LUDLOW **PINEY LANE**

MA STP(BR-OFF)-003S(782)X 1

TITLE SHEET & INDEX

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION HIGHWAY DIVISION

PLAN AND PROFILE OF

PINEY LANE

(BRIDGE NO. L-16-026)

IN THE TOWN OF

LUDLOW

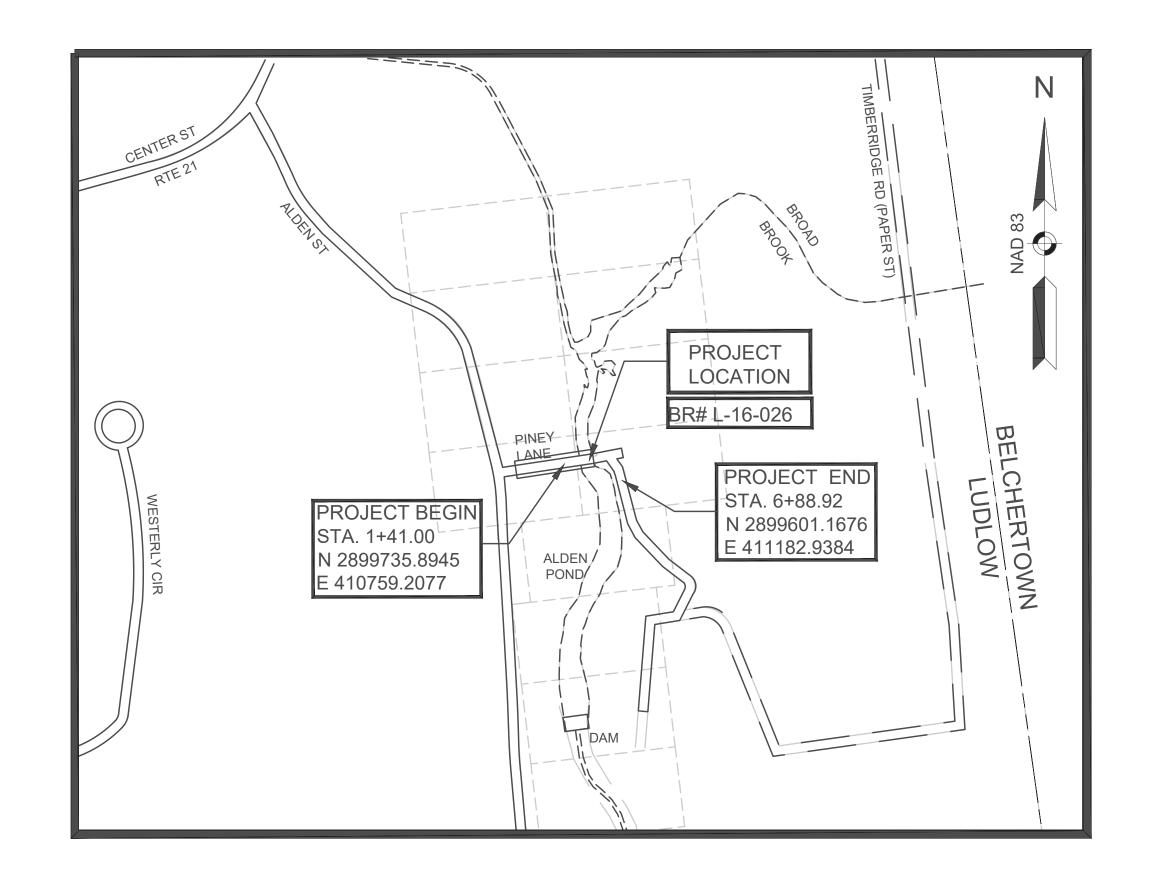
HAMPDEN COUNTY

FEDERAL AID PROJECT NO. STP(BR-OFF)-003S(782)X

AND THE LATEST EDITION OF THE AMERICAN STANDARD FOR NURSERY STOCK.

INDEX

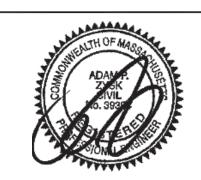
SHEET NO.	DESCRIPTION
1	TITLE SHEET & INDEX
2	LEGEND & ABBREVIATIONS
3	CONSTRUCTION BASELINE TIES
4	TYPICAL SECTIONS
5-6	CONSTRUCTION PLANS
7-8	PROFILES
9-10	CURB TIE & GRADING PLANS
11	PAVEMENT MARKING AND SIGNING PLAN
12	TEMPORARY TRAFFIC CONTROL PLAN
13	TEMPORARY TRAFFIC CONTROL DETAILS
14	UTILITY PLAN
15	RESTORATION PLAN
16	PLANTING PLAN
17-19	CONSTRUCTION DETAILS (3 SHEETS)
20-47	BRIDGE PLANS (28 SHEETS)
48-52	CROSS SECTIONS (5 SHEETS)



DESIGN DESIGNATION (PINEY LANE)			
DESIGN SPEED	25 MPH		
ADT (2022)	113		
ADT (2042)	125		
К	11.5%		
D	50%		
T (PEAK HOUR)	0%		
T (AVERAGE DAY)	0%		
DHV	13		
DDHV	7		
FUNCTIONAL CLASSIFICATION	RURAL LOCAL ROAD		



LENGTH OF PROJECT = 547.92 FEET = 0.10 MILES



ISSUED FOR CONSTRUCTION DESCRIPTION

Adam Zysk Digitally signed by Adam Zysk Date: 2024.09.13 11:59:38



BOSTON, MA 02110 P: (617) 695-3400 F: (617) 695-3310

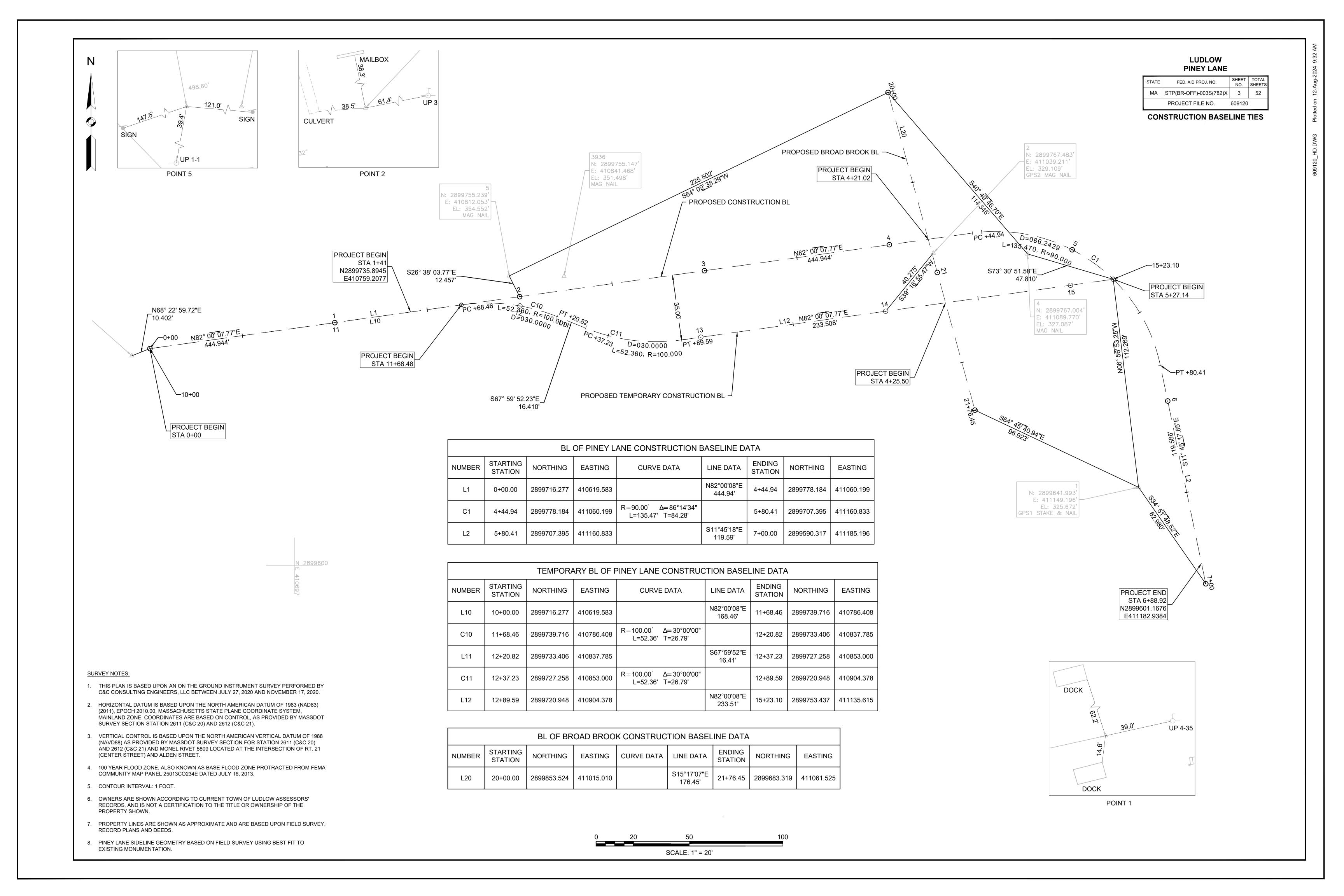
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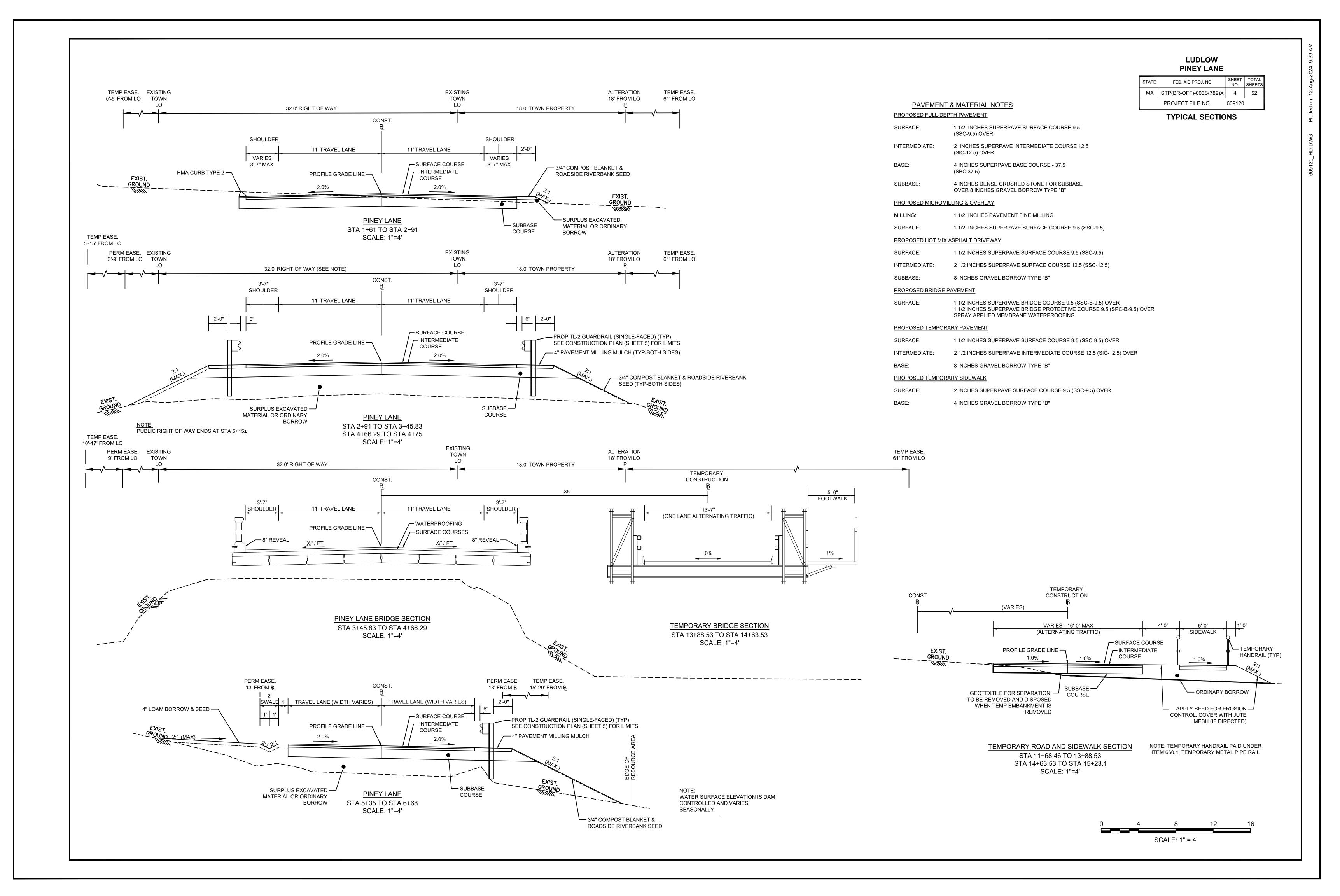
Carrie Jarallee Digitally signed by Carrie Lavallee, P.E. Date: 2024.09.16 11:30:23 -04'00' 09/16/2024

