP S.I.P. FORMS DO NOT SATISFY DESIGN REQUIREMENTS AN ALTERNATIVE FORMING SYSTEM CONSISTING OF DEEPER S.I.P. FORMS OR REMOVABLE FORMS SHALL BE DESIGNED AND DETAILED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL. THE DESIGN THICKNESS OF THE SLAB SHALL NOT BE REDUCED.

STAY-IN-PLACE FORM ATTACHMENT NOTES:

- 1. EMBEDDED ATTACHMENT PLATES SHALL BE HOT-DIPPED GALVANIZED AASHTO M270 GRADE 36 STEEL. THE PLATES SHALL BE IN LENGTHS FROM 3' TO 12' WITH PIECES BUTTED TOGETHER WITHOUT END CONNECTIONS FOR FULL LENGTH OF BEAM. THE HEADED ANCHORS SHALL BE ATTACHED TO THE PLATES PRIOR TO GALVANIZING.
- 2. HEADED ANCHORS SHALL CONFORM TO M8.04.1 FOR MATERIAL REQUIREMENTS
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE S.I.P. FORM SEAT AND WELD.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING THE CONCRETE LAITANCE FROM THE ATTACHMENT PLATE BEFORE INSTALLING THE S.I.P. FORMS.

DECK CONSTRUCTION JOINT NOTES:

- 1. BRIDGE DECK SLAB SHALL BE PLACED IN ACCORDANCE WITH THE PLACEMENT SEQUENCE SHOWN ON THE PLANS.
- 2. THE SURFACE OF THE PREVIOUSLY CAST CONCRETE SHALL BE BLAST CLEANED, ROUGHENED. WETTED WITH CLEAN WATER, AND THEN FLUSHED WITH A MORTAR COMPOSED OF EQUAL PARTS OF THE CEMENT AND SAND SPECIFIED FOR THE NEW CONCRETE, BEFORE NEW CONCRETE IS PLACED ADJACENT THERETO. NEW CONCRETE SHALL BE PLACED BEFORE MORTAR HAS TAKEN INITIAL SET.
- 3. IN LIEU OF THE MORTAR, AN EPOXY ADHESIVE SUITABLE FOR BONDING FRESH CONCRETE TO HARDENED CONCRETE FOR LOAD BEARING APPLICATIONS MAY BE USED. THE EPOXY ADHESIVE SHALL CONFORM TO AASHTO M 235 TYPE V AND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 4. DOWEL BAR SPLICERS SHALL BE USED WHERE USE OF LAP SPLICES IS NOT FEASIBLE.



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- 1. THE RIGHT AND LEFT ORIENTATION IS TAKEN LOOKING UPSTATION ALONG THE BEAM.
- 2. RIGHT BLOCKING DISTANCE = THEORETICAL BLOCKING DISTANCE + 0.375". LEFT BLOCKING DISTANCE = THEORETICAL BLOCKING DISTANCE - 0.375".

HAUNCH DETAIL SCALE: 1" = 1'-0"

(SEE NOTE 3) S.I.P. FORM SEAT (SEE NOTE 3)
3" Ø HEADED CONC. ANCHOR,
6" LONG, 12" O.C. MAX. 4"
MAX. FROM PLATE END, WELD TO PLATE BEFORE GALVANIZING—
13"
$\frac{3}{8}$ "X2 $\frac{1}{2}$ " EMBEDDED
ATTACHMENT P SEE NOTE 1
BOX BEAM 1" 1" MIN.

DECK PLACEMENT SEQUENCE

SCALE: 3/16" = 1'-0"

-SEQUENCE

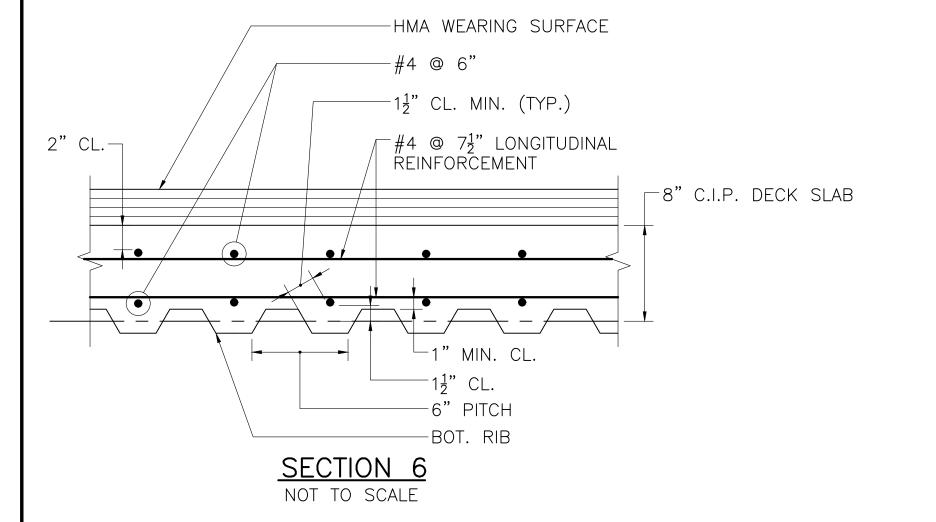
NUMBER.