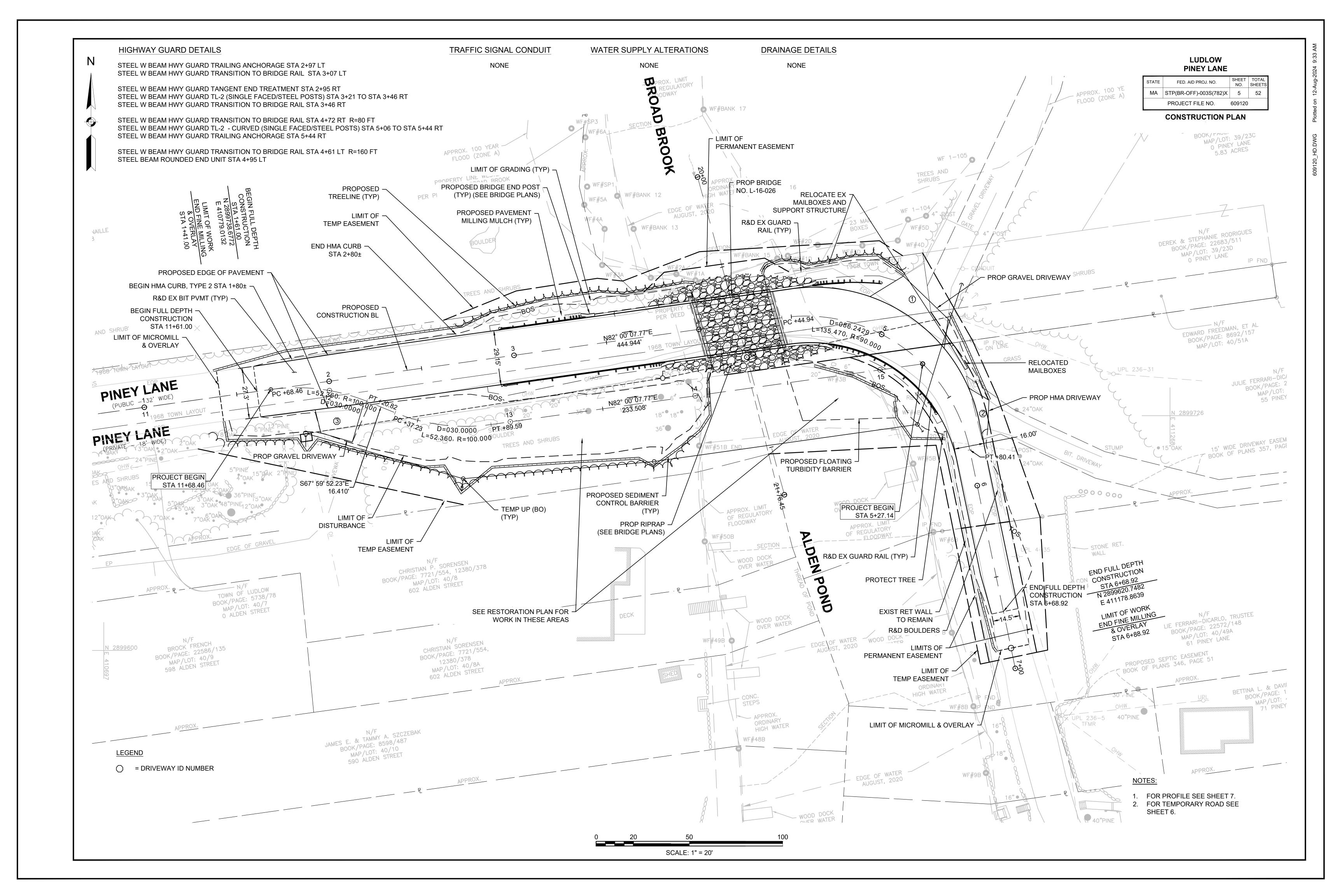
CHIEF ENGINEER

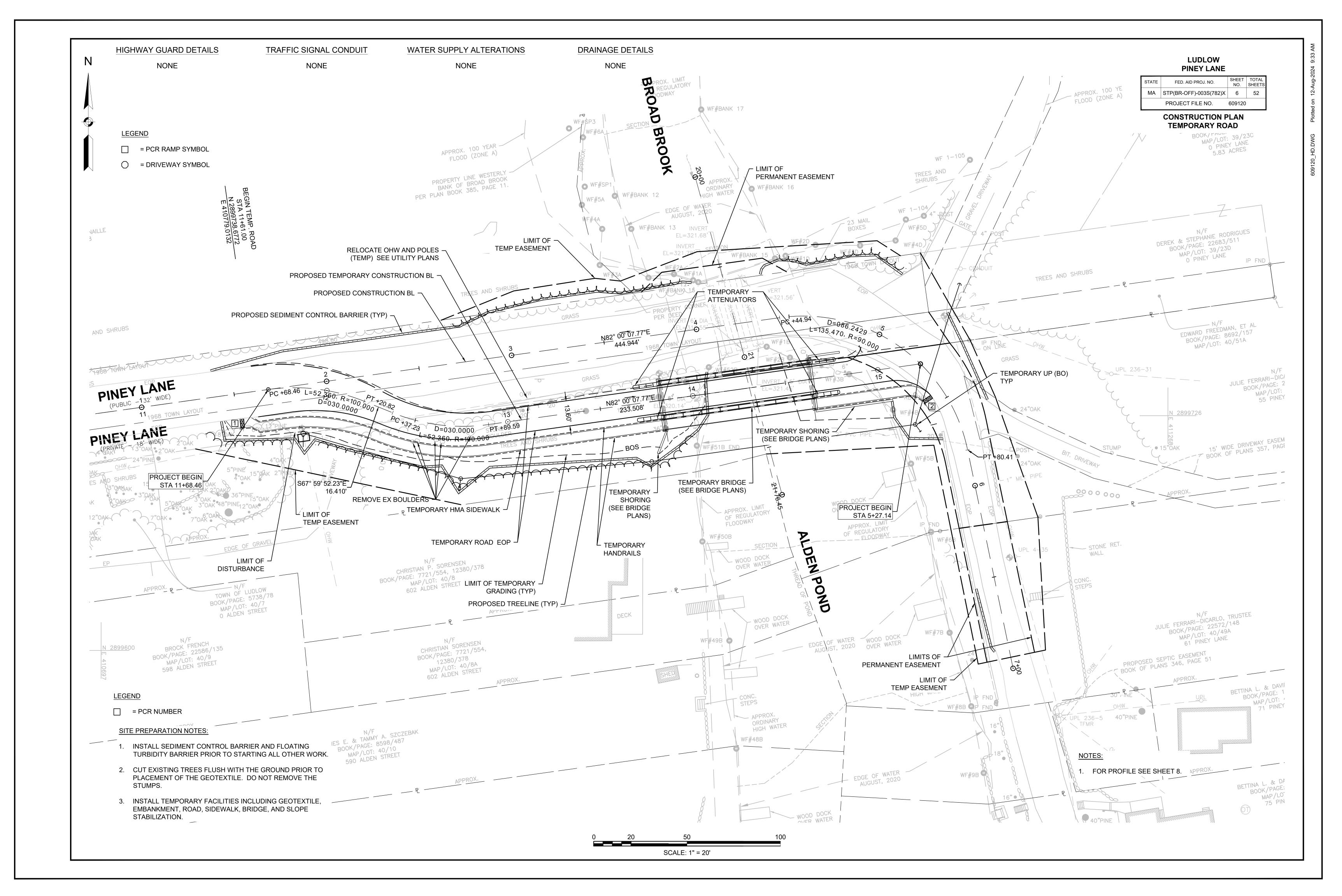
DATE

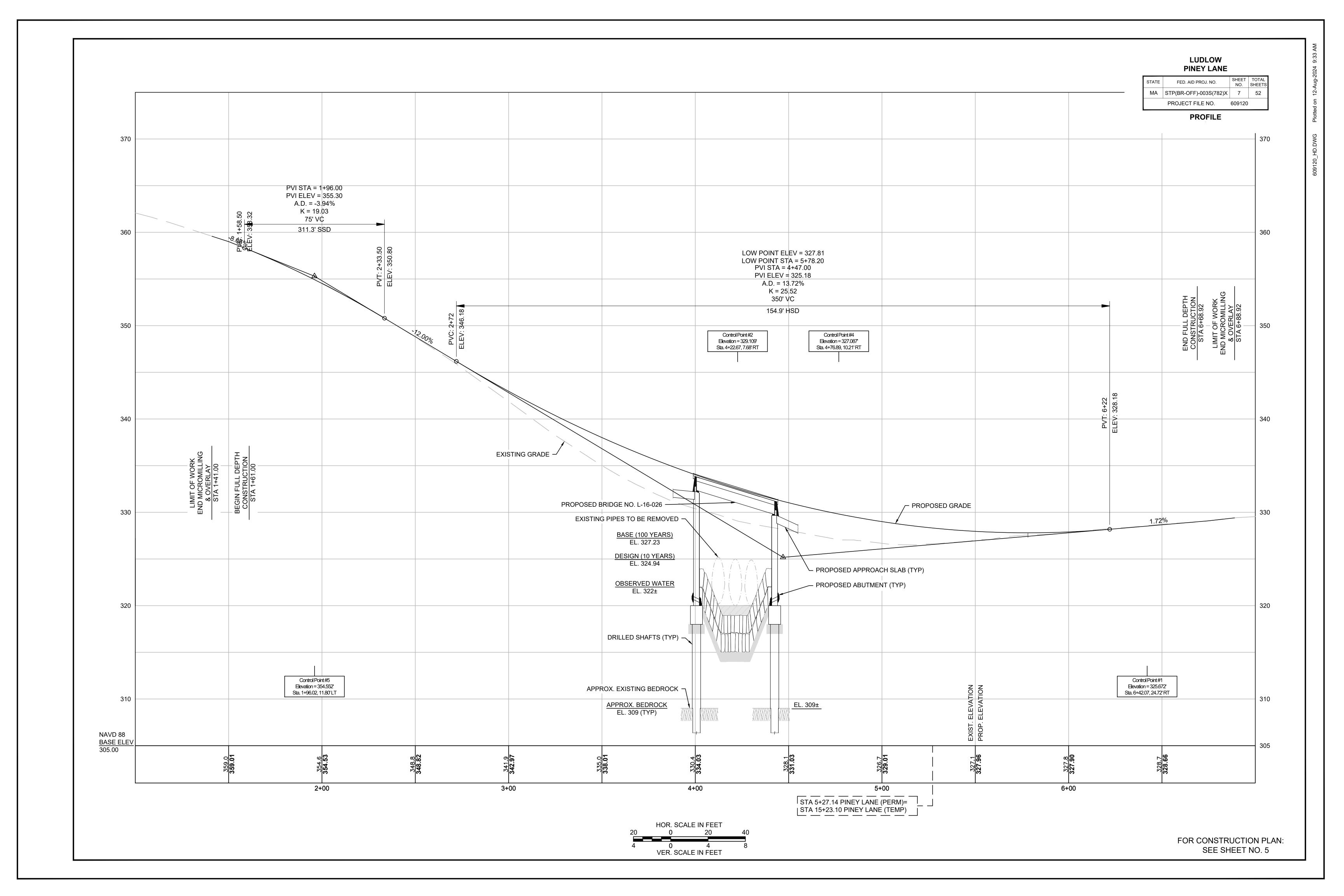
	SYMBOLS		TRAFFIC SY	MBOLS		ABBREV	/IATIONS	_	
EXISTING	PROPOSED	<u>DESCRIPTION</u>				GENERAL			LUDLOW PINEY LANE
☐ JB	<u></u> JB	JERSEY BARRIER	<u>EXISTING</u>	PROPOSED	DESCRIPTION	AADT	ANNUAL AVERAGE DAILY TRAFFIC		STATE FED. AID PROJ. NO. SHEETS
⊞⊕⊞св	СВ СВ	CATCH BASIN	Ø 1	Ø 1	CONTROLLER PHASE ACTUATED	ABAN ADJ	ABANDON ADJUST		
	(III)	CATCH BASIN CURB INLET		0		APPROX.	APPROXIMATE		MA STP(BR-OFF)-003S(782)X 2 52 PROJECT FILE NO. 609120
G GP	♥ FP G GP	FLAG POLE GAS PUMP		0	TRAFFIC SIGNAL HEAD (SIZE AS NOTED)	A.C.	ASPHALT CONCRETE		
□ MB	□ MB	MAIL BOX			MUDE LOOP RETECTOR (CL., CLTVR LINII EQQ OTHERWISE OREGIEIER)	ACCM PIPE	ASPHALT COATED CORRUGATED METAL PIPE		LEGEND & ABBREVIATIONS
		POST SQUARE		Ш	WIRE LOOP DETECTOR (6' x 6' TYP UNLESS OTHERWISE SPECIFIED)	BIT.	BITUMINOUS		
\bigcirc	0	POST CIRCULAR	77	7	VIDEO DETECTION CAMERA	BC	BOTTOM OF CURB		
⊕ WELL	⊕ WELL	WELL	$\triangleright\Box$	>=	MICROWAVE DETECTOR	BD. BL	BOUND BASELINE	45555	\
- EHH	□ EHH	ELECTRIC HANDHOLE	\oplus	•	PEDESTRIAN PUSH BUTTON, SIGN (DIRECTIONAL ARROW AS SHOWN) AND SADDLE	BLDG	BUILDING	ABBRE	VIATIONS (cont.)
o GG	O O GG	FENCE GATE POST GAS GATE	*	-		BM	BENCHMARK	GENERA	<u>L</u>
BHL #	BHL#	BORING HOLE	本	*	EMERGENCY PREEMPTION CONFIRMATION STROBE LIGHT	ВО	BY OTHERS	PVT	POINT OF VERTICAL TANGENCY
◆ MW #	→ MW #	MONITORING WELL	<	—	VEHICULAR SIGNAL HEAD	BOS	BOTTOM OF SLOPE	PVMT	PAVEMENT
■ TP #	T P#	TEST PIT	≪/——	₩—	VEHICULAR SIGNAL HEAD, OPTICALLY PROGRAMMED	BR.	BRIDGE	PWW	PAVED WATER WAY
Q	P	HYDRANT	◆	←	FLASHING BEACON	CB CBCI	CATCH BASIN CATCH BASIN WITH CURB INLET	R&D	RADIUS OF CURVATURE REMOVE AND DISPOSE
*	*	LIGHT POLE		-	PEDESTRIAN SIGNAL HEAD, (TYPE AS NOTED OR AS SPECIFIED)	CC	CEMENT CONCRETE	RCP	REINFORCED CONCRETE PIPE
□ CO.BD.		COUNTY BOUND				CCM	CEMENT CONCRETE MASONRY	RD	ROAD
		GPS POINT CABLE MANHOLE	⊠ RRSG	☑ RRSG	RAILROAD SIGNAL	CEM	CEMENT	RDWY	ROADWAY
(6)	© @	DRAINAGE MANHOLE		•	SIGNAL POST AND BASE (ALPHA-NUMERIC DESIGNATION NOTED)	CI	CURB INLET	REM	REMOVE
(E)	Œ	ELECTRIC MANHOLE	o—0	20'	MAST ARM, SHAFT AND BASE (ARM LENGTH AS NOTED)	CIP	CAST IRON PIPE	RET	RETAIN
G	<u> </u>	GAS MANHOLE			HIGH MAST POLE OR TOWER	CLF	CHAIN LINK FENCE	RET WALL ROW	RETAINING WALL RIGHT OF WAY
M	W	MISC MANHOLE				CL CMP	CENTERLINE CORRUGATED METAL PIPE	ROW	RAILROAD
S	S	SEWER MANHOLE		-0	SIGN AND POST	CSP	CORRUGATED METAL PIPE CORRUGATED STEEL PIPE	R&R	REMOVE AND RESET
(T)	① 	TELEPHONE MANHOLE	00	00	SIGN AND POST (2 POSTS)	CO.	COUNTY	R&S	REMOVE AND STACK
(W)	w	WATER MANHOLE		★ 20'	MAST ARM WITH LUMINAIRE	CONC	CONCRETE	RT	RIGHT
■ MHB □ MON	■ MHB	MASSACHUSETTS HIGHWAY BOUND MONUMENT		—	OPTICAL PRE-EMPTION DETECTOR	CONT	CONTINUOUS	SB	STONE BOUND
□ MON □ SB		STONE BOUND				CONST	CONSTRUCTION	SHLD SMH	SHOULDER SEWER MANHOLE
■ TB		TOWN OR CITY BOUND			CONTROL CABINET, GROUND MOUNTED	CR GR	CROWN GRADE	SMH ST	STREET
Δ		TRAVERSE OR TRIANGULATION STATION			CONTROL CABINET, POLE MOUNTED	DHV DI	DESIGN HOURLY VOLUME DROP INLET	STA	STATION
TPL or GUY	→ TPL or GUY				FLASHING BEACON CONTROL AND METER PEDESTAL	DIA	DIAMETER	SSD	STOPPING SIGHT DISTANCE
o HTP		TRANSMISSION POLE				DIP	DUCTILE IRON PIPE	SHLO	STATE HIGHWAY LAYOUT LINE
-b- UFB	- 占 - UFB	UTILITY POLE W/ FIREBOX		\boxtimes	LOAD CENTER ASSEMBLY	DW	STEADY DON'T WALK - PORTLAND ORANGE	SW	SIDEWALK
-∳- UPDL	-∳- UPDL	UTILITY POLE WITH DOUBLE LIGHT			PULL BOX 12"x12" (OR AS NOTED)	DWP	DETECTABLE WARNING PANEL	Т	TANGENT DISTANCE OF CURVE/TRUCK
-δ- ULT	-&- ULT	UTILITY POLE W / 1 LIGHT			ELECTRIC HANDHOLE 12"x24" (OR AS NOTED)	DWY	DRIVEWAY	TAN	TANGENT
-O- UPL	-∽ UPL	UTILITY POLE			= TRAFFIC SIGNAL CONDUIT		ELEVATION	TEMP	TEMPORARY
CIZE % TYPE		BUSH TREE				EMB	EMBANKMENT	TC TOS	TOP OF CURB TOP OF SLOPE
SIZE & TYPE		STUMP		•	REFLECTORIZED DRUM - 10' ON CENTER SPACING	EOP	EDGE OF PAVEMENT	TYP	TYPICAL
*		SWAMP / MARSH		_	SIGN MOUNTED ON BREAKAWAY POST	EXIST (or EX)		UP	UTILITY POLE
• WG	• WG	WATER GATE			SIGN MOUNTED ON TYPE III BARRICADE	EXC F&C	EXCAVATION FRAME AND COVER	VAR	VARIES
• PM	• PM	PARKING METER			WORK ZONE (APPROXIMATE)	F&G	FRAME AND GRATE	VERT	VERTICAL
		- OVERHEAD CABLE/WIRE				FDN.	FOUNDATION	VC	VERTICAL CURVE
					TEMPORARY BARRIER (TL-2)	FLDSTN	FIELDSTONE	WG	WATER GATE
		- CONTOURS (ON-THE-GROUND SURVEY DATA)				GAR	GARAGE	WIP WM	WROUGHT IRON PIPE WATER METER/WATER MAIN
100-99-		CONTOLIDE (DUOTOCDAMMETDIC DATA)				0.5	GROUND	VVIVI	
		- CONTOURS (PHOTOGRAMMETRIC DATA)				GD			CROSS SECTION
	-	- UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER)	PAVEMENT	MARKINGS S	YMBOLS	GG	GAS GATE	X-SECT	CROSS SECTION
		UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER)UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER)				GG Gl	GAS GATE GUTTER INLET		CROSS SECTION
		 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) 	PAVEMENT EXISTING	MARKINGS SY	YMBOLS <u>DESCRIPTION</u>	GG GI GIP	GAS GATE GUTTER INLET GALVANIZED IRON PIPE		CROSS SECTION
		 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER) 				GG GI GIP GRAN	GAS GATE GUTTER INLET GALVANIZED IRON PIPE GRANITE	X-SECT	
		 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) 			DESCRIPTION PAVEMENT ARROW - WHITE	GG GI GIP GRAN GRAV	GAS GATE GUTTER INLET GALVANIZED IRON PIPE	X-SECT	IC SIGNAL ABBREVIATIONS CABINET
		 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER) 			DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE	GG GI GIP GRAN	GAS GATE GUTTER INLET GALVANIZED IRON PIPE GRANITE GRAVEL	X-SECT TRAFFI	IC SIGNAL ABBREVIATIONS
		 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER) UNDERGROUND WATER MAIN (DOUBLE LINE 24 INCH AND OVER) 		PROPOSED The second se	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE	GG GI GIP GRAN GRAV GRD HDW HMA	GAS GATE GUTTER INLET GALVANIZED IRON PIPE GRANITE GRAVEL GUARD	TRAFFI CAB CCVE DW	IC SIGNAL ABBREVIATIONS CABINET
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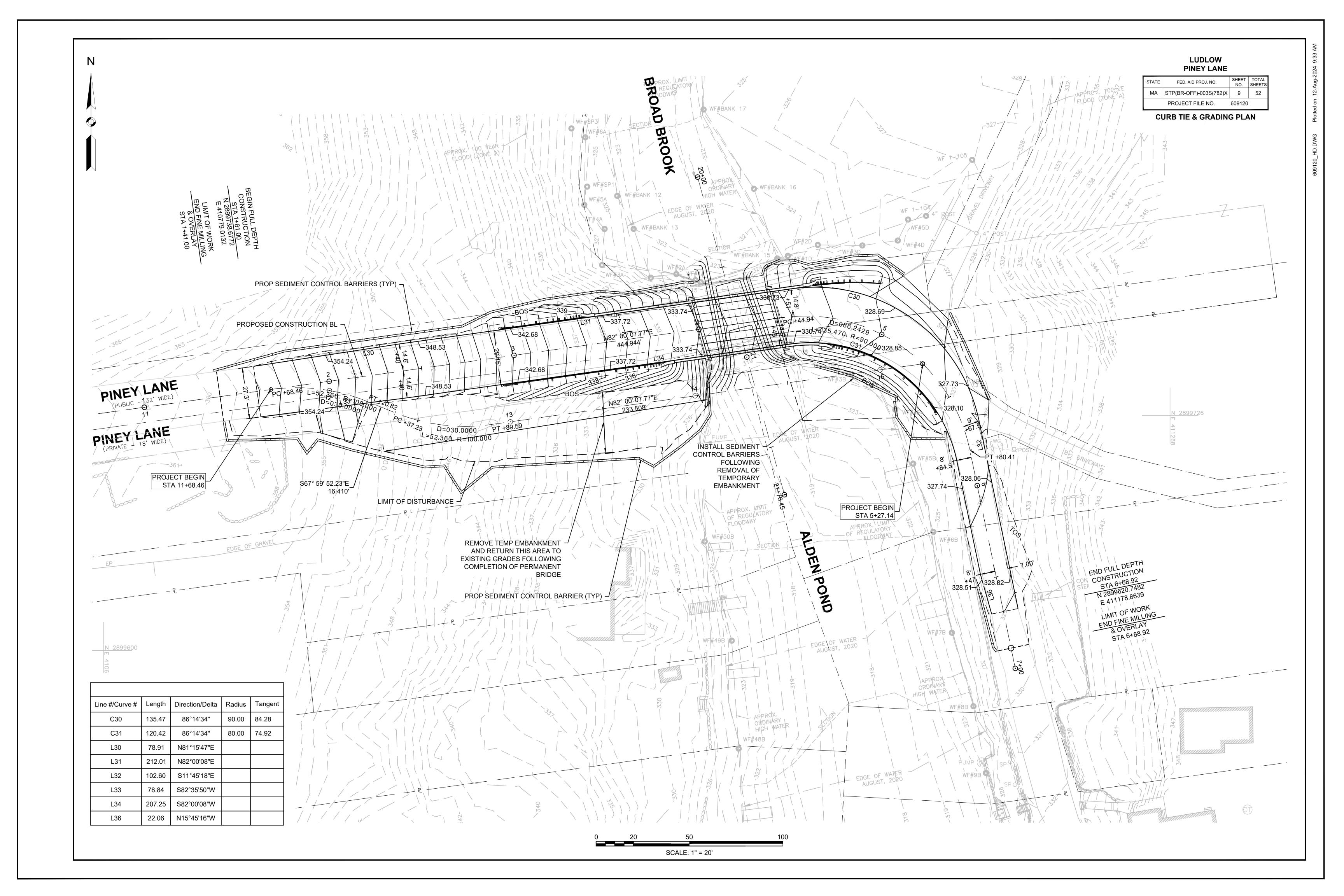