(TYP.)

STAY-IN-PLACE FORM ATTACHMENT DETAIL SCALE: 3'' = 1'-0''

	TOP OF FORM ELEVATIONS FOR DECK SLAB PRIOR TO PLACEMENT OF CONCRETE						
BEAM	INCREASING STATIONS						
NO.	ℚ BRG.	1/4 PT.	1/2 PT.	3/4 PT.	© BRG.		
1	18.32	18.41	18.48	18.53	18.56		
2	18.46	18.54	18.61	18.66	18.69		
3	18.59	18.67	18.74	18.79	18.83		
4	18.56	18.65	18.72	18.77	18.80		
5	18.43	18.51	18.58	18.63	18.67		
NOTE							

AFTER THE BEAMS ARE ERECTED BUT BEFORE THE FORMS ARE BUILT. ELEVATIONS ON TOP OF THE FLANGE OF THE BEAMS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE SHOWN IN THE TABLE GIVES THE THEORETICAL BLOCKING DISTANCE FROM THE TOP OF BEAM TO THE BOTTOM OF THE SLAB AT CENTER LINE OF BEAM



Q WEST ABUT.

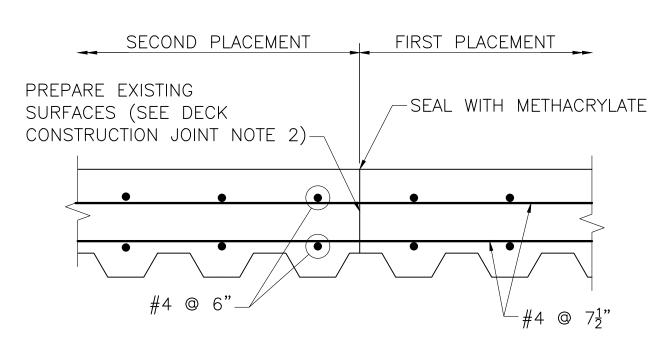
4'-0"

-FACE OF

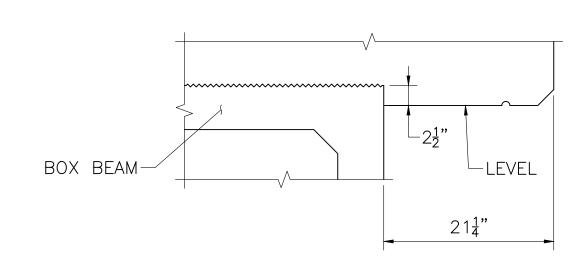
ABUT. (TYP.)

—CONST. JT.

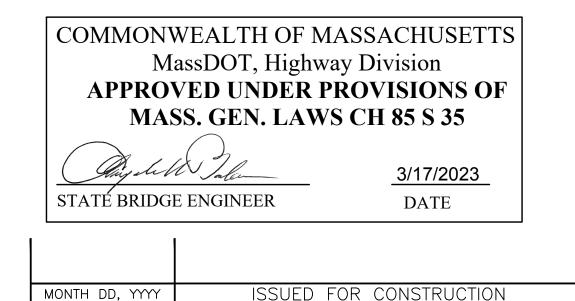
(TYP.)



TRANSVERSE CONSTRUCTION JOINT DETAIL IN DECK SLAB NOT TO SCALE



SLAB OVERHANG AT FASCIA BEAM SCALE: 1" = 1'-0"