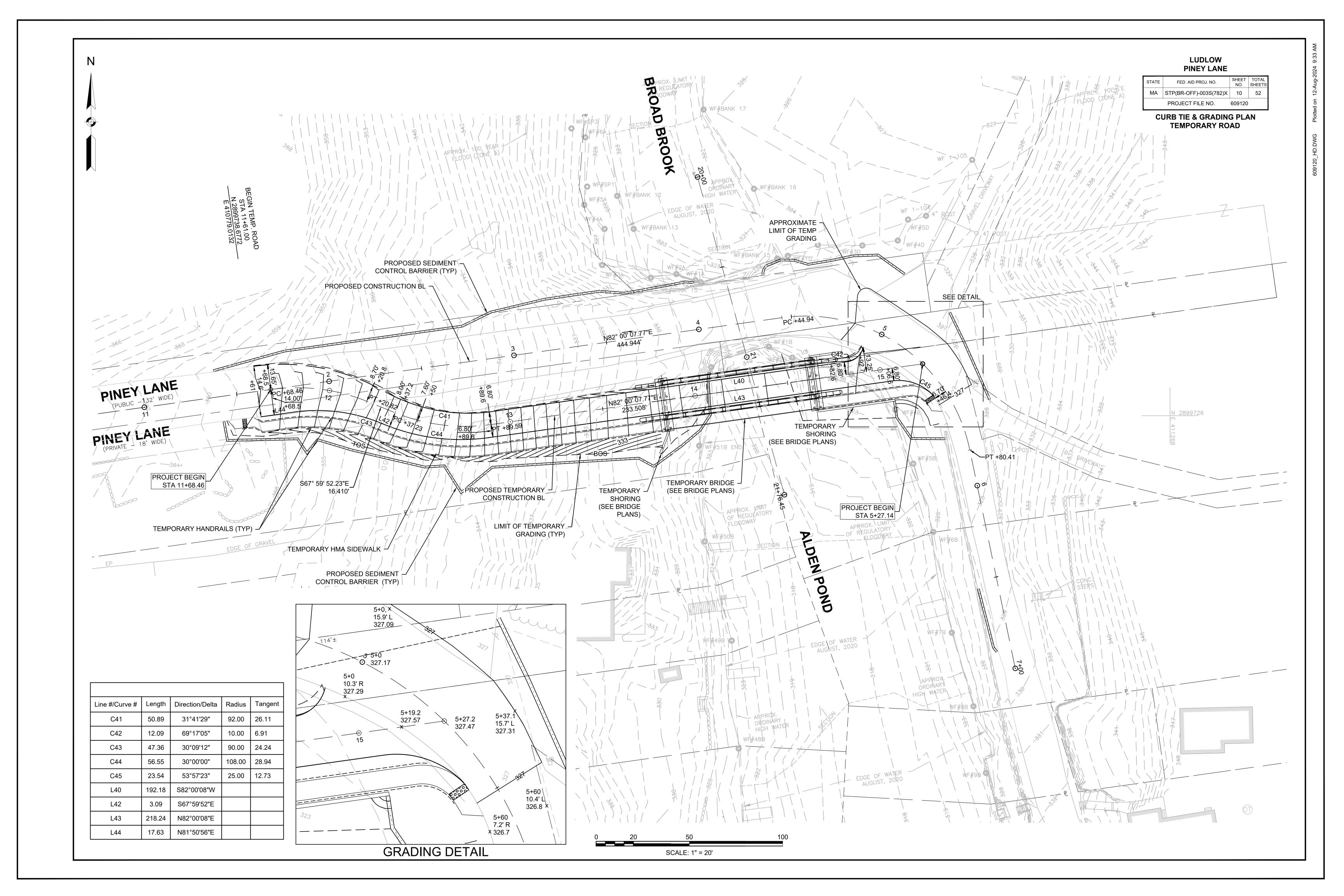


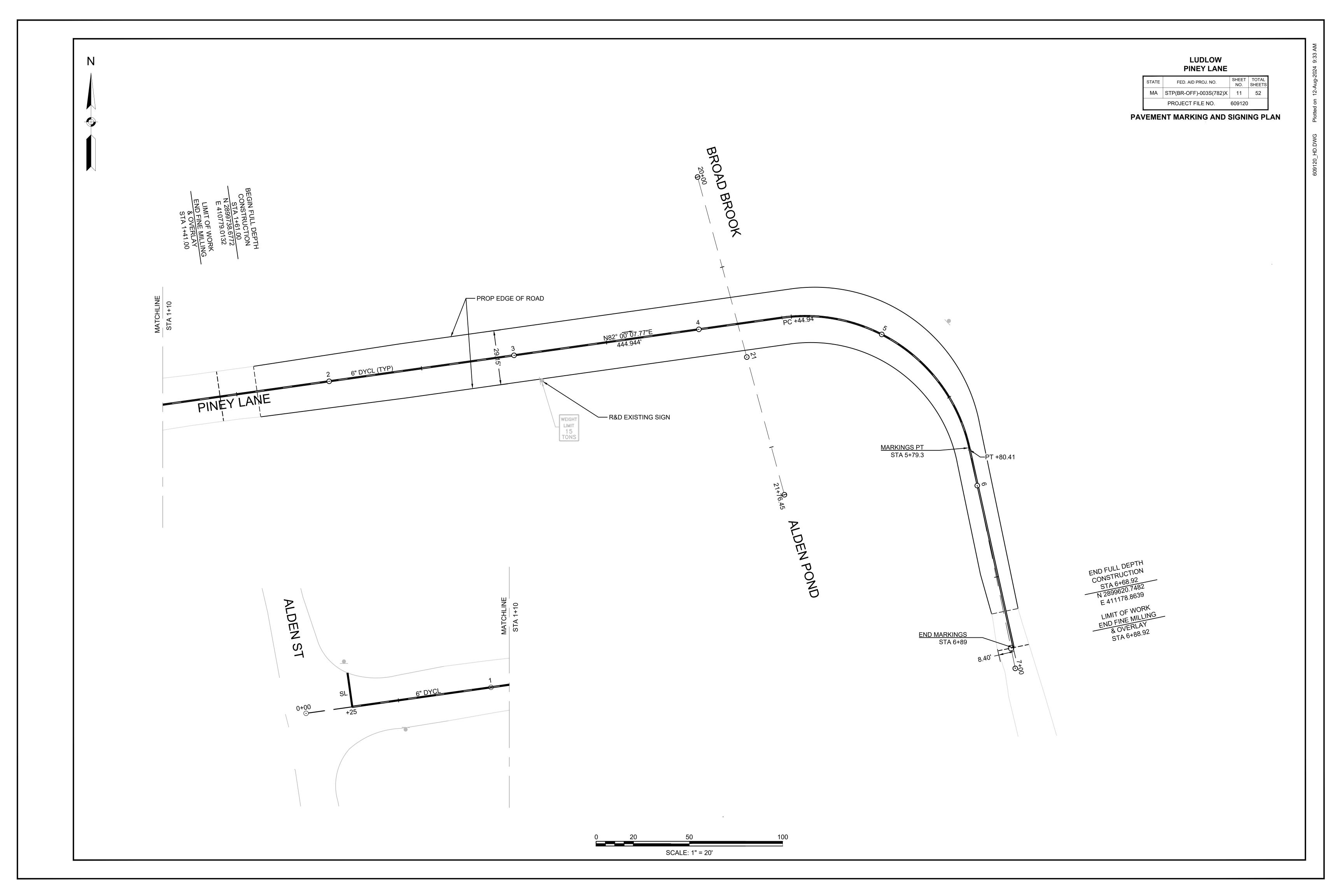


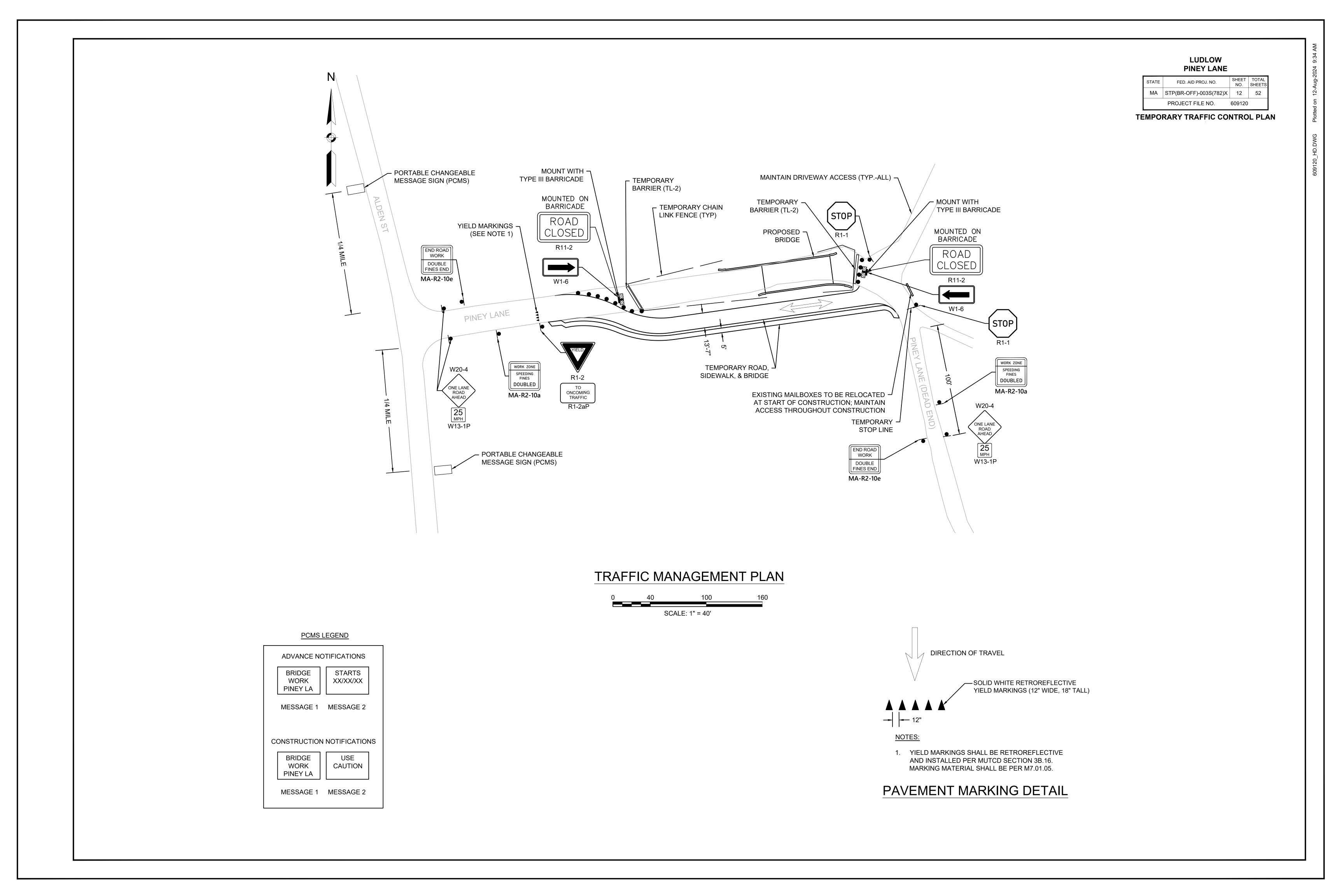












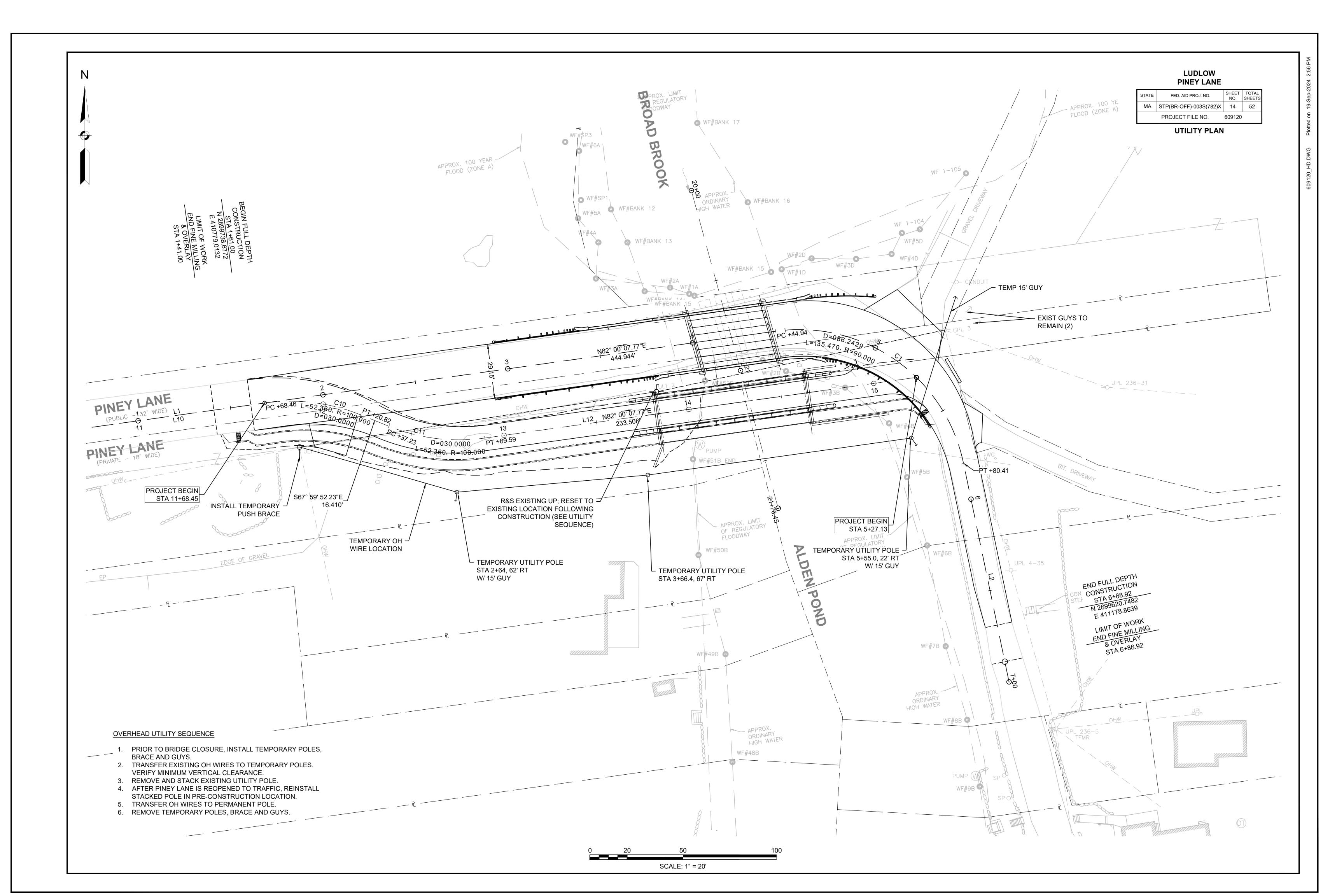
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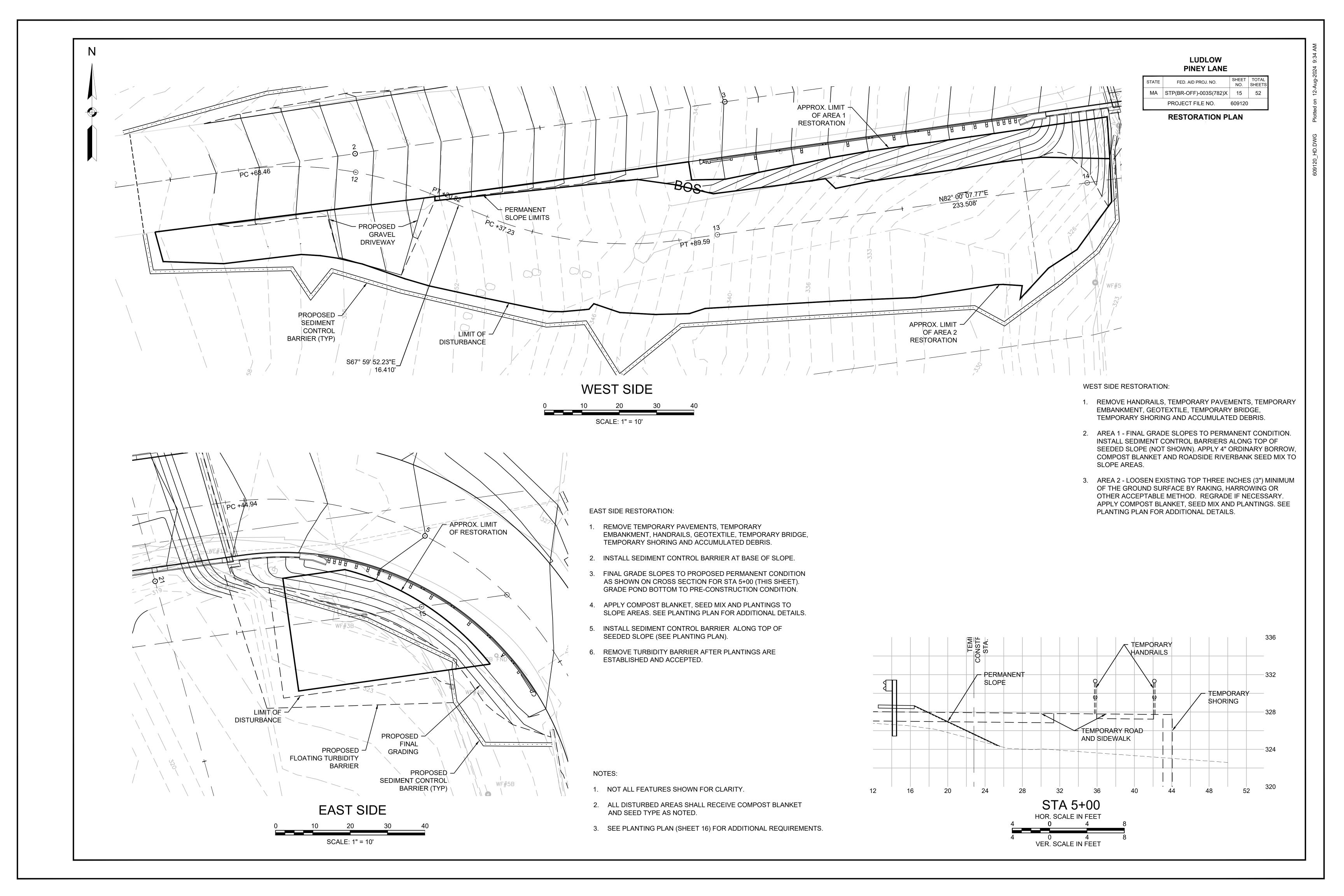
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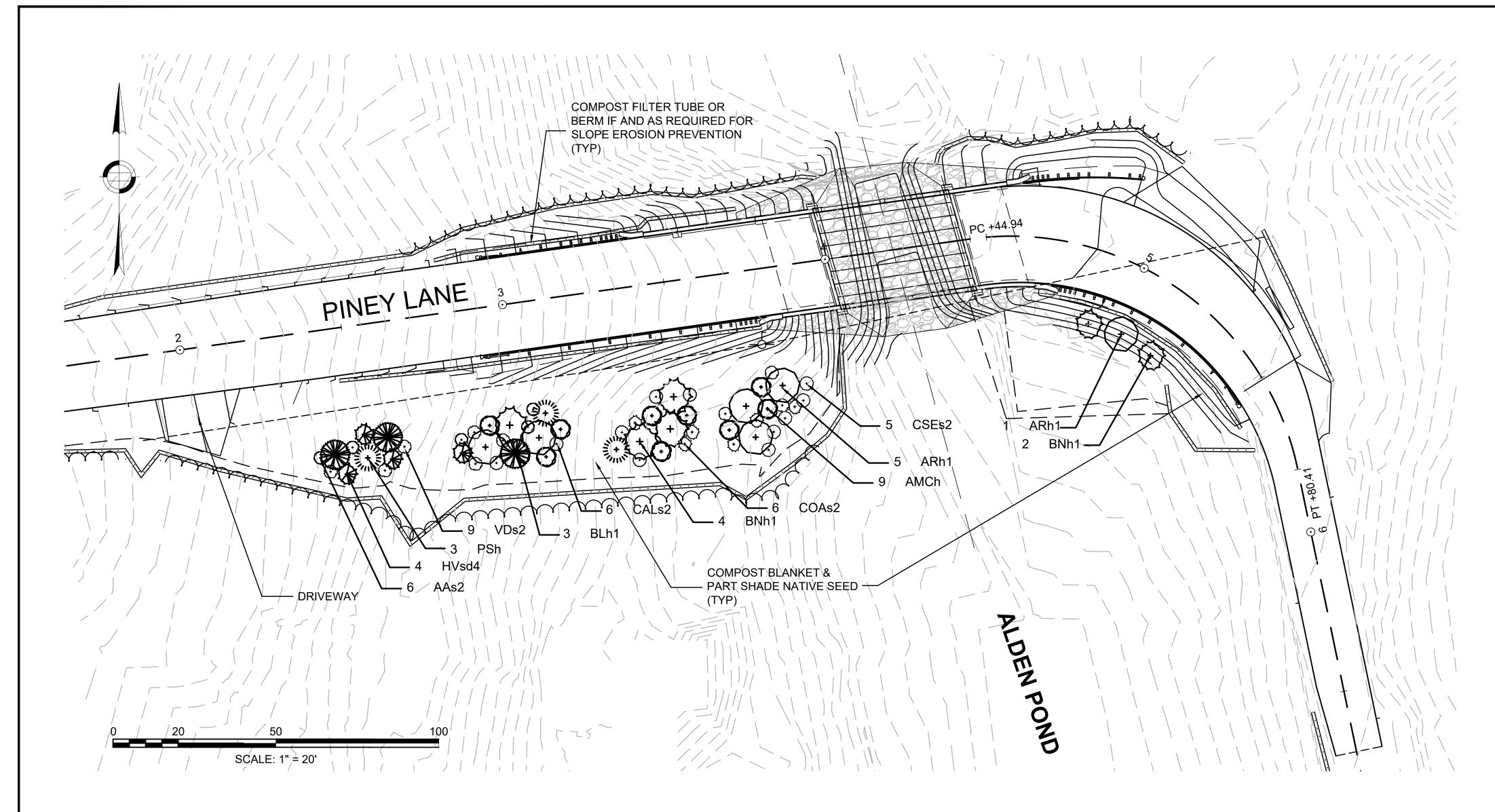
TEMPORARY TRAFFIC CONTROL DETAILS

TEMPORARY TRAFFIC CONTROL NOTES

- 1. ALL TEMPORARY TRAFFIC CONTROL WORK SHALL CONFORM TO THE LATEST EDITION OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" (MUTCD) AND ALL REVISIONS AND MASSDOT AMENDMENTS AS WELL AS THE LATEST EDITION OF THE MASSDOT STANDARD SIGNS MANUAL (UNLESS SUPERCEDED BY THESE PLANS).
- 2. ALL SIGN LEGENDS, BORDERS, AND MOUNTING SHALL BE IN ACCORDANCE WITH THE MUTCD.
- 3. TEMPORARY CONSTRUCTION SIGNING AND ALL OTHER TRAFFIC CONTROL DEVICES SHALL BE IN PLACE PRIOR TO THE START OF ANY WORK.
- 4. TEMPORARY CONSTRUCTION SIGNING, BARRICADES, AND ALL OTHER NECESSARY WORK ZONE TRAFFIC CONTROL DEVICES SHALL BE REMOVED FROM THE HIGHWAY OR COVERED WHEN THEY ARE NOT REQUIRED FOR CONTROL OF TRAFFIC.
- 5. SIGNS AND SIGN SUPPORTS LOCATED ON OR NEAR THE TRAVELED WAY, CHANNELIZING DEVICES, BARRIERS, AND CRASH ATTENUATORS MUST PASS THE CRITERIA SET FORTH IN NCHRP REPORT 350, "RECOMMENDED PROCEDURES FOR THE SAFETY PERFORMANCE EVALUATION OF HIGHWAY FEATURES" AND/OR "MANUAL FOR ASSESSING SAFETY HARDWARE" (MASH).
- 6. CONTRACTORS SHALL NOTIFY EACH ABUTTER AT LEAST 24 HOURS IN ADVANCE OF THE START OF ANY WORK THAT WILL REQUIRE THE TEMPORARY CLOSURE OF ACCESS, SUCH AS CONDUIT INSTALLATION, EXISTING PAVEMENT EXCAVATION, TEMPORARY DRIVEWAY PAVEMENT PLACEMENT, AND SIMILAR OPERATIONS.
- 7. THE FIRST TEN (10) PLASTIC DRUMS OF A TAPER SHALL BE MOUNTED WITH SEQUENTIAL FLASHING LIGHTS.
- 8. THE ADVISORY SPEED LIMIT, IF REQUIRED, SHALL BE DETERMINED BY THE ENGINEER.
- 9. DISTANCES ARE A GUIDE AND MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER.
- 10. MAXIMUM SPACING OF TRAFFIC DEVICES IN A TAPER (DRUMS OR CONES) IS EQUAL IN FEET TO THE SPEED LIMIT IN MPH.
- 11. MINIMUM LANE WIDTH IS TO BE 11 FEET (3.3m) UNLESS OTHERWISE SHOWN. MINIMUM LANE WIDTH TO BE MEASURED FROM THE EDGE OF DRUMS OR MEDIAN BARRIER.
- 12. ALL SIGNS SHALL BE MOUNTED ON THEIR OWN STANDARD SIGN SUPPORTS UNLESS NOTED.
- 13. ALL CHANNELIZING DEVICES (INCLUDING DRUMS/CONES) SHALL BE A MINIMUM HEIGHT OF 36".
- 14. MAINTAIN ACCESS TO ALL PROPERTIES THAT ABUT THE WORK ZONE UNLESS TEMPORARY CLOSURE IS REQUIRED. SEE NOTE 6 FOR TEMPORARY CLOSURE REQUIREMENTS.
- 15. PEDESTRIAN ACCESS THAT MEETS APPLICABLE REGULATIONS SHALL BE MAINTAINED AT ALL TIMES. PEDESTRIAN DETOURS OR BYPASSES SHALL INCLUDE PROPER MARKINGS, SIGNAGE, BARRICADES, RAILINGS, ETC.







PL	ANT	LIST

SYM TREES	QTY	SPECIES	SIZE	NOTES
ARh1	6	Maple — Red Acer rubrum	5-6 FT	
AMCh	9	Serviceberry — Shadblow Amelanchier canadensis	4-5 FT	CLUMP
BLh1	3	Birch — Cherry Betula lenta	5-6 FT	CLUMP
BNh1	6	Birch — River Betula nigra	5-6 FT	CLUMP
HVsd4	4	Witchhazel — Common Hamamelis virginiana	3-4 FT	
PSh	3	Cherry — Black Prunus serotina	4-5 FT	
AAs2	6	Red Chokeberry Aronia arbutifolia	2-2.5 FT	
CALs2	6	Summersweet Clethra alnifolia	2-2.5 FT	
COAs2	6	Dogwood — Silky Cornus amomum	2-2.5 FT	
CSEs2	5	Dogwood — Redosier Cornus redosier	2-2.5 FT	
VDs2	9	Viburnum — Arrowwood Viburnum dentatum	2-2.5 FT	

LEGEND

- ARONIA ABUTIFOLIA
- CORNUS AMOMUM
- CORNUS REDOSIER CLETHRA ALNIFOLIA
- VIBURNUM DENTATUM

HAMAMELIS VIRGINIANA • AMELANCHIER CANADENSIS

+ BETULA NIGRA

PRUNUS SEROTINA

BETULA LENTA

+ ACER RUBRUM

LUDLOW **PINEY LANE**

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(782)X	16	52
	PROJECT FILE NO.	609120	

PLANTING PLAN

SITE PREPARATION

- 1. PRIOR TO ORDERING PLANTS AND PRIOR TO APPLICATION OF COMPOST AND SEED, SOIL AND SITE PREPARATION SHALL BE INSPECTED AND APPROVED BY THE ENGINEER.
- 2. IF REQUIRED BY THE ENGINEER, COMPOST SEDIMENT BARRIER OR COMPOST BLANKET BERM (5-6 INCHES) SHALL BE PLACED AT THE TOP OF SLOPE TO SLOW AND ABSORB FLOW FROM ROADWAY UNTIL SEED ESTABLISHES.

PLANTING NOTES

- 1. PLANT LOCATIONS ARE SCHEMATIC ONLY. PRIOR TO PLANTING, LOCATION OF ALL PLANT MATERIAL MUST BE APPROVED BY THE MASSDOT LANDSCAPE ARCHITECT.
- 2. ALL PLANT MATERIAL WILL HAVE TAGS INDICATING COMMON NAME, BOTANICAL NAME, CULTIVAR, & SIZE.
- 3. IMMEDIATELY AFTER ACCEPTANCE OF PLANTING, TAGS AND RIBBONS SHALL BE REMOVED.
- 4. ALL PLANTS WILL BE MULCHED PER PLANS AND SPECIFICATIONS.
- 5. ALL SHRUB AND PERENNIAL BEDS WILL BE WEEDED AND OTHERWISE NEATLY MAINTAINED FOR THE DURATION OF THE CONTRACT.
- 6. PLANTS AND PLANTING BEDS SHALL BE THOROUGHLY WATERED AS NECESSARY AND PER SPECIFICATIONS.
- 7. PLANTS INCLUDED IN THE CONTRACT BUT NOT SHOWN ON THE PLAN SHALL BE LOCATED AS REQUIRED BY THE MASSDOT LANDSCAPE ARCHITECT.

UPLAND NATIVE SEEDING NOTES

- 1. ALL DISTURBED AREAS OTHER THAN AREAS OF EXISTING LAWN SHALL BE SEEDED WITH THE MASSDOT PART SHADE MIX.
- 2. SEEDING SHALL BE BROADCAST METHOD ONLY (NOT HYDROSEED) UNLESS APPROVED OTHERWISE BY THE MASSDOT LANDSCAPE ARCHITECT.
- 3. SEEDING AND SUBMITTALS SHALL BE PER THE SPECIAL PROVISIONS.
- 4. SUBMITTALS FOR SEED MIXES SHALL BE APPROVED BY THE ENGINEER AND LANDSCAPE ARCHITECT PRIOR TO SEED APPLICATION.
- 5. SITE PREPARATION SHALL BE PER SPECIFICATIONS AND APPROVED BY THE ENGINEER PRIOR TO SEEDING.
- 6. WHEN SEEDING OUT OF SEASON APPLICATION RATE SHALL BE INCREASED BY 50%.

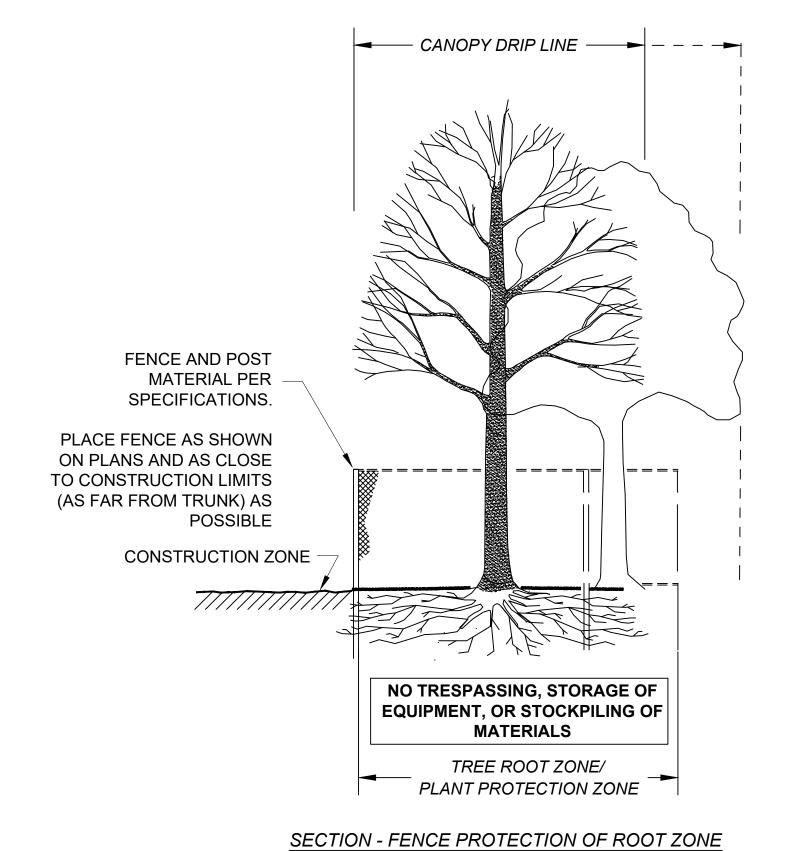
COMPOST FILTER TUBES

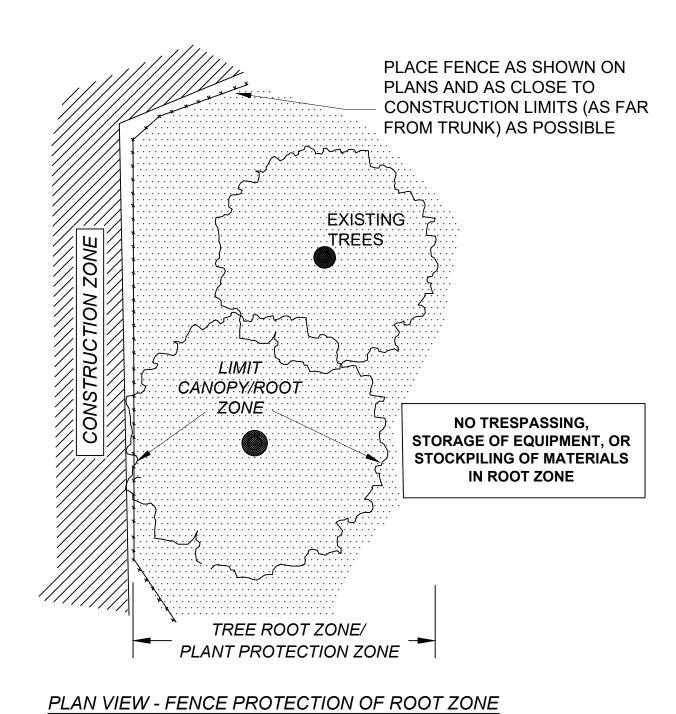
- 1. COMPOST FILTER TUBES (OR SIMILAR BARRIER APPROVED BY THE ENGINEER) SHOWN ON LANDSCAPE PLAN SHALL BE LEFT TO DEGRADE OVER TIME AND THEREFORE MUST BE COMPRISED OF 100% BIODEGRADABLE MATERIALS - BURLAP, COTTON, HEMP, ETC.
- 2. USE SHALL BE IF AND WHERE NEEDED AND AS APPROVED BY THE ENGINEER. PLACEMENT SHALL BE WHERE MOST EFFECTIVE TO REDUCE POTENTIAL EROSION OF SLOPES UNTIL VEGETATION ESTABLISHES.

PINEY LANE FED. AID PROJ. NO. MA STP(BR-OFF)-003S(782)X 17 52 PROJECT FILE NO.

LUDLOW

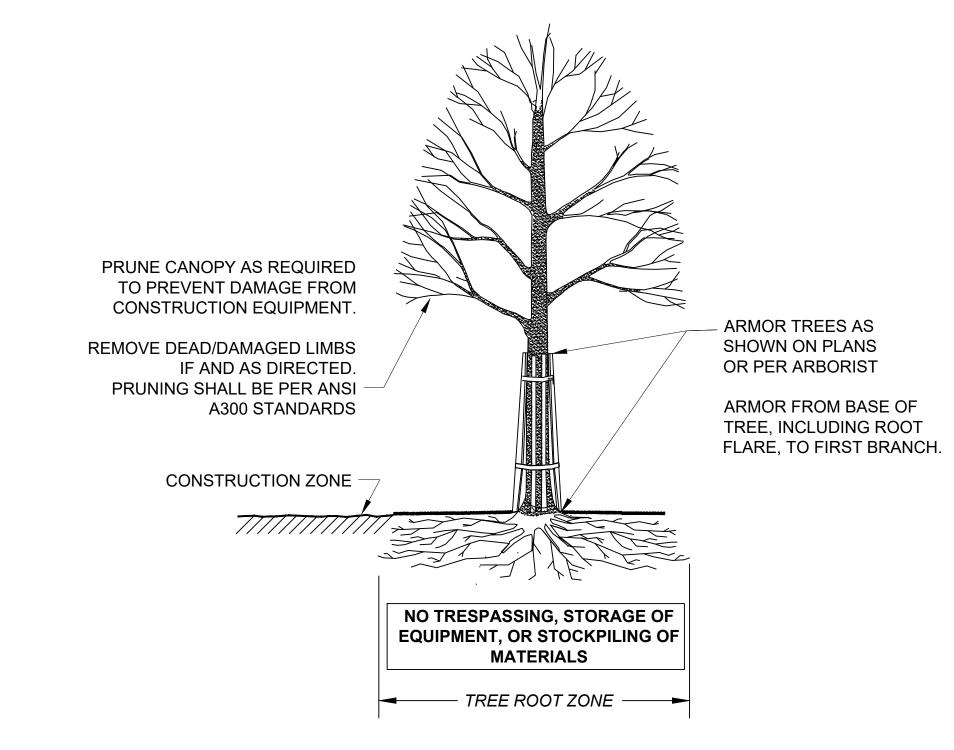
CONSTRUCTION DETAILS I (SHEET 1 OF 3)





TREE PROTECTION - ROOT ZONE

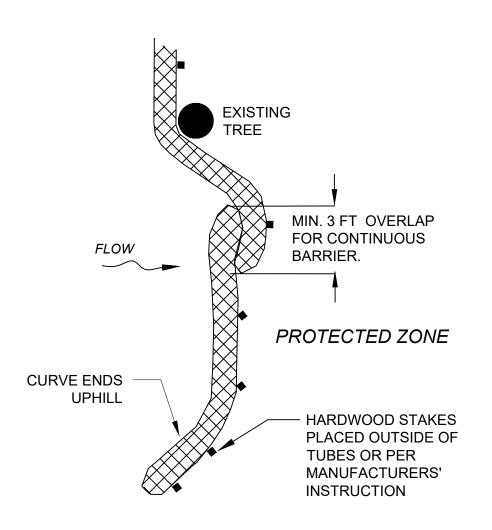
SCALE: NOT TO SCALE



SECTION - TRUNK ARMORING & PRUNING

TREE PROTECTION - TRUNK

SCALE: NOT TO SCALE

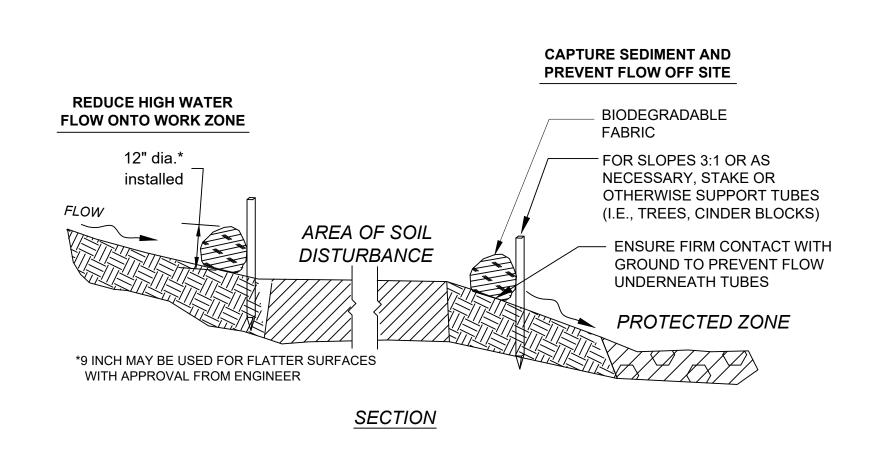


NOTES:

PLAN VIEW

- 1. PLACE TUBE ALONG CONTOURS AND PERPENDICULAR TO FLOW.
- 2. PLACE AS CLOSE TO LIMIT OF SOIL DISTURBANCE AS POSSIBLE
- 3. ADJUST LOCATION AS REQUIRED FOR OPTIMUM EFFECTIVENESS. DO NOT INSTALL IN WATERWAYS.
- 4. PLACE STAKES AS NEEDED TO SECURE TUBES IN PLACE.

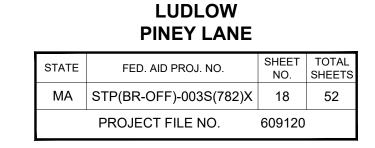
COMPOST FILTER TUBE SCALE: NOT TO SCALE



SEDIMENT BARRIER - COMPOST FILTER TUBES SCALE: NOT TO SCALE

5' MIN. OVERLAP FLOW PROTECTED ZONE PLAN VIEW 1" X 1" X 4' HARDWOOD STAKES 9-12" STAKE A MIN. OF EVERY 5 FEET TO DIA. SECURE TUBE OR PER INSTALLED MANUFACTURERS' INSTRUCTION AREA OF SOIL DISTURBANCE PROTECTED ZONE

COMPOST FILTER TUBE BERM (SLOPES 2:1 OR STEEPER) SCALE: NOT TO SCALE

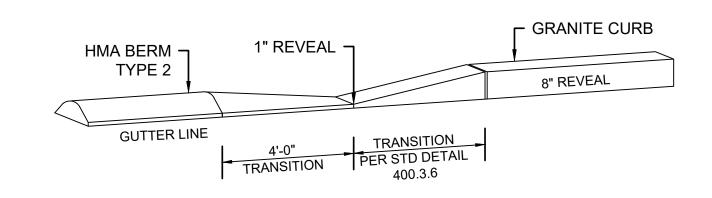


CONSTRUCTION DETAILS II (SHEET 2 OF 3)

DRIVEWAY

REPLACEMENT

LENGTH (FT)



HMA BERM TRANSITION TO GRANITE CURB STA. 3+23 LT

DRIVEWAY DETAILS

DRIVEWAY ID

SCALE: NOT TO SCALE

APPLY TACK COAT OF RS-1-**EMULSION TO VERTICAL FACE** OF EXISTING PAVEMENT PRIOR TO PLACEMENT OF NEW HMA PAVEMENT - SAWCUT EXISTING BITUMINOUS PAVEMENT IN A NEAT LINE. NEW SUPERPAVE -SURFACE COURSE CUT DEPTH TO EQUAL MILLING DEPTH +0.5 INCHES. EXISTING BASE SURFACE MATERIAL WATERIAL EXISTING GROUND -PAVEMENT MATCH/

MILLED SURFACE DETAIL

SCALE: NOT TO SCALE

FLOATING TURBIDITY BARRIER

NOTE: TURBIDITY BARRIER NOT TO BE USED IN

CLOSED CELL FOAM FLOTATION

PERMEABLE FABRIC

BOTTOM BALLAST CHAIN

SCALE: NOT TO SCALE

MOVING WATER

STRESS PLATES WITH SHACKLE CONNECTION

TOP TENSION CABLE

REINFORCED GROMMET

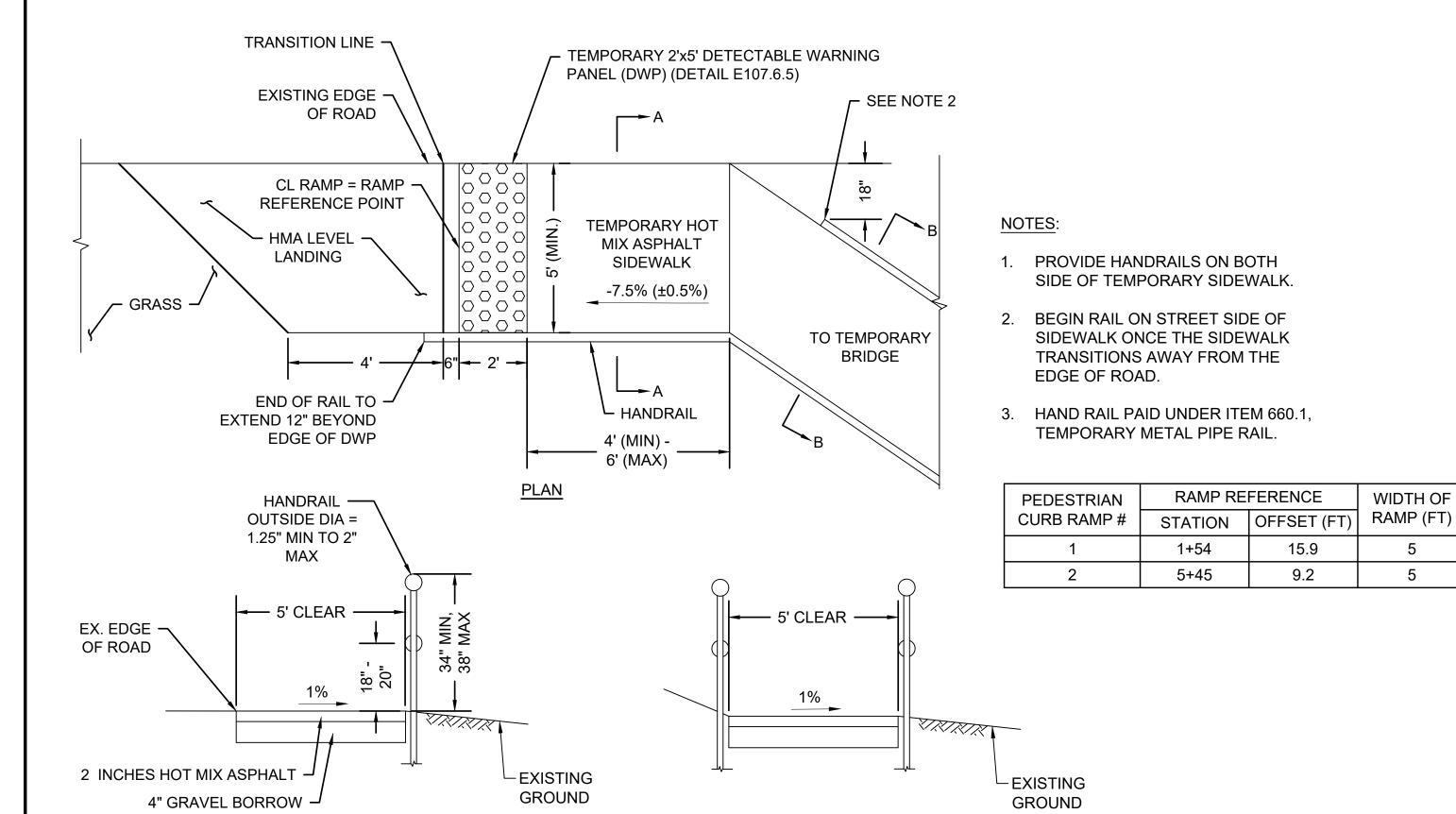
SECTION CONNECTORS

STEEL SAFETY

SECTION "A-A"

CONNECTION

PLATES



TEMP PEDESTRIAN BRIDGE

ACCESS DETAIL

SCALE: NOT TO SCALE

SECTION "B-B"