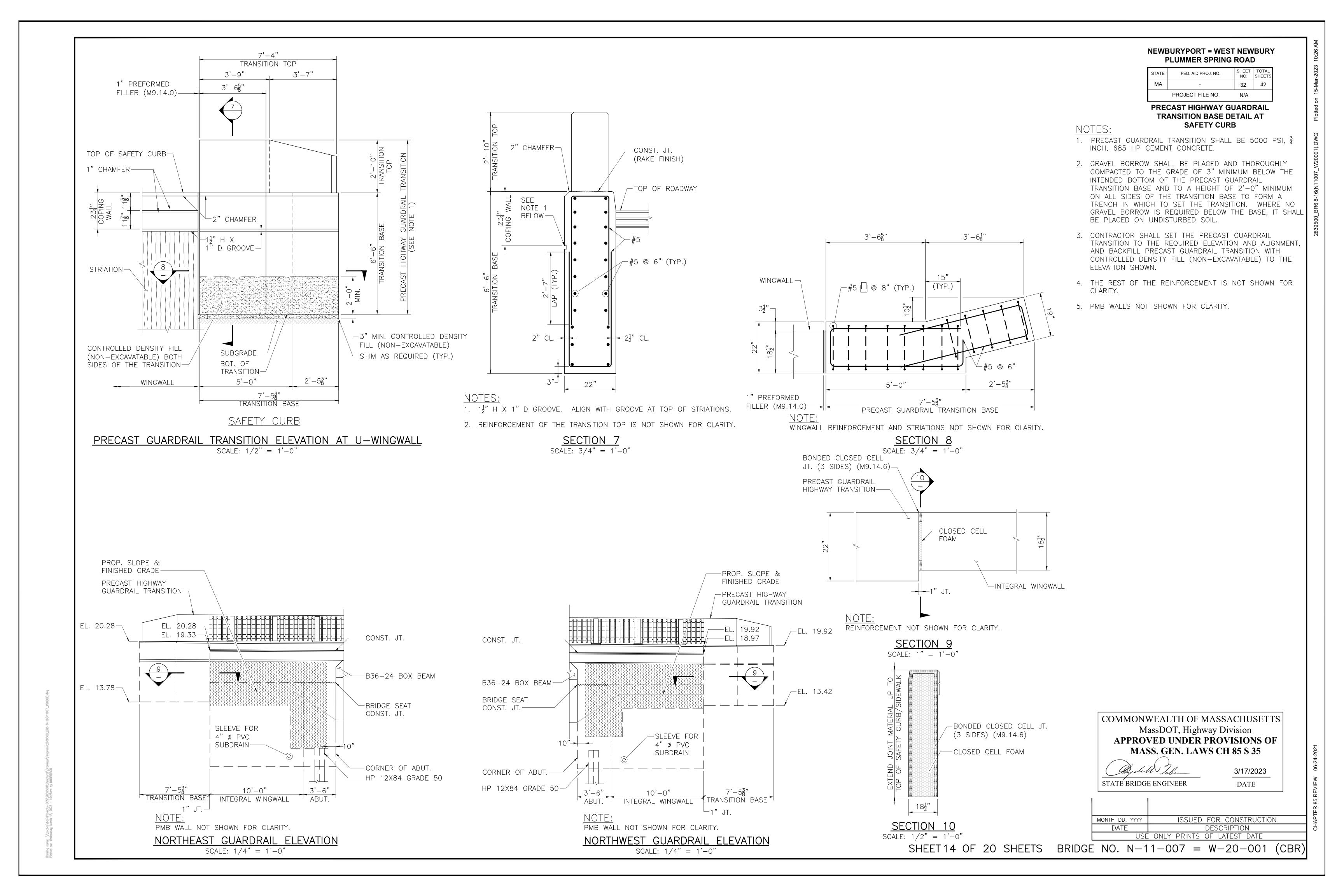


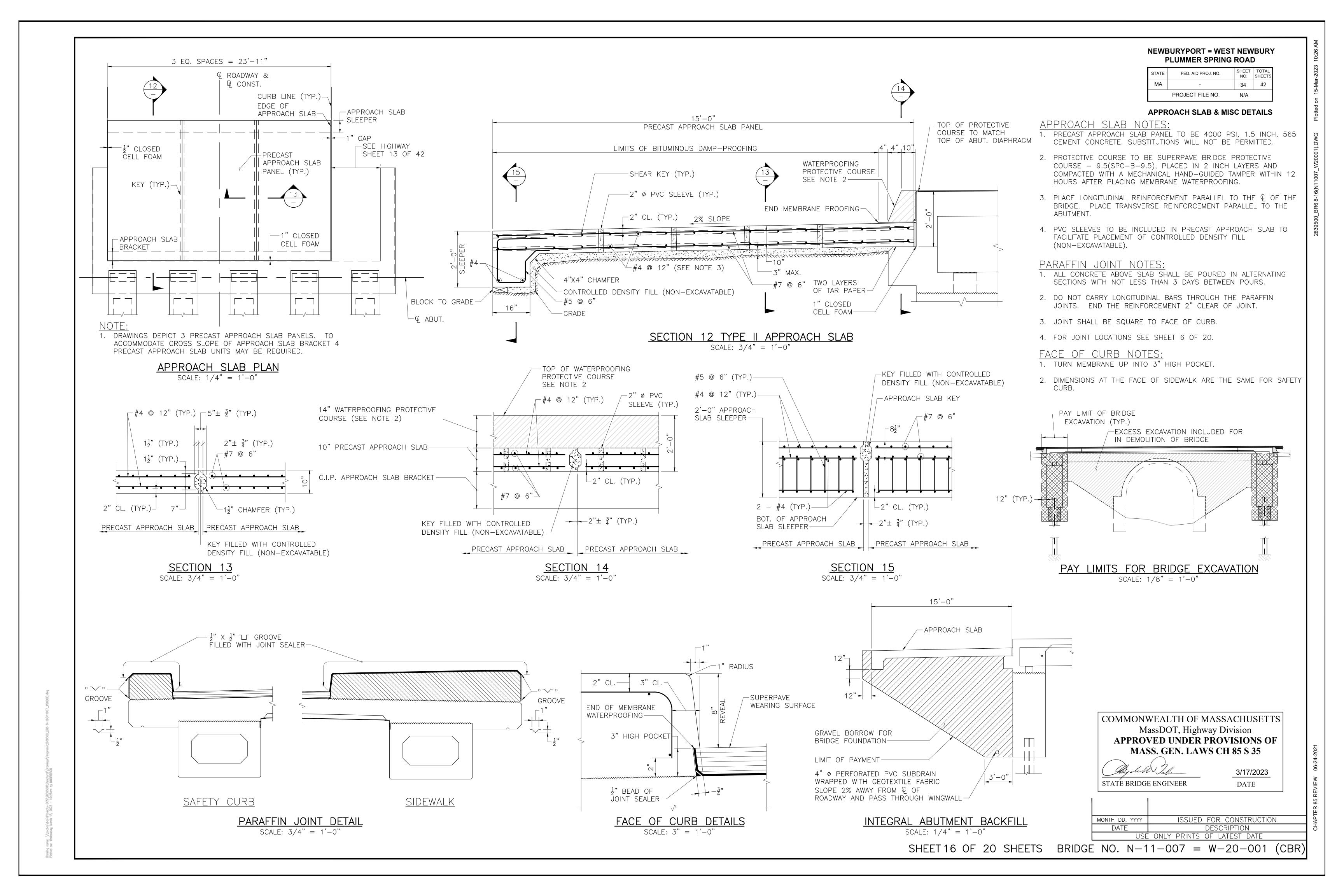