APPROXIMATE LIMIT
OF GRADING

EX DRIVEWAY
TO REMAIN

EX DRIVEWAY TO
BE REPLACED

© = APRON REFERENCE POINT ROADWAY DOWNGRADE

EXISTING DRIVEWAY

TO REMAIN TO BE REPLACED ROADWAY

(LENGTH VARIES)

SEE TYPICAL SECTIONS
FOR FULL DEPTH
PAVEMENT DETAILS

SURFACE TREATMENT
(SEE TABLE)

8" COMPACTED
GRAVEL BORROW

SECTION "A-A"

<u>LEGEND</u>

RAMP REFERENCE

STATION

5+62

2+03

OFFSET (FT)

15.6 LT

9.8 LT

14.0 RT

* = CONSTRUCTION TOLERANCE ±0.5% HSL = HIGH SIDE TRANSITION LENGTH CC = CEMENT CONCRETE

= DRIVEWAY SYMBOL

DRIVEWAY

TYPE

 HMA

GRAVEL

GRAVEL

HMA = HOT MIX ASPHALT

= PCR RAMP SYMBOL

NOTE:

1. VERTICAL FACES OF ALL ASPHALT JOINTS SHALL RECEIVE TACK COAT.

2. SEE DRIVEWAY DETAILS CHART FOR ASSOCIATED DIMENSIONS.

ROADWAY PROFILE GRADE	HIGH SIDE TRANSITION LENGTH (HSL)
=0%	6'-6"
>0% TO 1%	7'-8"
>1% TO 2%	9'-0"
>2% TO 3%	11'-0"
>3% TO 4%	14'-0"
>4% TO 6%	15'-0" MAX.
	·

DF	RIVEWAY TREAT	SURFACE MENT
DRIVEWA	Y TYPE	MATERIALS & THICKNESS
НМ	A	1 1/2" HMA SURFACE COURSE OVER 2 1/2" INTERMEDIATE COURSE
GRA	√EL	4" GRAVEL BORROW

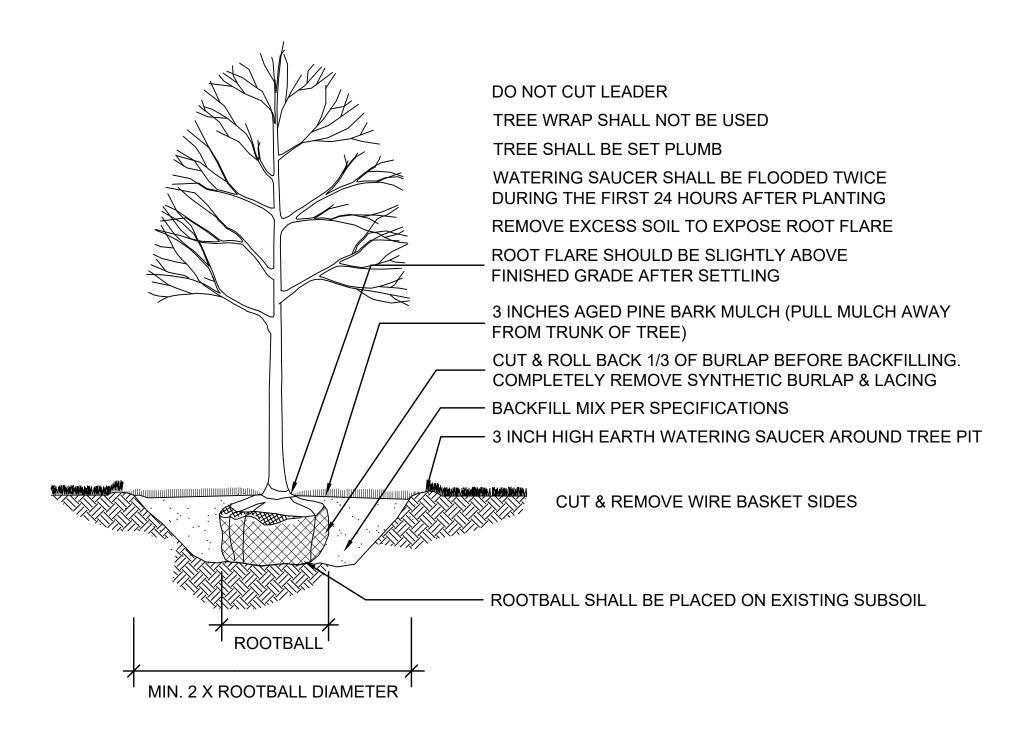
DRIVEWAY DETAILS

SCALE: NOT TO SCALE

LUDLOW PINEY LANE

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(782)X	19	52
	PROJECT FILE NO.	609120	

CONSTRUCTION DETAILS III (SHEET 3 OF 3)

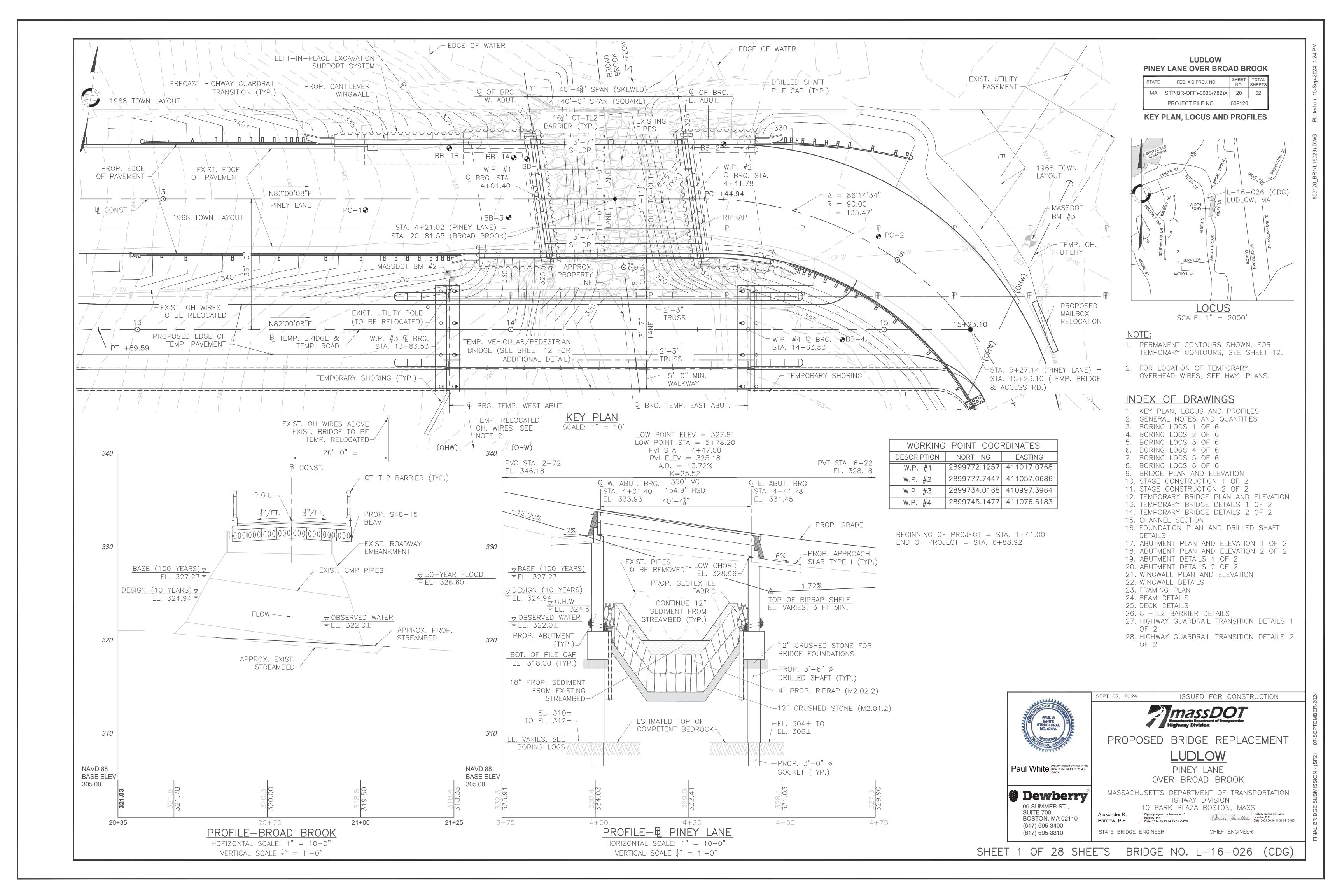


DECIDUOUS TREE PLANTING

SCALE: NOT TO SCALE

WATER BY FLOODING TWICE IN FIRST TWO HOURS EXCAVATE TO REQUIRED DEPTH AFTER PLANTING. WATER & MAINTAIN AS PER AND BACKFILL WITH PLANTING MIX STANDARD SPECIFICATIONS SHRUBS SHALL BE SET PLUMB CROWN OF SHRUB SHOULD BE SLIGHTLY ABOVE FINISHED GRADE AFTER SETTLING 2-3 INCH DEPTH AGED PINE BARK MULCH (PULL AWAY FROM BASE OF SHRUB) — BACKFILL MIX PER STANDARD SPECIFICATIONS 3 INCH HIGH EARTH WATERING SAUCER AROUND SHRUB COMPLETELY REMOVE SYNTHETIC BURLAP AND LACING. FOR CONTAINERIZED PLANTS, REMOVE - CONTAINER PRIOR TO PLANTING. SCORE SIDES OF CONTAINER AND LOOSEN ANY ROOTS ENCIRCLING THE ROOT BALL. - EXISTING SUBSOIL LOOSE OR CRACKED ROOTBALLS WILL 3 X ROOTBALL NOT BE ACCEPTED FOR PLANTING

SHRUB PLANTING
SCALE: NOT TO SCALE



DESIGN:

IN ACCORDANCE WITH THE 2020 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS FOR HL-93 LOADING.

MASSDOT BENCHMARK:

BM #1: MAG NAIL - SET UP 1FT UP IN FRONT OF ULT #1-1 N 2899718.90 E 410804.47 EL. 357.25 STA. 1+83.46, OFFSET 23.13 FT (RT)

BM #2: MAG NAIL - SET UP 1FT UP IN FRONT OF ULT #2 N 2899748.33 E 410995.63 EL. 333.49 STA. 3+76.86, OFFSET 20.58 FT (RT)

BM #3: MAG NAIL - SET UP 1FT UP IN FRONT OF ULT #3 N 2899779.25 E 411148.27 EL. 328.46 STA. 5+20.26, OFFSET 27.66 FT (LT)

ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

DATE:

TO BE PLACED ON THE INSIDE FACE OF THE NORTHEAST AND SOUTHWEST HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

MASSDOT SURVEY NOTEBOOKS:

SURVEY PERFORMED BY C&C CONSULTING ENGINEERS, 1380 SOLDIERS FIELD ROAD BOSTON, MA 02135 BETWEEN JULY 27, 2020 AND NOVEMBER 17, 2020, AND AGAIN IN MARCH OF 2023. COPIES OF THE FILES MAY BE OBTAINED FROM THE MASSACHUSETTS DEPARTMENT OF TRANSPORTATION.

SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS (A3).

SEISMIC GROUND SHAKING HAZARD:

SEISMIC GROUND SHAKING HAZARD IN ACCORDANCE WITH THE 2011 AASHTO GUIDE SPECIFICATIONS FOR LRFD SEISMIC BRIDGE DESIGN WITH INTERIM REVISIONS THROUGH 2015.

TEMPORARY DETOUR:

BRIDGE IS TO BE CLOSED DURING CONSTRUCTION. TEMPORARY BRIDGE SHALL BE CONSTRUCTED JUST SOUTH OF THE EXISTING CULVERT PRIOR TO THE CLOSURE. TEMPORARY ROAD SHALL BE PROVIDED WITH FOOTWALK. TRAVELWAY WILL BE ONE LANE OF ALTERNATING TRAFFIC.

EXISTING CONDITIONS:

EXISTING CONDITIONS ARE NOT GUARANTEED. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENT AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL HE HAS MADE THE REQUIRED MEASUREMENTS AND THE EXTENT OF THE PROPOSED WORK HAS BEEN APPROVED BY THE ENGINEER.

FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

GEOTECHNICAL REPORT:

REFER TO THE GEOTECHNICAL REPORT DATED MARCH 2022 (REVISED FEBRUARY 13, 2024) PREPARED BY GEI CONSULTANTS, 400 UNICORN PARK, WOBURN, MA, 01810. A COPY OF THE REPORT MAY BE OBTAINED FROM THE MASSACHUSETTS DEPARTMENT OF TRANSPORTATION.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATION OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

CONCRETE:

ALL CONCRETE SHALL BE 5000 PSI HP CONCRETE, EXCEPT AS NOTED BELOW:

THE CT-MTL2 BARRIER SHALL BE 5000 PSI 3 IN. HP CONCRETE.

TEMPORARY DIVERSION SYSTEMS

THIS SYSTEM IS REQUIRED FOR CHANNEL RECONSTRUCTION. SEE PLANS, AND SPECIAL PROVISIONS, ITEM NO. 950.11.

EXCAVATION SUPPORT SYSTEM:

COBBLES AND BOULDERS COULD PRESENT OBSTRUCTIONS DURING SUPPORT SYSTEM INSTALLATION. SHALLOW OBSTRUCTIONS MAY NEED TO BE REMOVED BEFORE SUPPORT SYSTEM INSTALLATION. PAYMENT IS UNDER CLASS B ROCK EXCAVATION. THIS SYSTEM SHALL BE USED IN CONJUNCTION WITH CONTROL OF WATER TO CONSTRUCT SUBSTRUCTURE ELEMENTS IN-THE-DRY, THIS SYSTEM SHALL BE REMOVED IN ITS ENTIRETY EXCEPT AS NOTED ON THE PLANS. SEE SPECIAL PROVISIONS ITEM 953.1.

REINFORCEMENT:

GENERAL NOTES

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1. NONE	16"	19"	23"
2. 12 INCHES OF CONCRETE BELOW BAR	20"	25"	30"
3. EPOXY COATED BARS, COVER < 3db, OR CLEAR SPACING < 6d	d _b 23"	29"	34"
4. COATED BARS, ALL OTHER CASES	18"	23"	27 "
5. CONDITION 2 AND 3	26"	32"	39"
6. CONDITION 2 AND 4	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWING.

REINFORCING BARS USED IN THE FOLLOWING ELEMENTS SHALL BE EPOXY COATED: BACKWALLS, BEAM SEATS DECK SLABS, DECK BEAMS, CT-TL2 BARRIER AND TRANSITION TOP OF THE PRECAST HIGHWAY GUARDRAIL

ALL REINFORCING STEEL SHALL BE A MINIMUM 2" CLEAR FROM THE SURFACE OF THE CONCRETE UNLESS OTHERWISE NOTED.

MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS - SPRAY APPLIED.

CONSTRUCTION JOINTS:

CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT PRIOR APPROVAL OF THE ENGINEER.

UTILITIES:

TEMPORARY RELOCATION OF OVERHEAD WIRES TO ALLOW FOR CONSTRUCTION OF BOTH THE PERMANENT AND TEMPORARY BRIDGES IS REQUIRED. CONTRACTOR TO COORDINATE THE UTILITY COMPANIES.

PRECAST CONCRETE ELEMENTS/TOLERANCES:

THE PROPOSED BRIDGE SHALL CONSIST OF PRECAST CONCRETE DECK BEAMS. THE CONTRACTOR SHALL SELECT A FABRICATOR THAT MEETS THE CRITERIA PER CONTRACT DOCUMENTS. SEE SPECIAL PROVISIONS ITEM 995.01

DRILLED SHAFTS:

ITEM NO. DESCRIPTION

SEE SPECIAL PROVISIONS ITEMS 945.102, 945.201, 945.302, 945.502, 945.602. SEE DRILLED SHAFT NOTES ON SHEET 16 OF 28.

MICROPILES:

SEE SPECIAL PROVISIONS ITEMS 945.10, 948.60, 948.61. SEE SHEET 14 OF 28 FOR MICROPILE

ESTIMATED QUANTITIES (NOT GUARANTEED)

QUANTITY UNIT

TILIVI INO.	DESCRIPTION	QUANTITI	OINII
115.1	DEMOLITION OF CULVERT NO. L-16-026 (OQX)	1	LS
127.1	REINFORCED CONCRETE EXCAVATION	75	CY
140.	BRIDGE EXCAVATION	710	CY
143.	CHANNEL EXCAVATION	420	CY
144.	CLASS B ROCK EXCAVATION	10	CY
151.2	GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES	140	CY
156.	CRUSHED STONE	157	TON
156.1	CRUSHED STONE FOR BRIDGE FOUNDATIONS	59	TON
450.60	SUPERPAVE BRIDGE SURFACE COURSE - 9.5 (SSC-B-9.5-P)	13	TON
450.70	SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 (SPC-B-9.5-P)	13	TON
482.31	SAWING & SEALING JOINTS IN ASPHALT PAVEMENT AT BRIDGES '	60	FT
698.3	GEOTEXTILE FABRIC FOR SEPARATION	100	SY
945.10	DRILLED MICROPILES	360	FT
945.102	DRILLED SHAFT EXCAVATION 3.5 FOOT DIAMETER	114	FT
945.201	ROCK SOCKET EXCAVATION 3 FOOT DIAMETER	72	FT
945.302	OBSTRUCTION EXCAVATION 3.5 FOOT DIAMETER	18	FT
945.502	DRILLED SHAFT 3.5 FOOT DIAMETER	186	FT
945.602	PERMANENT CASING 3.5 FOOT DIAMETER	126	FT
945.71	CROSS HOLE SONIC TEST ACCESS PIPES	240	FT
945.72	CROSS HOLE SONIC TEST	12	EA
948.60	MICROPILE VERIFICATION LOAD TEST	1	EA
948.61	MICROPILE PROOF LOAD TEST	2	EA
950.101	TEMPORARY SHORING	150	SY
950.11	DIVERSION SYSTEM	80	SY
953.1	EXCAVATION SUPPORT SYSTEM	460	SY
983.011	NATURAL STREAMBED RESTORATION	55	CY
983.1	RIPRAP	498	TON
991.1	CONTROL OF WATER - STRUCTURE NO. L-16-026 (CDG)	1	LS
993.1	TEMPORARY BRIDGE NO. L-16-026	1	LS
993.11	TEMPORARY BRIDGE NO. L-16-026 REMOVED AND STACKED	1	LS
995.01	BRIDGE STRUCTURE, BRIDGE NO. L-16-026 (CDG)	1	LS
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STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(782)X	21	52
	PROJECT FILE NO.	609120	

GENERAL NOTES & QUANTITIES

TRAFFIC DATA		
	ROADWAY OVER	ROADWAY UNDER
DESIGN YEAR	2042	
AVERAGE DAILY TRAFFIC — PRESENT	113	
AVERAGE DAILY TRAFFIC — DESIGN YEAR	125	
DESIGN HOURLY VOLUME	13	
DIRECTIONAL DISTRIBUTION	50%	X
TRUCK PERCENTAGE — AVERAGE DAY	0%	
TRUCK PERCENTAGE — PEAK HOUR	0%	
DESIGN SPEED	25 MPH	
DIRECTIONAL DESIGN HOURLY VOLUME	7	

1000
1000
0.065
0.162
0.060
С
A

HYDRAULIC DESIGN DATA	
DRAINAGE AREA (SQ. MILES)	13.50
DESIGN FLOOD DISCHARGE (C.F.S.)	938
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	6.42
DESIGN FLOOD ELEVATION (FEET, NAVD)	324.94
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	1959
BASE FLOOD ELEVATION (FEET, NAVD)	327.23
DESIGN AND CHECK SCOUR DATA	
DESIGN SCOUR FLOOD EVENT	25
RETURN FREQUENCY (YEARS)	20
DESIGN FLOOD ABUTMENT SCOUR DEPTH (FEET)	6.31
DESIGN FLOOD PIER SCOUR DEPTH (FEET)	N/A
CHECK SCOUR FLOOD EVENT	50
RETURN FREQUENCY (YEARS)	
CHECK FLOOD ABUTMENT SCOUR DEPTH (FEET)	6.56
CHECK FLOOD PIER SCOUR DEPTH (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	N/A
FREQUENCY (IF KNOWN, YEARS)	N/A
MAXIMUM ELEVATION (FEET, NAVD)	N/A
DATE (MM/YYYY)	N/A
HISTORY OF ICE FLOES	N/A
EVIDENCE OF SCOUR	N/A
AND EROSION	, , , .