USE ONLY PRINTS OF LATEST DATE

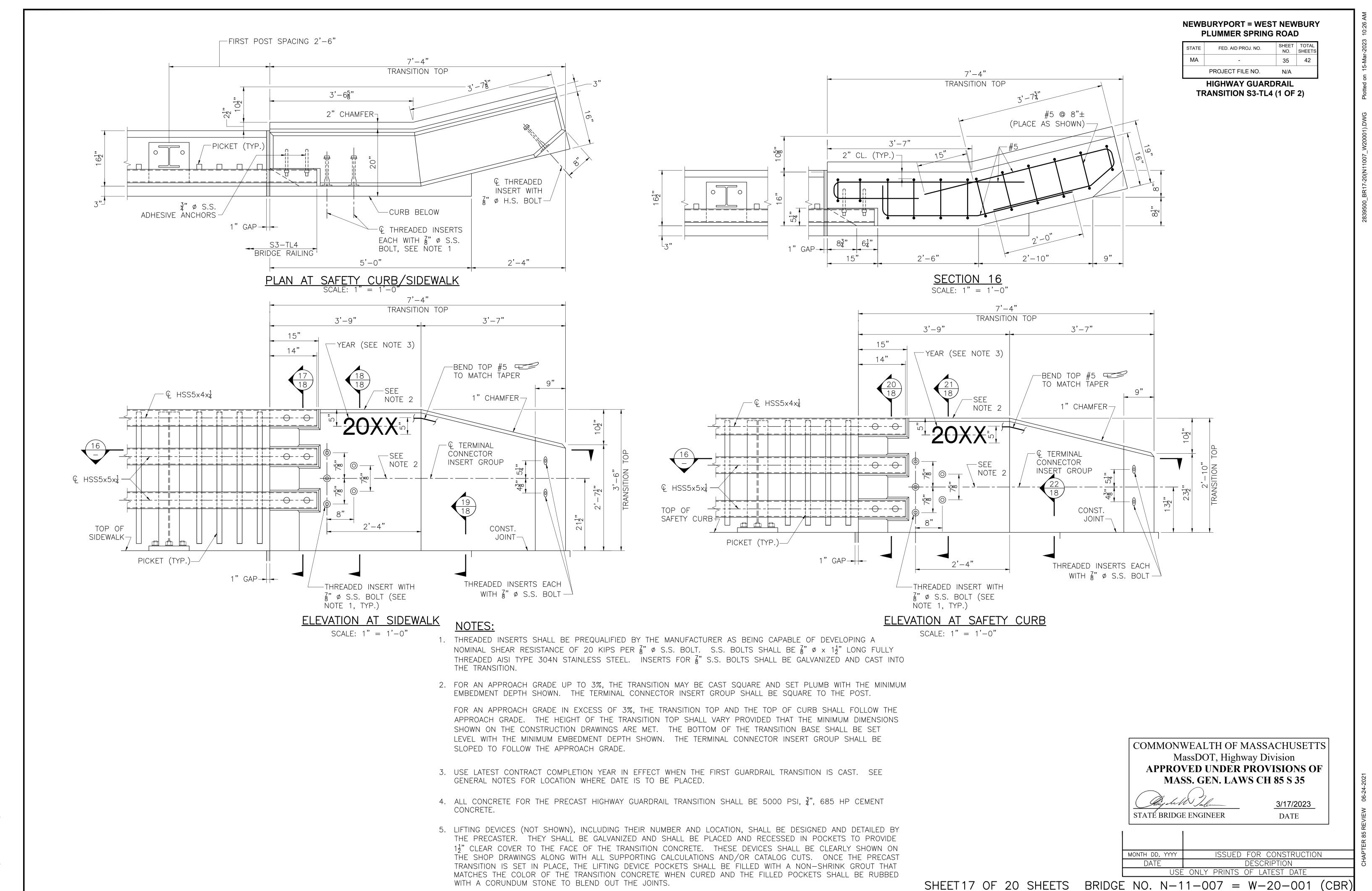
DESCRIPTION

SHEET 13 OF 20 SHEETS BRIDGE NO. N-11-007 = W-20-001 (CBR)

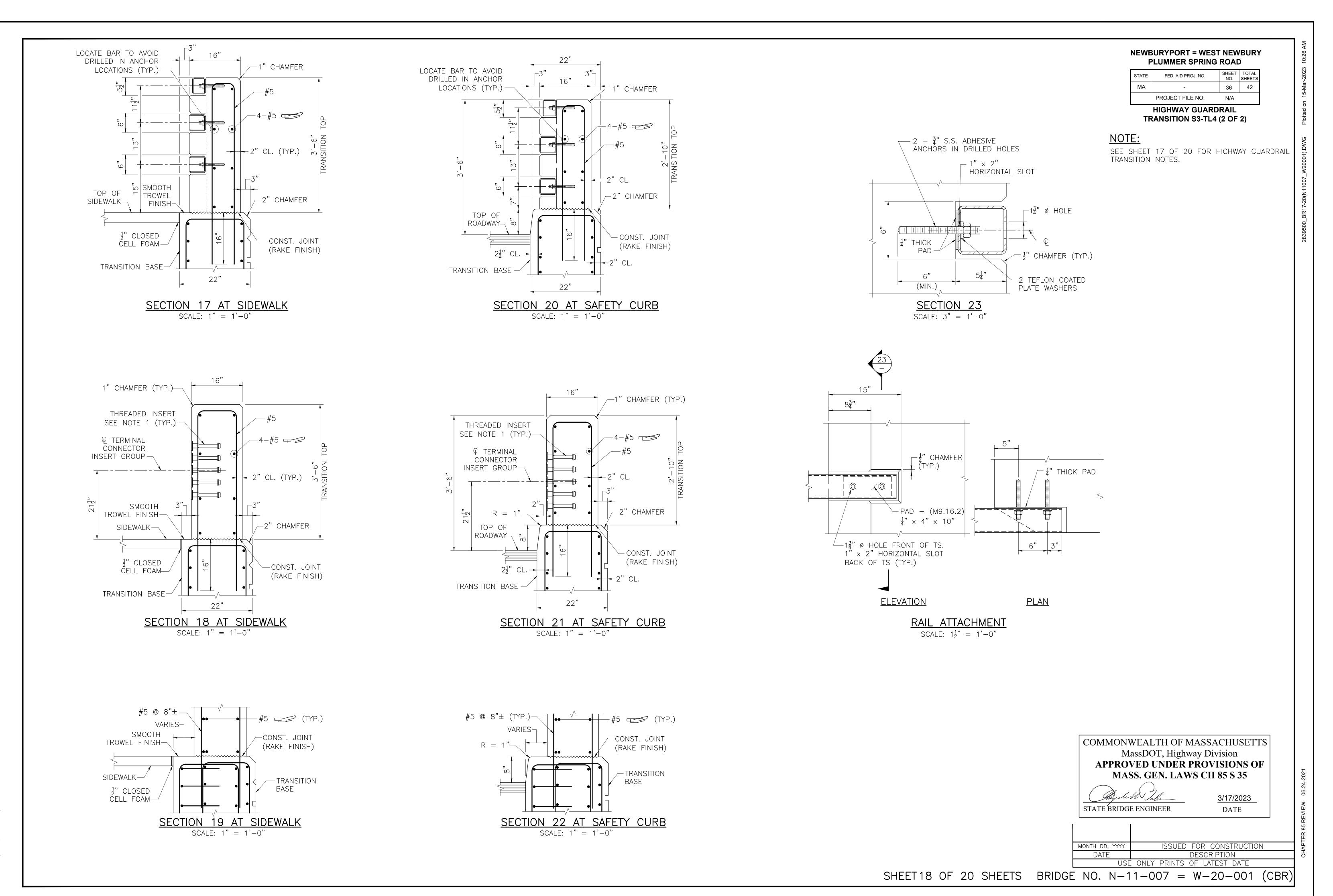
DATE







Drawing name: \\bscbos\bos\englis=BOS\2839500\Sfurctura\\Drawings\Progress\2839500_BR17-20(N11007_W20 Plotted on: Wednesday, March 15, 2023 - 10:26am by MMORRISON



Drawing name: \\betacbos\projects-BOS\2839500\Structural\Drawings\Progress\2839500_BR17-20(Plotted on: Wednesday, March 15, 2023 - 10:26am by MMORRISON



STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA		37	42
	PROJECT FILE NO.	N/A	

S3-TL4 RAILING DETAILS (1 OF 2)

1. RAIL POST AND BASE PLATES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 270 GRADE 50. HOLLOW RAILING STRUCTURAL TUBING (HSS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 500 WITH A CERTIFIED Fy = 50 KSI MINIMUM. THE MINIMUM HORIZONTAL BENDING RADII OF THE HSS TUBING SHALL BE 8 FEET. PICKET CARRIER ANGLES, ANCHOR PLATES, AND SPLICE TUBE PLATES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 270 GRADE 36. PICKET TUBING SHALL CONFORM TO ASTM A 513 WITH Fy = 36 KSI MIN. OR A 500 GRADE B.

RAILING NOTES:

2. ALL STEEL (EXCEPT THE §" ANCHOR PLATE AND FASTENERS) SHALL BE GALVANIZED AND PAINTED DARK BRONZE (FEDERAL STD. 595B COLOR NO. 10045). ANCHOR PLATE SHALL BE GALVANIZED ONLY. HEADS OF §" Ø ROUND HEAD BOLTS SHALL BE PAINTED TO MATCH RAIL.

3. ANCHOR BOLTS SHALL BE SET WITH TEMPLATES. THE NUT SECURING THE POST BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN AN ADDITIONAL 1/8 TURN AFTER STEEL IS IN PLACE.