TEMPORARY WATER CONTRO DESIGN DATA	L
DESIGN FLOOD DISCHARGE (C.F.S.)	938
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	12.26
DESIGN FLOOD ELEVATION (FEET, NAVD)	328.97

SEPT 07, 2024	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS CONSTRUCTION	APPROVED FOR Any Little Tale
AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER
USE	ONLY PRINTS OF LATEST DATE

BORING LOGS 1 OF 6

BORING INFORMATION BORING INFORMATION BORING BORING NORTHING: 2899782 **EASTING**: 411012 **STATION:** 3+97.5 OFFSET: 11' N **NORTHING**: 2899782 STATION: 3+94 OFFSET: 11' N **EASTING**: 411008 GROUND SURFACE EL. (ft): 330.5 DATE START/END: 7/7/2021 - 7/7/2021 GROUND SURFACE EL. (ft): 330.5 **DATE START/END:** 7/7/2021 - 7/7/2021 **BB-1** BB-1A VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling TOTAL DEPTH (ft): 8.0 TOTAL DEPTH (ft): 9.0 DRILLER NAME: Dale DRILLER NAME: Dale LOGGED BY: A. Parry RIG TYPE: Mobile B-57 LOGGED BY: A. Parry RIG TYPE: Mobile B-57 PAGE 1 of 1 PAGE 1 of 1 **DRILLING INFORMATION DRILLING INFORMATION HAMMER TYPE**: Automatic **CASING I.D./O.D.:** 4 inch / 4.5 inch CORE BARREL TYPE: NX HAMMER TYPE: Automatic CORE BARREL TYPE: NX **CASING I.D./O.D.:** 4 inch / 4.5 inch DRILL ROD O.D.: 2.625 inch AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: 2.625 inch CORE BARREL I.D./O.D.: 2.15 inch / 2.96 inch AUGER I.D./O.D.: NA / NA **CORE BARREL I.D./O.D.:** 2.15 inch / 2.96 inch **DRILLING METHOD:** Driven casing and washed with rotary tooling. DRILLING METHOD: Driven casing and washed with rotary tooling. WATER LEVEL DEPTHS (ft): Not measured WATER LEVEL DEPTHS (ft): Not measured ABBREVIATIONS: Pen. = Penetration Length NA, NM = Not Applicable, Not Measured ABBREVIATIONS: Pen. = Penetration Length S = Split Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured S = Split Spoon Sample Op = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength Sv = Pocket Torvane Shear Strength C = Core Sample Rec. = Recovery Length Rec. = Recovery Length C = Core Sample Blows per 6 in.: 140-lb hammer falling Blows per 6 in.: 140-lb hammer falling U = Undisturbed Sample RQD = Rock Quality Designation RQD = Rock Quality Designation LL = Liquid Limit U = Undisturbed Sample LL ≃ Liquid Limit 30 inches to drive a 2-inch-O.D. 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% = Length of Sound Cores>4 in / Pen.,% SC ≃ Sonic Core Pl = Plasticity Index PI = Plasticity index SC = Sonic Core split spoon sampler. split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D. = Inside Diameter/Outside Diameter I.D./O.D. = Inside Diameter/Outside Diameter WOH = Weight of Hammer HSA = Hollow-Stem Auger Sample Information Sample Information Drilling Remarks/ Drilling Remarks/ Elev. Depth Pen./ Pen./ Blows Blows Soil and Rock Description Soil and Rock Description (ft) (ft) Sample Depth (ft) (ft) Sample Depth Field Test Data Field Test Data Rec. per 6 in. or RQD Rec. per 6 in. (in) or RQD (ft) No. (ft) EL. 330.5± EXIST. GRADE EL. 330.5± EXIST. GRADE 1 0-1": ASPHALT. See BB-1 log for sample 24/16 | 11-10descriptions 0-8 feet. 330 --S1 (0-4"): moist, medium dense, dark brown, FINE TO COARSE 10-12 SAND and fine to medium gravel. S1 (4-11"): moist, medium dense, brown, FINE TO COARSE SAND, trace inorganic silt, trace fine to medium gravel. S1 (11-16"): moist, medium dense, brown, FINE TO MEDIUM SAND, trace fine gravel. S2 (0-4"): moist, medium dense, brown, FINE TO MEDIUM SAND, 24/13 11-8-5-7 some inorganic silt, trace fine gravel. S2 (4-13"): moist, medium dense, brown, FINE TO MEDIUM SAND, trace fine gravel. \$3: moist, brown, very loose, FINE TO COARSE SAND, trace fine 24/17 5-2-1-10 gravel. S4 (0-6"): Similar to S3, except medium dense. 22/15 11-5-7-S4 (6-13""): moist, medium dense, dark brown, FINE SAND AND to 7.8 100/4" 4-inch casing from 0 to 6.5 INORGANIC SILT, trace fine to medium gravel up to 1" feet. Couldn't advance 4-inch S4 (13-15""): moist, medium dense, dark brown, FINE TO 4-inch casing from 0 to 7 ft. Hit refusal at 7 feet. Pulled casing deeper than 6.5 feet. MEDIUM GRAVEL, some fine sand and inorganic silt." Casing rebounding, driller S4 (6-13"): moist, medium dense, dark brown, FINE SAND AND casing out and drive shoe is indicating casing is stuck on INORGANIC SILT, trace fine to medium gravel up to 1". S4 (13-15"): moist, medium dense, dark brown, FINE TO MEDIUM deformed. Attempted spinning possible boulder. 4-inch casing back down. GRAVEL, some fine sand and inorganic silt. C1 Spinning casing hits refusal at Bottom of boring at 8 feet upon refusal on possible boulder. 6/0 8.5 feet. Driller indicated that Backfilled with soil cuttings, sand, and concrete, and topped with casing is stuck on another asphalt cold patch. possible boulder. Sent core barrel down the hole.Core time: 2.6 min/6" After coring 6 inches through 320 the boulder, driller still unable to advance casing further than 8.5 feet. Terminate hole and relocate. 318.0 BOT. PILE CAP BOT. PILE CAP DRILLED SHAFT DRILLED SHAFT GEI Consultants PROJECT NAME: Piney Lane Bridge NOTES: Used 300-lb hammer to advance casing. PROJECT NAME: Piney Lane Bridge **NOTES:** Used 300-lb hammer to advance casing. Offsetted the boring ~3.5 ft west and drilled BB-1A. Offsetted boring ~17.5 ft west and drilled BB-1B. CITY/STATE: Ludlow, Massachusetts CITY/STATE: Ludlow, Massachusetts GEI PROJECT NUMBER: 2101263 GEI PROJECT NUMBER: 2101263

NOTES:

- LOCATION OF BORINGS ARE SHOWN THUS:
 BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. GROUND WATER DEPTH MEASUREMENTS WERE NOT TAKEN FOR BORINGS BB-1, BB-1A, BB-1B, AND BB-2. WATER LEVELS SHOWN ON BORING LOGS BB-3 AND BB-4 WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL. GROUND WATER LEVELS MAY VARY SIGNIFICANTLY DURING OTHER TIMES AND LOCATIONS. FOR FURTHER DISCUSSION ON THE GROUND WATER, SEE THE GEOTECHNICAL REPORT.
- 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 18" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30".
- 5. BORING SAMPLES ARE STORED AT A STORAGE FACILITY LOCATED ON ROUTE 114 (219 WINTHROP AVE.) IN LAWRENCE, MA. THE CONTRACTOR MAY EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING THE MASSDOT GEOTECHNICAL SECTION AT: 10 PARK PLAZA, BOSTON, MA.
- 6. BORINGS BB-1, BB-1A, BB-1B, AND BB-2 WERE MADE IN JULY, 2021. BORINGS BB-3 AND BB-4 WERE MADE IN MARCH, 2023.
- 7. BORINGS WERE MADE BY SEABOARD DRILLING AT 649 MEADOW ST, CHICOPEE, MA 01013.
- 8. THE NORTH AMERICAN VERTICAL DATUM (NAVD)
 OF 1988 IS USED THROUGHOUT.

SEPT 07, 2024	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
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AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER
USE	ONLY PRINTS OF LATEST DATE

BORING LOGS 2 OF 6

	NORTI GROU VERT.	HING: ND SURI /HORIZ.		ft):331 NAVD 8	8/NAD 83 N	MA State Pl	STATION: 3+76.5 DATE START/END: 7/ ane DRILLING COMPANY: DRILLER NAME: Dale	7/202 Sea	21 - 7/8/2021	BORING BB-1B				
			(ft):29. _A. Parry	***************************************			7 PAGE 1 of 2							
	HAMW AUGE DRILL	DRILLING INFORMATION HAMMER TYPE: Automatic CASING I.D./O.D.: 4 inch / 4.5 inch CORE BARREL TYPE: NX AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: 2.625 inch CORE BARREL I.D./O.D.: 2.15 inch / 2.96 inch DRILLING METHOD: Driven casing and washed with rotary tooling. WATER LEVEL DEPTHS (ft): Not measured												
	ABBR	EVIATIO	Rec. RQE WOI	Length of # R = Weight	y Length Iality Design f Sound Core	es>4 in / Pen	S = Split Spoon Sample C = Core Sample U = Undisturbed Sample% SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Sv = Pocket Torvane Shear Strength LL = Liquid Limit Pl = Plasticity Index	A, NM = Not Applicable, Not Measured ows per 6 In.: 140-lb hammer falling Linches to drive a 2-inch-O.D. uit spoon sampler.				
Г І 771 О⊥	Elev. (ft)	Depth (ft)	Sample No.		Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock (Description				
EL. 331.8± EXIST. GRADE	-						See boring log BB-1 for soil descriptions 0-8 ft.							
	330 —				MILLIMETER AREA PARA PARA PARA PARA PARA PARA PA	**************************************	4-inch casing from 0 to 6 feet.							
	-	5		*****	**************************************		Casing hit hard resistance at 4 feet. Driller suggest they are on another boulder. Hit very hard refusal. Drillers indicated casing advanced	1114						
325.0 DT. SPREAD			G1				past the first boulder and is now encountering resistance on the other side of casing. Drillers struggled to keep casing vertical. Drillers removed 4-inch casing and drove 5-inch	THE A THE STATE AND A THE A TH	G1: moist, brown/gray, FINE TO CO medium gravel, some inorganic silt.	E TO COARSE SAND, some fine to panic silt.				
	SS.GF. SI.SKK	10					casing to 6 feet. Grab sample from bottom of drive shoe when pulled up 4" casing Used 5-inch roller bit to clean hole to 10 feet, not very difficult drilling. Drove 4-inch casing after washing hole and gets hung up at about 6 feet. Drillers continued to drive							
S. S	20 2 320		S1	11.5 to 13.5	24/14	9-26-54- 67		LUVIAL DEPOSITS	S1 (0-12"): moist, very dense, brown trace inorganic silt, trace fine to med S1 (12-14") moist, very dense, brown some fine to coarse sand, some inor DISTRIBUTION PERFORMED]. C1: QUARTZ MONZODIORITE BO	ium gravel. n, FINE GRAVEL UP TO 3/8", ganic silt. [GRAIN SIZE				
# 68 A M G S A		15	C1	14.3 to 15.4	13/2	0	4-inch casing to 14 ft. Very difficult drilling, drillers decided to send core barrel down. Core barrel drops after 13 inches.Core time: 1.4 min/13"	GLACIOFLUVIAL						
i N C	NOTE	\$: lead	300-lb ham	Imper to ad	vance coci	00		DRO!	IECT NAME: Piney Lane Bridge					
e accenting	TO CO	J. USEU	ovens tidi!	ты ко aq	ivanije Udšl	··명,		CITY/	STATE: Ludlow, Massachusetts PROJECT NUMBER: 2101263	GEI Consultants				

	GROU	ND SUR	2899782 FACE EL. (DATUMS:	ft): <u>331</u>	.8	410991 MA State Pl	STATION: 3+76.5 DATE START/END: _7 ane DRILLING COMPANY:	/7/202		BORING BB-1B PAGE 2 of 2
			S	ample Ini	formation			ø		
	Elev. (ft)	Depth (ft)	***************************************	***************************************	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and I	Rock Description
EL. 312.0± ESTIMATED TOP OF COMPETENT BEDROCK	310	20	C2	19.8 to 24.8	60/60	75	Drove 4-inch casing to 19 feet. Difficult to keep casing vertical. Hit hard resistance at 19 feet and on possible rock. Wash casing to ~19.5 feet. Core times: 7.1, 6.25, 8, 6.1, and 10.25 min/ft		C2: QUARTZ MONZODIORI medium grained, joints space light gray to dark blue (BELC)	TE, hard, fresh, massive, fine to ed at 1" to 10", dipping at 10-45 deg, HERTOWN COMPLEX).
		- 25 -	C3	24.8 to 29.1	51/35	100	Core times: 6.1, 8.15, 9.5, and 6.8, min/ft; 4.1 min/3"	BEDROCK	C3: Similar to C2, except joint	s range from 10-15 deg.
3/3/22	300	- 30 -							Bottom of boring at 29.1 feet. Backfilled with soil cuttings, sa asphalt cold patch.	and, and concrete, and topped with
AYER NAME PINEYLN-BORINGLOGS.GPJ 3/3		- - 35 								
WOBURN STD 7-S-O-N-E-LAYER		- 40 S: Used	300-lb han	nmer to ad	vance cas	ing.			IECT NAME: Piney Lane Bridge STATE: Ludlow, Massachusett	

ISSUED FOR CONSTRUCTION SEPT 07, 2024 DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER

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STATE FED. AID PROJ. NO. SHEET NO. SHEETS

MA STP(BR-OFF)-003S(782)X 24 52

PROJECT FILE NO. 609120

BORING LOGS 3 OF 6

BORING INFORMATION **BORING NORTHING (ft):** 2,899,794 **EASTING (ft)**: 411,066 GROUND SURFACE EL. (ft): 327.9 **DATE START/END:** 7/6/2021 - 7/6/2021 **BB-2** VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling TOTAL DEPTH (ft): 31.7 **DRILLER NAME:** Dale Griffin LOGGED BY: A. Parry RIG TYPE: Mobile B-57 PAGE 1 of 2 DRILLING INFORMATION CORE BARREL TYPE: NX **HAMMER TYPE:** Automatic CASING I.D./O.D.: 4 inch/ 4.5 inch CORE BARREL I.D./O.D.: 2.15 inch / 2.96 inch AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: 2.625 inch **DRILLING METHOD:** Driven casing and washed with rotary tooling. WATER LEVEL DEPTHS (ft): Not measured **ABBREVIATIONS:** Pen. = Penetration Length NA, NM = Not Applicable, Not Measured S = Split Spoon Sample Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength Rec. = Recovery Length C = Core Sample Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PI = Plasticity Index split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D.= Inside Diameter/Outside Diameter Elev. | Depth | Sample | Depth | No. | (ft) | Drilling Remarks/ Pen./ Blows Soil and Rock Description Field Test Data Rec. per 6 in. (in) or RQD ~1" ASPHALT 4-inch casing from 0 to 21.7 S1: Dry, loose, brown, FINE TO COARSE SAND. 24/15 5-5-4-4 S2: Moist, loose, brown, FINE TO COARSE SAND, some fine 24/12 5-3-3-5 S2 gravel, trace inorganic silt. S3: Moist, loose, brown, FINE TO COARSE SAND, some fine S3 24/19 | 4-4-3-5 gravel up to 3/8", trace inorganic silt. [GRAIN SIZE DISTRIBUTION S4 (0-17"): Moist, dark brown/gray, medium dense, FINE TO COARSE SAND, some fine to medium gravel up to 1/2". S4 (17-19"): Wet, brown, medium dense, FINE TO MEDIUM GRAVEL AND FINE TO COARSE SAND, some inorganic silt. 24/19 | 7-8-9-22 | 320 — S5 (0-8"): Wet, gray, dense, FINE TO COARSE SAND, trace S5 24/12 | 20-16inorganic silt.
S5 (8-12"): Moist, gray, dense, FINE TO COARSE SAND, some inorganic silt, some fine to medium gravel up to 1/2". S6: Moist, gray, medium dense, FINE TO COARSE SAND, trace gravel, trace inorganic silt. 11 to 13 24/18 | 6-5-7-14 S7 (0-8"): Moist, brown with little orange, very dense, FINE TO COARSE SAND, some gravel, some inorganic silt. 13 to 15 24/12 16-21-S7 35-27 S7 (8"-12"): Moist, brown, very dense, FINE SAND AND INORGANIC SILT, trace gravel. **PROJECT NAME:** Piney Lane Bridge Over Broad Brook - Bridge No. L-16-026 **NOTES:** Used 300-lb hammer to advance casing.

CITY/STATE: Ludlow, Massachusetts
GEI PROJECT NUMBER: 2101263

EL. 328.0± EXIST. GRADE

EL. 318.0 BOT. PILE CAP

DRILLED SHAFT-

		GROUI	ND SUR		(ft): <u>327</u>		MA State Pla	EASTING (ft): 411,066 DATE START/END: 7 ne DRILLING COMPANY:	/6/20		BORING BB-2
						. ,.	ł		0		PAGE 2 of 2
		Elev. (ft)	Depth (ft)		Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and	Rock Description
		310 —	- - - 20		20				GLACIAL TILL	S8: Maist grav van denso	FINE TO COAPSE SAND, some fine
	EL 306.0±	_	_	S8	to 20.8	10/6	100/10"		 - 	gravel up to 3/8", some inorg DISTRIBUTION PERFORMI	FINE TO COARSE SAND, some fine anic silt. [GRAIN SIZE ED].
	EL. 306.0± ESTIMATED TOP OF COMPETENT BEDROCK	_	<u>-</u>	C1	21.7 to 26.7	60/60	62	Core times: 4.3, 4.4, 5.6, 7, 5.1 min/ft.		massive, medium to coarse s	TE, gray to dark blue, hard, fresh, size grains, massive, generally intact 3 to 13 inches, dipping from 10-45 COMPLEX).
		-	- 25 -						OCK		
	EL. 300.0 BOT. OF SHAFT/ ROCK SOCKET	300 —	_ _ _	C2	26.7 to 31.7	60/60	82	Core times: 5, 4, 4.5, 4.9, 5.6 min/ft.	BEDROCK	C2: QUARTZ MONZODIOR	TE, Similar to C1.
		_	— 30 -							Bottom of boring at 31.7 feet	
	PINEYLN-BORINGLOGS.GPJ 4/11/23	-	-							Backfilled with soil cuttings, sasphalt cold patch.	sand, and concrete, and topped with
	ME		— 35 - -								
	NORTH-EAST-LAYER NA	290 —	-								
-	STD 5-1	NOTES	— 40 3: Used	300-lh hai	nmer to ad	vance casi	ina.	1	DDO	IECT NAME: Dipout one Bridge	a Over Broad
	GEI WOBURN		3300						Brook CITY	JECT NAME: Piney Lane Bridge k - Bridge No. L-16-026 /STATE: Ludlow, Massachuse PROJECT NUMBER: 2101263	

SEPT 07, 2024 ISSUED FOR CONSTRUCTION

DATE DESCRIPTION

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STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(782)X	25	52
	PROJECT FILE NO.	609120	

BORING LOGS 4 OF 6

BORING INFORMATION **BORING** NORTHING: 2899763 **EASTING**: 410971 STATION: 3+53 OFFSET: 3'S GROUND SURFACE EL. (ft): 334.5 **DATE START/END:** 7/7/2021 - 7/7/2021 PC-1 VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling TOTAL DEPTH (ft): 2.5 DRILLER NAME: Dale LOGGED BY: A. Parry RIG TYPE: Mobile B-57 PAGE 1 of 1 DRILLING INFORMATION CORE BARREL TYPE: HAMMER TYPE: Automatic **CASING I.D./O.D.:** 4 inch / 4.5 inch DRILL ROD O.D.: NM AUGER I.D./O.D.: NA / NA CORE BARREL I.D./O.D.: NA / NA DRILLING METHOD: Driven casing. WATER LEVEL DEPTHS (ft): Not measured ABBREVIATIONS: Pen. = Penetration Length S = Split Spoon Sample Qp = Pocket Penetrometer Strength NA, NM = Not Applicable, Not Measured Rec. = Recovery Length C = Core Sample Sv = Pocket Torvane Shear Strength Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation U = Undisturbed Sar = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. PI = Plasticity Index split spoon sampler. WOR = Weight of Rods PID = Photolonization Detector DP = Direct Push Sample I.D./O.D. = Inside Diameter/Outside Diameter WOH = Weight of Hammer HSA = Hollow-Stem Auger Sample Information Elev. Depth (ft) Sample Depth No. (ft) Drilling Remarks/ Pen./ Blows Rec. per 6 in. (in) or RQD Soil and Rock Description Field Test Data C1 (0-6"): ASPHALT. C1 6/6 0.5 24/12 13-16- 18-8 S1 (0-3"): moist, dense, brown, FINE TO COARSE SAND, some **S**1 fine to medium gravel, trace inorganic silt.
S1 (3-12"): moist, dense, brown, FINE TO COARSE SAND, some fine to medium gravel and silt. Bottom of boring at 2.5 feet. Backfilled with soil cuttings, sand, and concrete, and topped with asphalt cold patch. 330 ---320 ---PROJECT NAME: Piney Lane Bridge CITY/STATE: Ludlow, Massachusetts