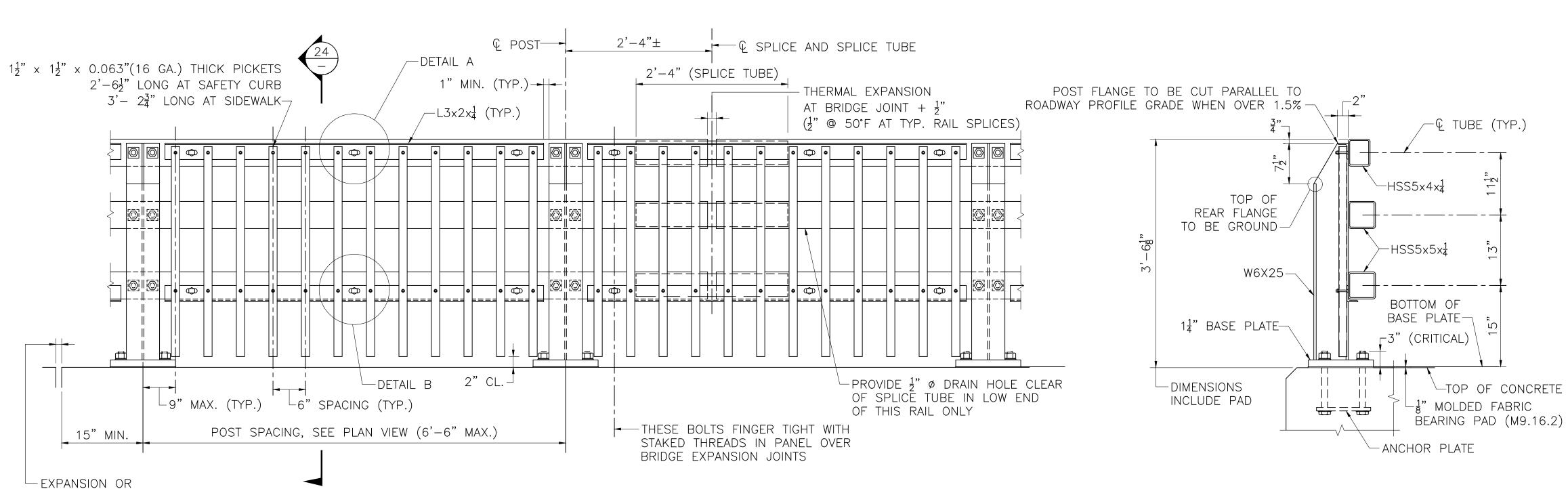
4. RAILS SHALL BE CONTINUOUS OVER A MINIMUM OF FOUR (4) POSTS WITHOUT SPLICES WHERE POSSIBLE. RAILS SHALL BE SPLICED IN THE PANELS OVER EXPANSION JOINT.

5. ENDS OF TUBE SECTIONS SHALL BE SAWED. GRIND SMOOTH EXPOSED EDGES. ALL CUT ENDS SHALL BE TRUE AND SMOOTH.

6. ALL POSTS TO BE PLUMB WHEN PROFILE GRADE EXCEEDS 1.5%. FOR PROFILE GRADES LESS THAN 1.5%, POSTS SHALL BE SET PERPENDICULAR TO GRADE.

7. POST FLANGE WELD DOES NOT REQUIRE MAGNETIC PARTICLE TESTING.
WELD SHALL BE BACK—GOUGED ON BACK SIDE EXCEPT AT WEB. WELD
IS THE SAME ON BOTH FLANGES.

8. 7" Ø ROUND HEAD BOLTS SHALL CONFORM TO THE CHEMICAL AND PHYSICAL REQUIREMENTS OF AASHTO M 164.



CONSTRUCTION

EXPANSION OR

JOINT

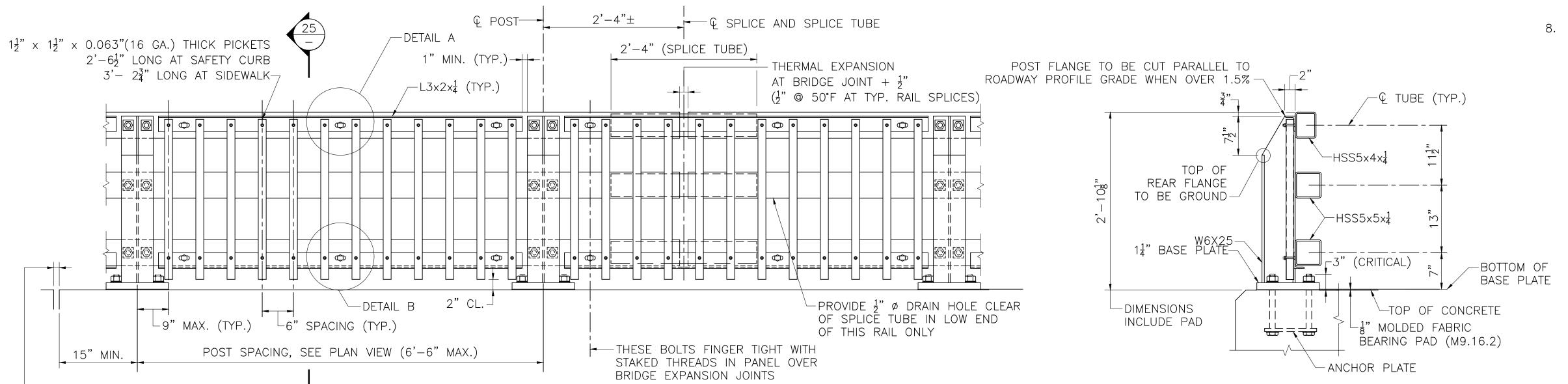
CONSTRUCTION

JOINT

BRIDGE RAILING ELEVATION AT SIDEWALK

SCALE: 1" = 1'-0"

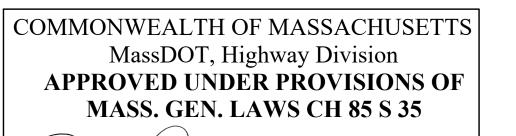
SCALE: 1" = 1'-0"



BRIDGE RAILING ELEVATION AT SAFETY CURB

SCALE: 1" = 1'-0"

SECTION 25 SCALE: 1" = 1'-0"

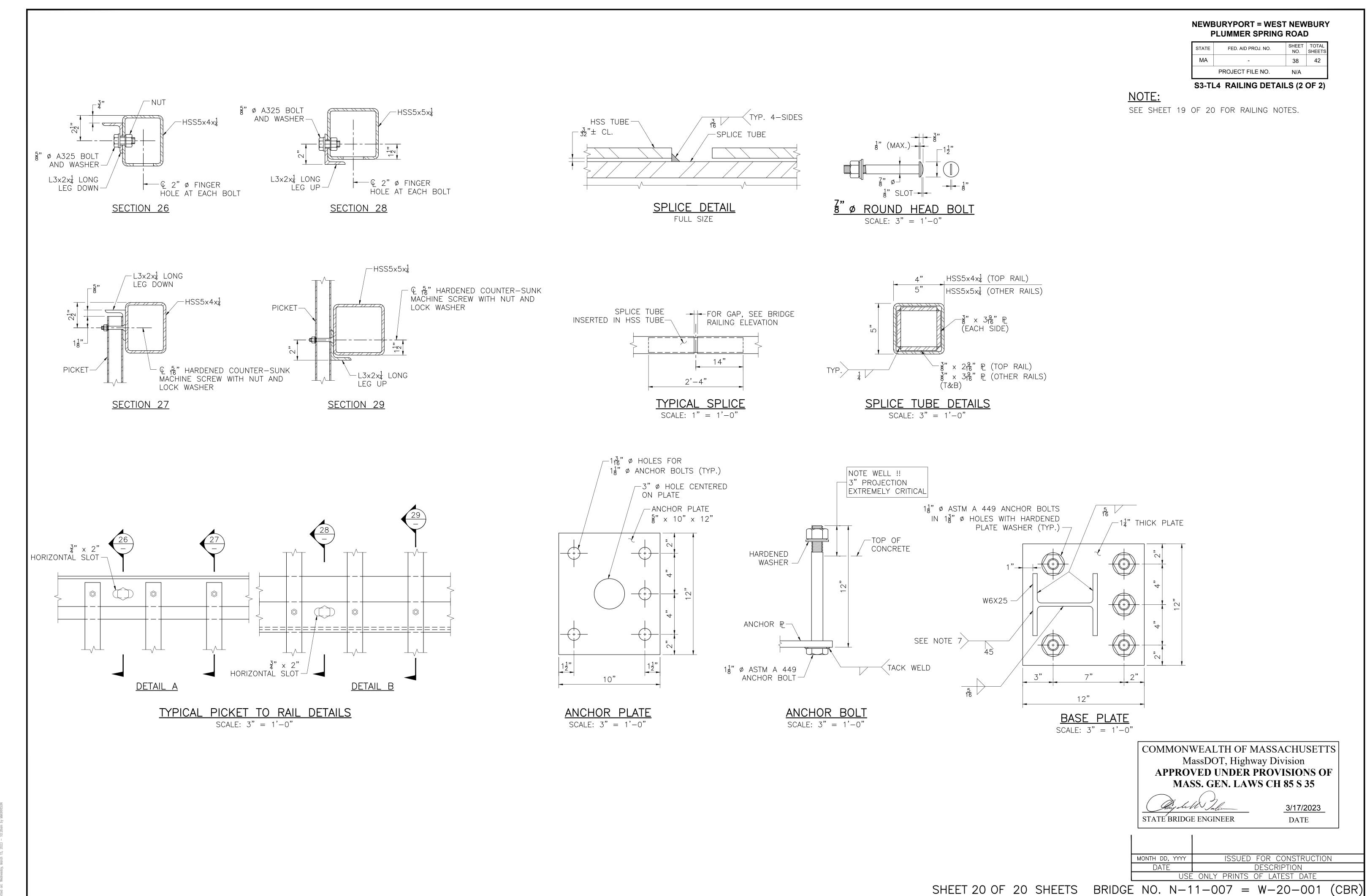


3/17/2023

DATE

STATE BRIDGE ENGINEER

MONTH DD, YYYY ISSUED FOR CONSTRUCTION
DATE DESCRIPTION



Drawing name: \\bscbos\brojects=BOS\2839500\Structura\Drawings\Progress\2839500_BR17-20(N11007_W