GEI PROJECT NUMBER: 2101263

EL. 334.5± EXIST. GRADE

	NORTI GROU VERT. TOTAI	HING: _: ND SUR /HORIZ. L DEPTH		t): <u>326.</u> NAVD 88	7 /NAD 83 N	411108 MA State Plane	DATE START/END: 7	/6/202 Sea e	21 - 7/6/2021				
	HAMN AUGE DRILL						CASING I.D./O.D.: 4 inch / 4.5 inch CORE BARREL TYPE: DRILL ROD O.D.: NM CORE BARREL I.D./O.D.: NA / NA						
	ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen.,% WOR = Weight of Rods					ation es>4 in / Pen.,%	S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger		Op = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit Pl = Plasticity Index PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Diameter				
	Elev. (ft)	Depth (ft)		ample Info Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description				
	41		C1 S1	0 to 0.3 0.3 to 2.1	<u>4/4</u> 22/11	11-9-7- 100/4"			C1 (0-4"): ASPHALT. S1 (0-2"): moist, medium dense, brown/dark brown, FINE TO COARSE SAND, some inorganic silt, trace fine to medium gravel up to 1". S1 (2-11"): moist, medium dense, brown, FINE TO MEDIUM SAND, some inorganic silt, trace fine to medium gravel up to 1/2".				
		5	433434444						Bottom of boring at 2.3 feet. Backfilled with soil cuttings, sand, and concrete, and topped with asphalt cold patch.				
068.6PJ 3/3/22	320	10											
S-O-N-E-LAYER NAME PINEYI.N-BORINGI.C	Anticated												
WOBURN STD 7.5	NOTES	5 :						CITY/	JECT NAME: Piney Lane Bridge //STATE: Ludlow, Massachusetts PROJECT NUMBER: 2101263				

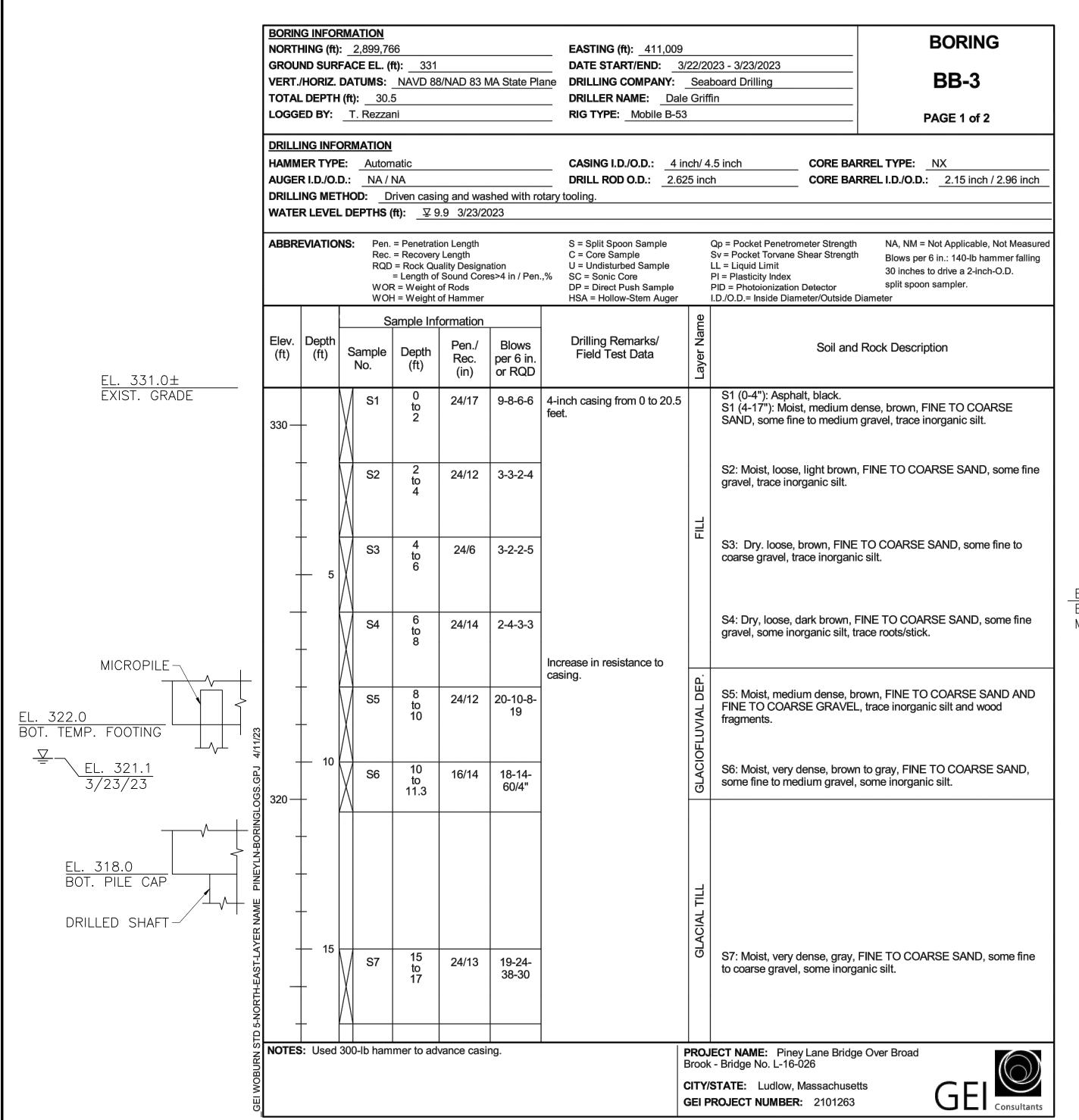
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DATE	DESCRIPTION	
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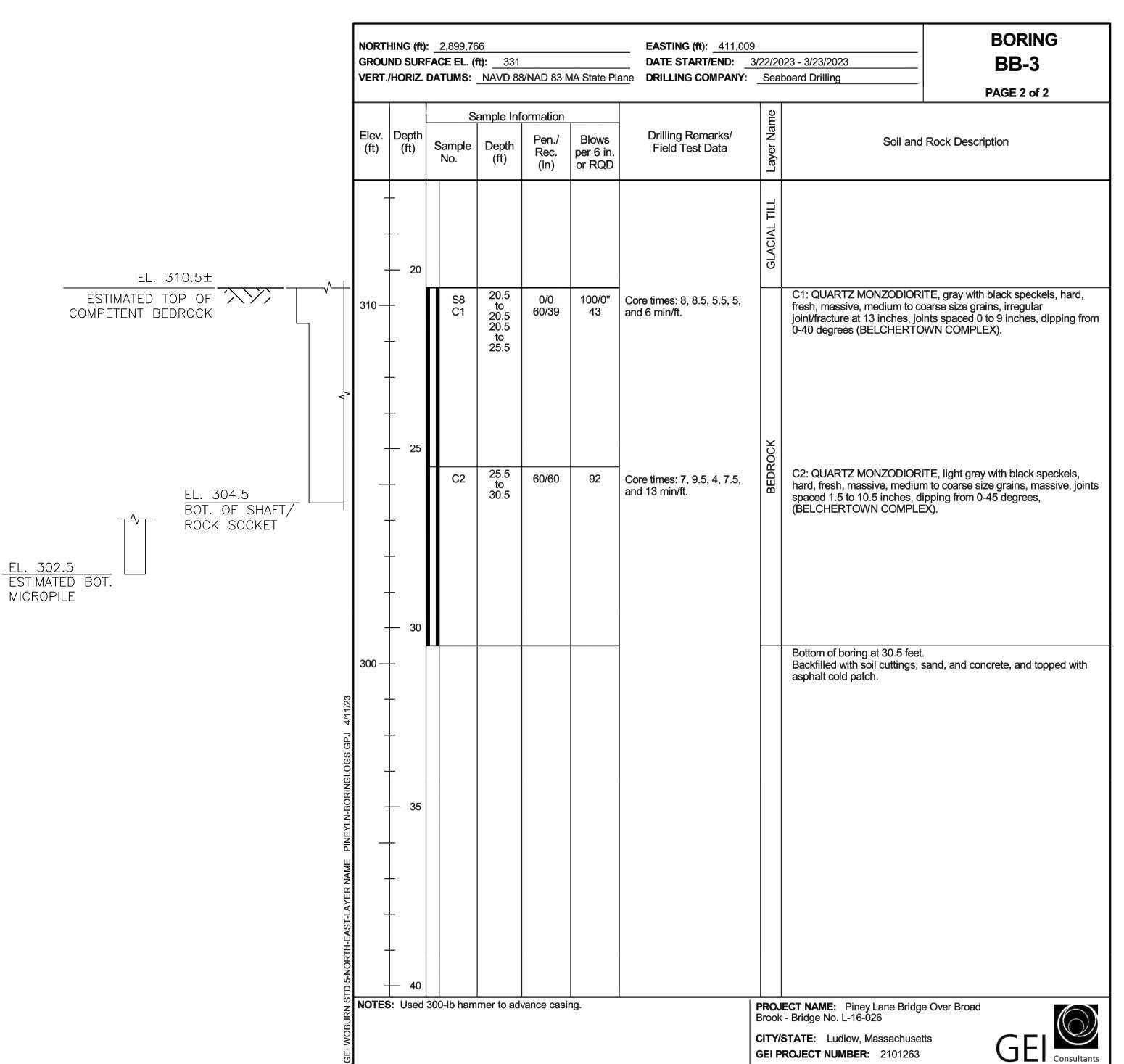
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MA STP(BR-OFF)-003S(782)X 26 52

PROJECT FILE NO. 609120

BORING LOGS 5 OF 6





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BORING LOGS 6 OF 6

BORING INFORMATION **BORING NORTHING (ft):** 2,899,746 **EASTING (ft):** 411,102 **DATE START/END:** 3/21/2023 - 3/22/2023 GROUND SURFACE EL. (ft): 324.5 **BB-4** VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling TOTAL DEPTH (ft): 30.5 **DRILLER NAME:** Dale Griffin LOGGED BY: T. Rezzani **RIG TYPE:** ATV D-101 PAGE 1 of 2 **DRILLING INFORMATION CASING I.D./O.D.:** 4 inch/ 4.5 inch HAMMER TYPE: Automatic CORE BARREL TYPE: NX AUGER I.D./O.D.: NA / NA DRILL ROD O.D.: 2.625 inch **CORE BARREL I.D./O.D.:** 2.15 inch / 2.96 inch **DRILLING METHOD:** Driven casing and washed with rotary tooling. **WATER LEVEL DEPTHS (ft):** $\subseteq 5.6$ 3/22/2023 NA, NM = Not Applicable, Not Measured ABBREVIATIONS: Pen. = Penetration Length Qp = Pocket Penetrometer Strength S = Split Spoon Sample Sv = Pocket Torvane Shear Strength Rec. = Recovery Length C = Core Sample Blows per 6 in.: 140-lb hammer falling RQD = Rock Quality Designation U = Undisturbed Sample LL = Liquid Limit 30 inches to drive a 2-inch-O.D. = Length of Sound Cores>4 in / Pen.,% SC = Sonic Core PI = Plasticity Index split spoon sampler. WOR = Weight of Rods DP = Direct Push Sample PID = Photoionization Detector WOH = Weight of Hammer HSA = Hollow-Stem Auger I.D./O.D.= Inside Diameter/Outside Diameter Sample Information Elev. | Depth | Drilling Remarks/ Pen./ Blows Soil and Rock Description (ft) (ft) Sample Depth No. (ft) Field Test Data Rec. per 6 in. (in) or RQD EL. 324.5± EXIST. GRADE S1 (0-8"): Dry, loose, brown, FINE TO MEDIUM SAND, some 24/12 | 2-3-3-3 | 4-inch casing from 0 to 20.5 organic silt, dry. Roots present. TOPSOIL S1 (8-12") Dry, loose, brown, FINE TO COARSE SAND, some fine to medium gravel, trace inorganic silt. S2: Dry, loose, brown, FINE TO COARSE SAND, some fine to 24/5 2-2-2-2 S2 medium gravel, trace inorganic silt. MICROPILE -S3: Moist, medium dense, black, FINE SAND AND INORGANIC 4 S3 24/9 2-1-20-320 **—** S4: Wet, gray, very dense, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt. S4 13/9 16-14- Drilled indicated possible 100/1" | boulders from ~6-8 feet. S5: Wet, gray, dense, FINE TO COARSE SAND AND FINE TO COARSE GRAVEL, trace inorganic silt. 24/11 10-8-26-10 to S6: Moist, very dense, brownish gray, FINE TO COARSE SAND, 24/18 24-32some fine to coarse gravel, some inorganic silt. 33-21 13 to 13.9 S7: Similar to S6. S7 11/7 25-100/5" Slowed roller bit from 13.5 to 20.5 feet, driller noted weathered rock fragments. 310 — **NOTES:** Used 300-lb hammer to advance casing. **PROJECT NAME:** Piney Lane Bridge Over Broad Brook - Bridge No. L-16-026 CITY/STATE: Ludlow, Massachusetts GEI PROJECT NUMBER: 2101263

BORING NORTHING (ft): 2,899,746 **EASTING (ft):** 411,102 **BB-4** GROUND SURFACE EL. (ft): 324.5 **DATE START/END:** 3/21/2023 - 3/22/2023 VERT./HORIZ. DATUMS: NAVD 88/NAD 83 MA State Plane DRILLING COMPANY: Seaboard Drilling PAGE 2 of 2 Elev. | Depth | Drilling Remarks/ Blows Soil and Rock Description (ft) | Sample | Depth Field Test Data Rec. per 6 in. (in) or RQD No. (ft) EL. 304.0± C1: QUARTZ MONZODIORITE, light gray with black speckels, very hard, fresh, fine to medium grained, joints spaced 6 to 23 inches, dipping from 0 to 20 degrees from horizontal (BELCHERTOWN COMPLEX). ESTIMATED TOP OF 20.5 92 Core times: 4.5, 3, 2.5, 3, COMPETENT BEDROCK and 3 min/ft. 25.5 300 -C2: QUARTZ MONZODIORITE, light gray with black speckels, 60/60 100 C2 Core times: 5.5, 7.5, 6, 8, very hard, fresh, massive, fine to medium grained, joints spaced 5.5 to 21 inches, dipping from 0 to 60 degrees, light gray (BELCHERTOWN COMPLEX). and 6.5 min/ft. 30.5 Driller noted portion from C1 was recovered in C2. MICROPILE Bottom of boring at 30.5 feet. Backfilled with soil cuttings and sand. 290 -**NOTES:** Used 300-lb hammer to advance casing. **PROJECT NAME:** Piney Lane Bridge Over Broad Brook - Bridge No. L-16-026 **CITY/STATE:** Ludlow, Massachusetts

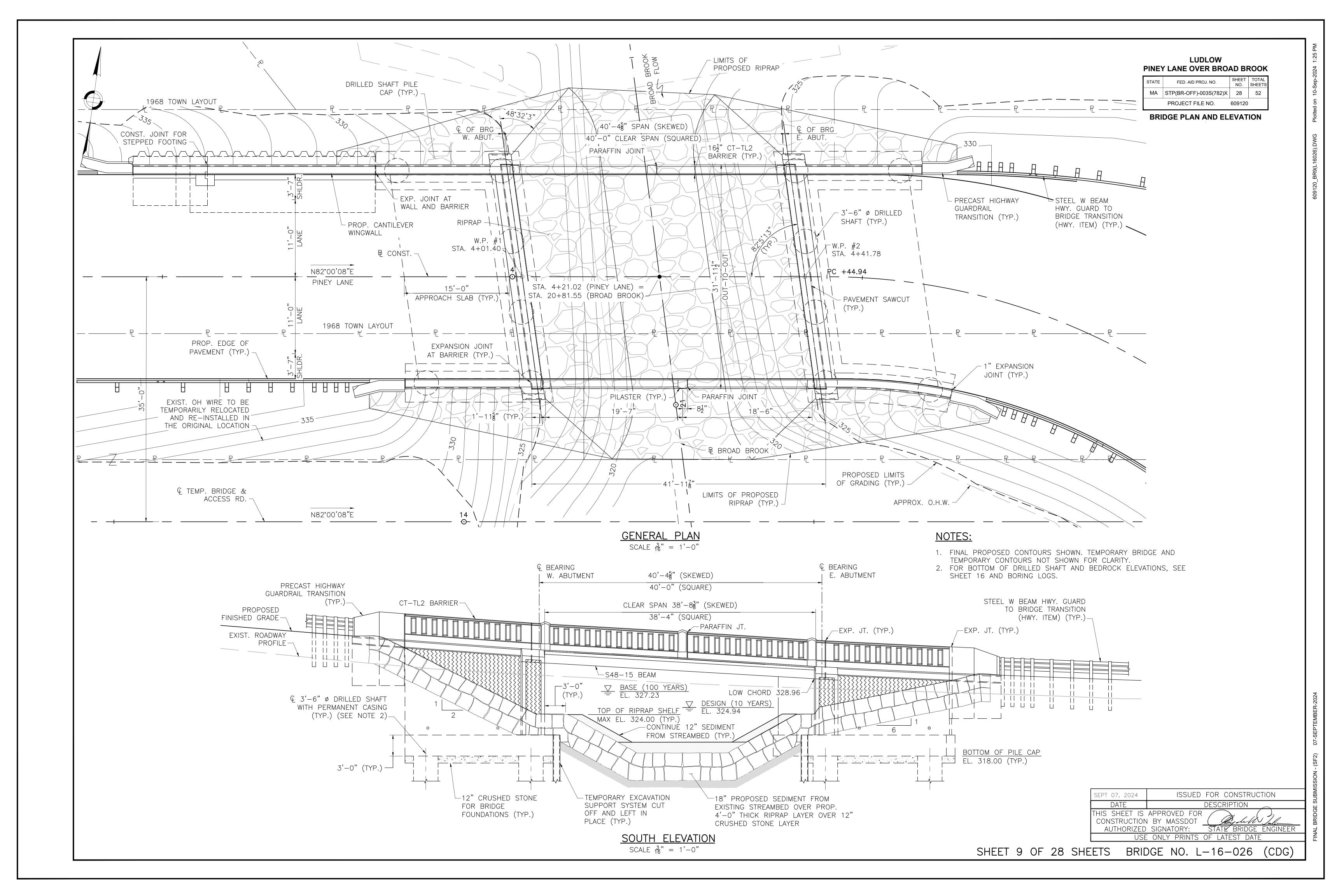
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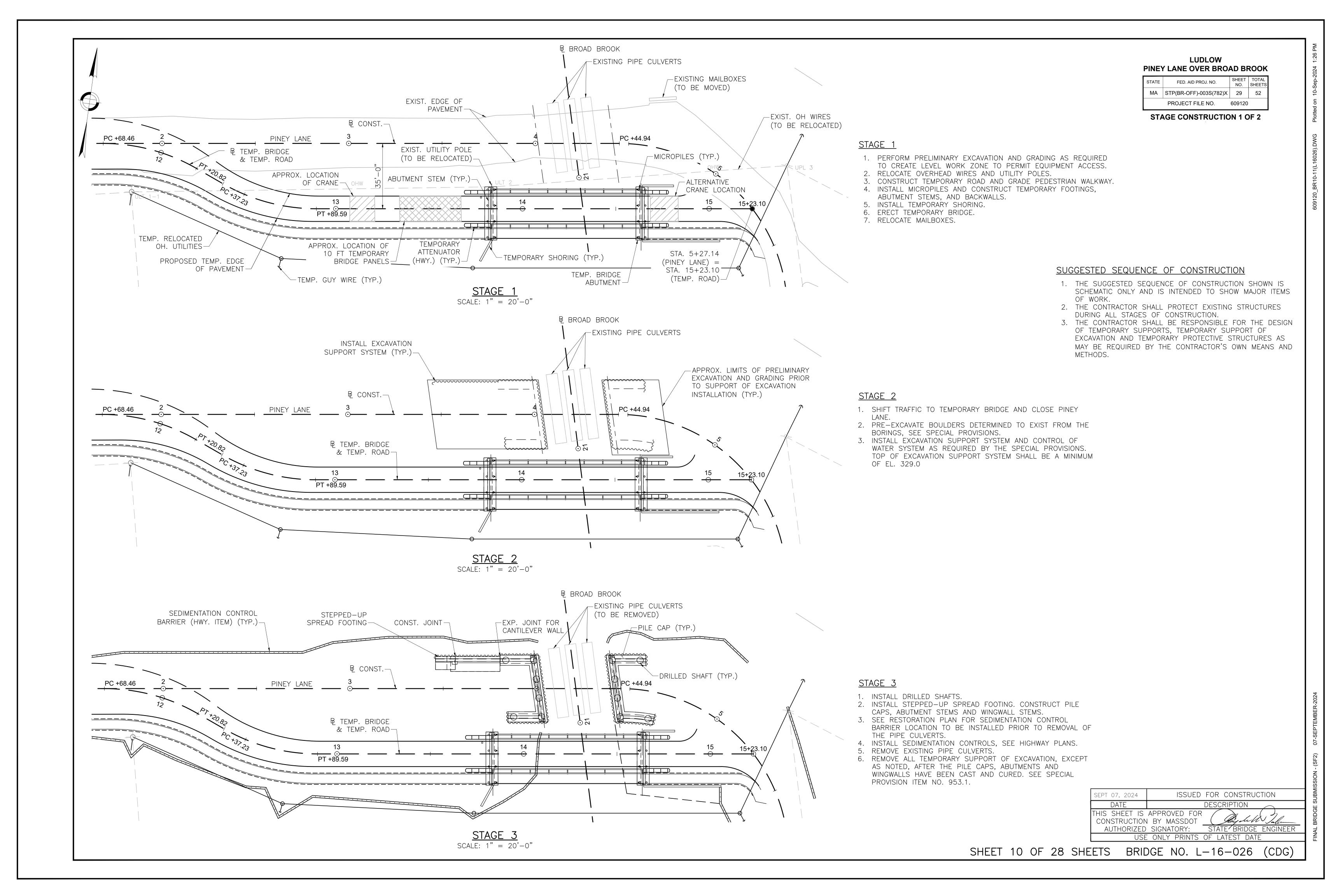
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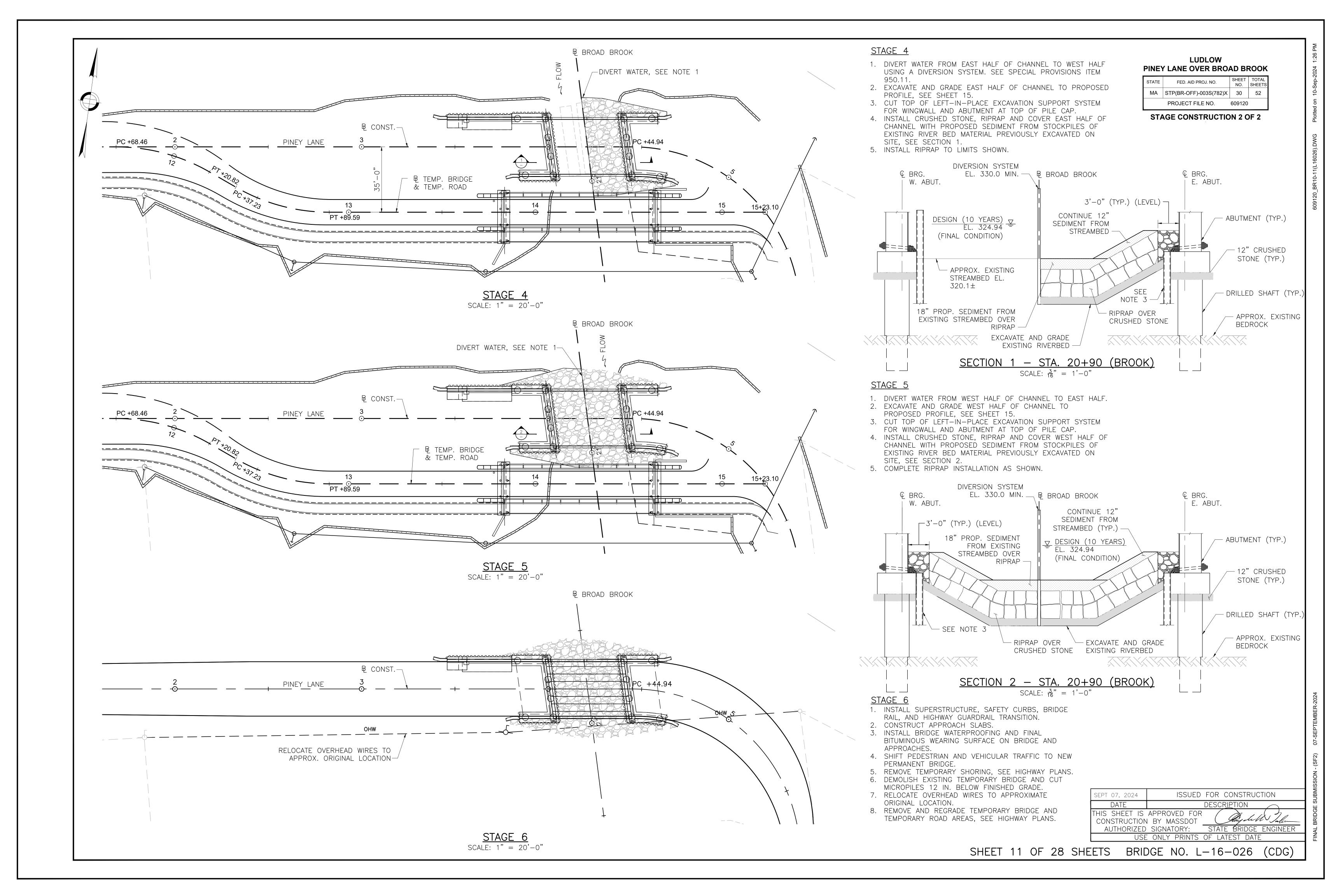
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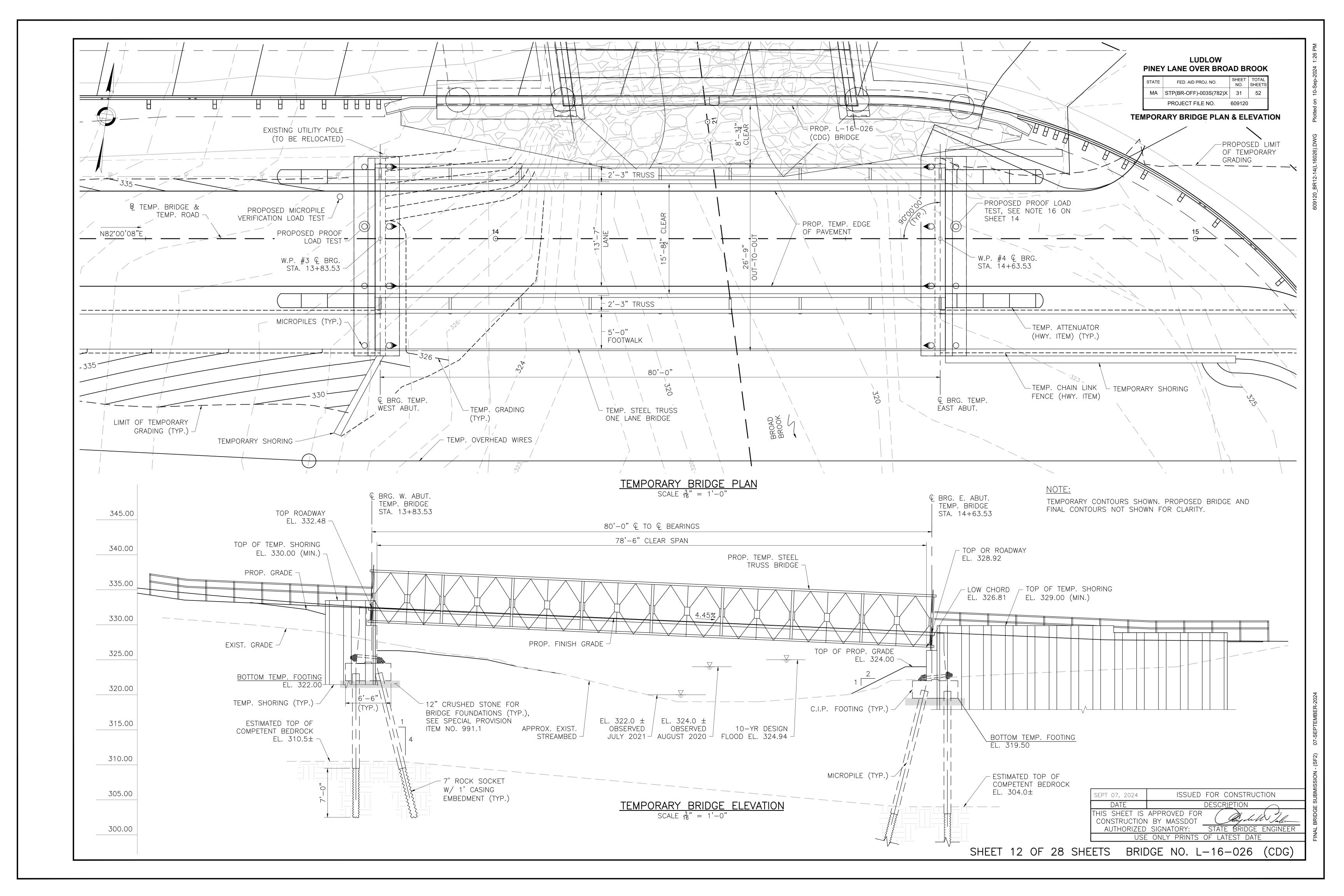
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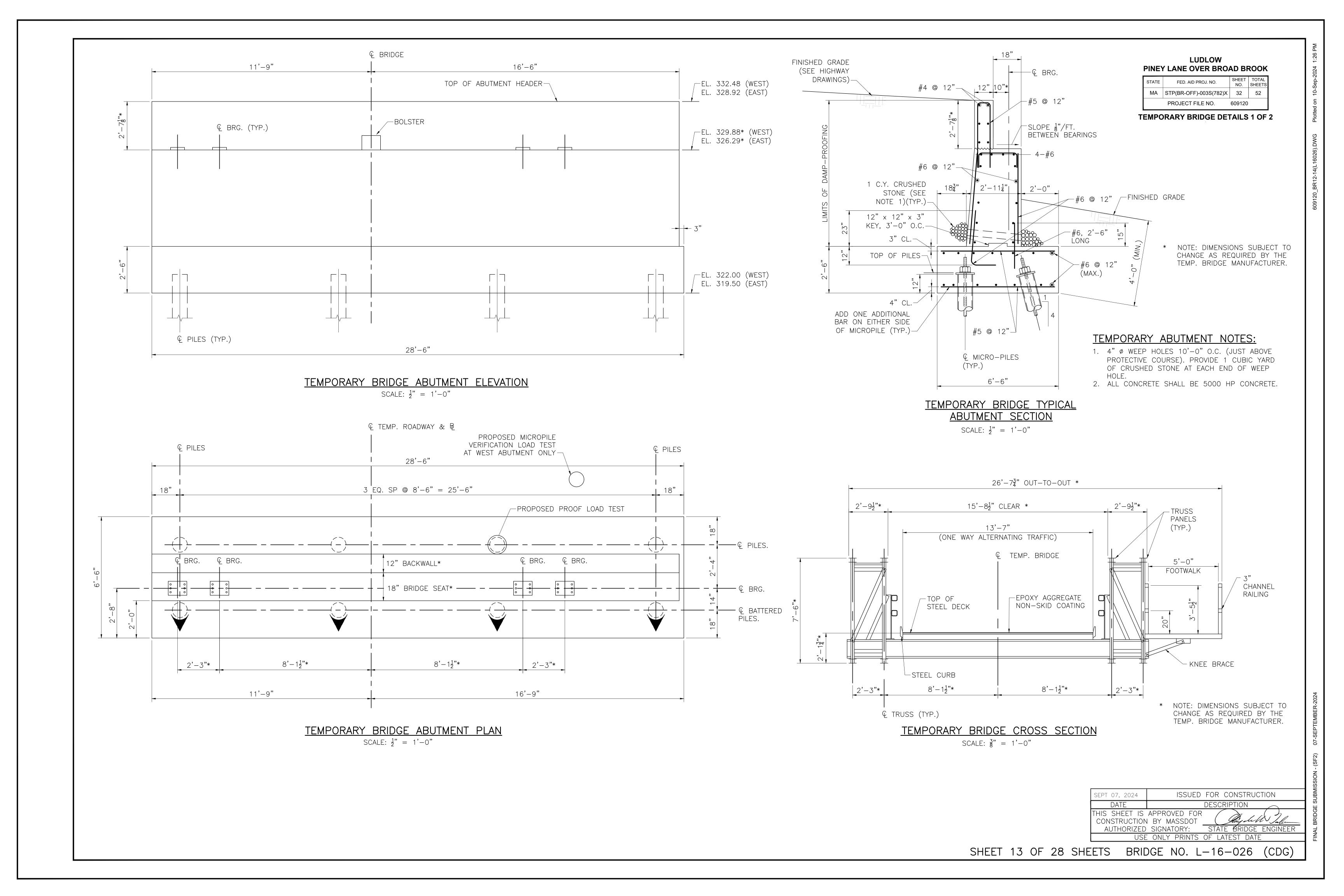
GEI PROJECT NUMBER: 2101263

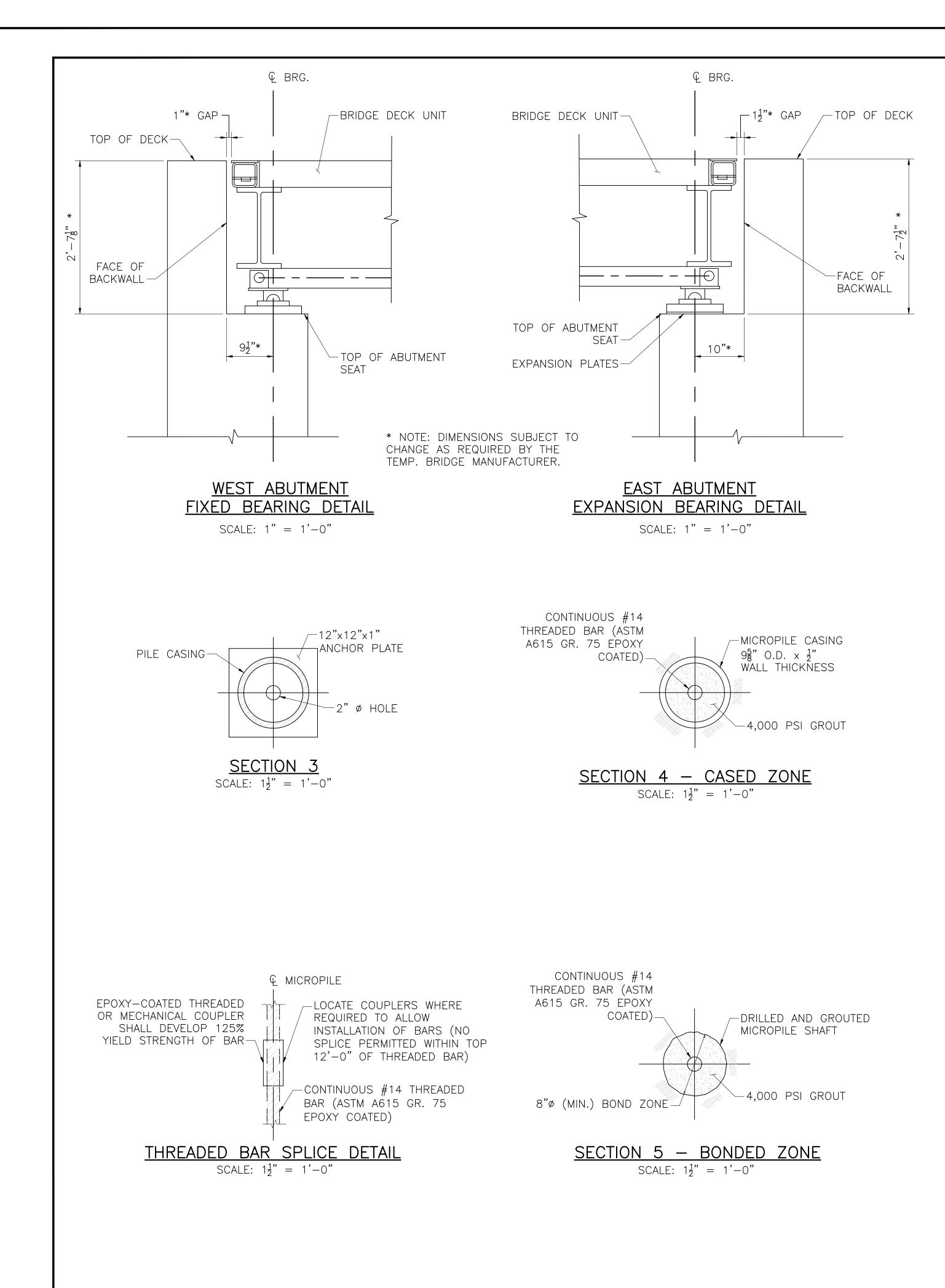


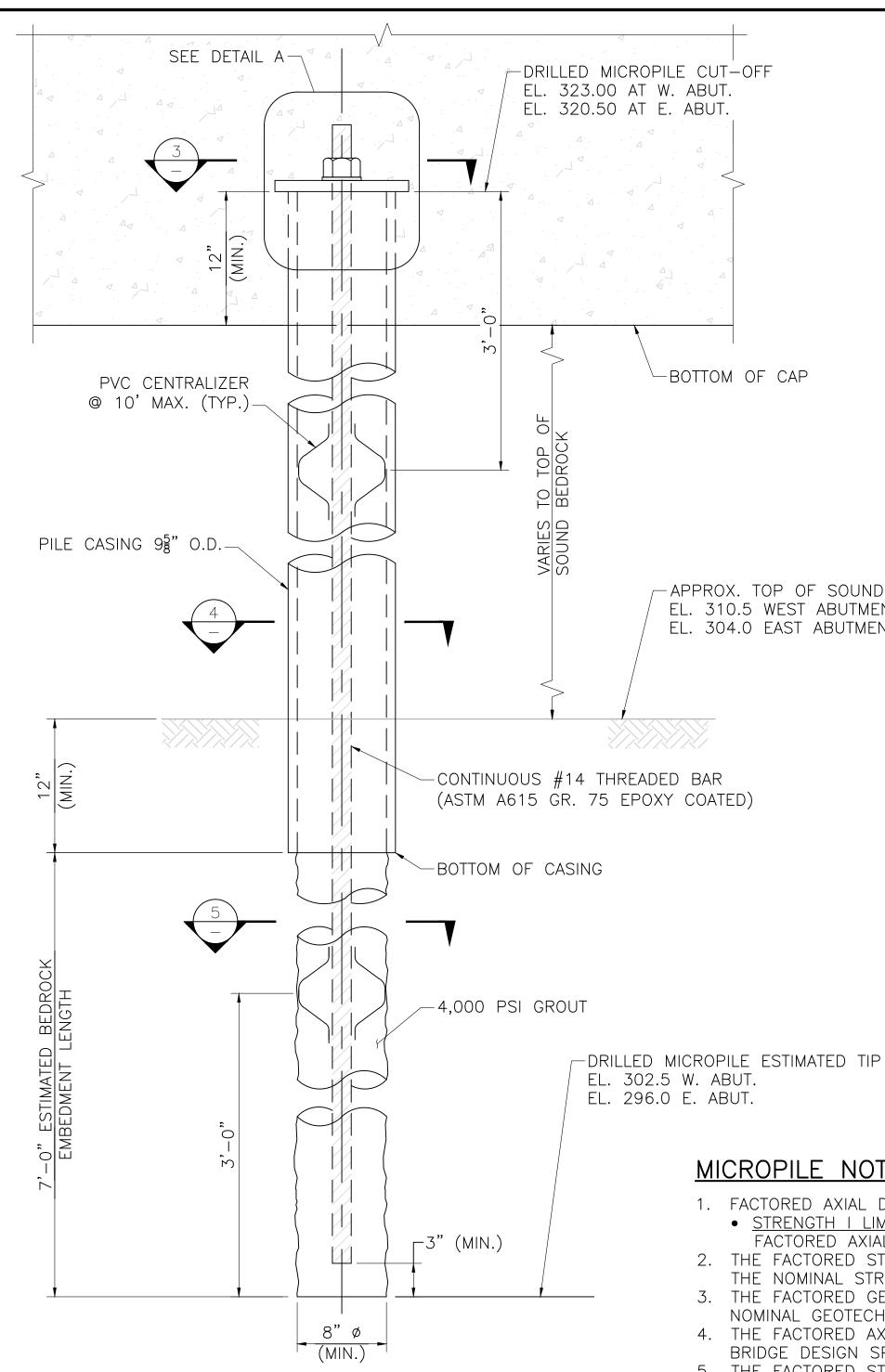












VERTICAL SECTION THROUGH PILE SCALE: $1\frac{1}{2}$ " = 1'-0"

REACTIONS PER ABUTMENT		
LOADS	UNFACTORED (KIPS)	
DC	56	
DW	2	
1*HL-93 TRUCK	64	
1*HL-93 LANE	26	
PEDESTRIAN LIVE LOAD	17	
WIND LOAD	12	
BRAKING	11	
FRICTION	6	

NOTE: LIVE LOAD VALUES SHOWN DO NOT INCLUDE DYNAMIC ALLOWANCE FACTOR OR MULTIPLE PRESENCE FACTOR. ALL LOADS ARE ACTING VERTICALLY DOWNWARDS EXCEPT FOR WIND WHICH IS ACTING TRANSVERSELY TO THE BRIDGE AND BRAKING AND FRICTION WHICH ARE ACTING LONGITUDINALLY ALONG THE BRIDGE.

9_8^{5} " O.D. $\times \frac{1}{2}$ " WALL THICKNESS - APPROX. TOP OF SOUND BEDROCK DETAIL A SCALE: $1\frac{1}{2}$ " = 1'-0" MICROPILE ROCK SOCKET DATA NOMINAL AXIAL COMPRESSIVE RESISTANCE 395.2 KIPS FACTORED AXIAL COMPRESSIVE RESISTANCE 201.5 KIPS MINIMUM REQUIRED STATIC TEST LOAD 230.7 KIPS 1. FACTORED AXIAL DESIGN LOAD PER PILE PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

LUDLOW

PINEY LANE OVER BROAD BROOK

TEMPORARY BRIDGE DETAILS 2 OF 2

COATED)

CONTINUOUS #14

TACK WELD, ALL

THE WAY AROUND

MICROPILE CASING

THREADED BAR (ASTM A615 GR. 75 EPOXY

FED. AID PROJ. NO.

PROJECT FILE NO.

MA STP(BR-OFF)-003S(782)X 33

MICROPILE NOTES:

-BOTTOM OF CAP

EL. 310.5 WEST ABUTMENT

EL. 304.0 EAST ABUTMENT

- STRENGTH | LIMIT STATE
- FACTORED AXIAL COMPRESSION DESIGN LOAD = 161.5 KIPS.

FOR CONCRETE BREAKOUT, THE CONTROLLING FAILURE MODE.

THREADED BAR NUT-

12"x12"x1"

ANCHOR PLATE -

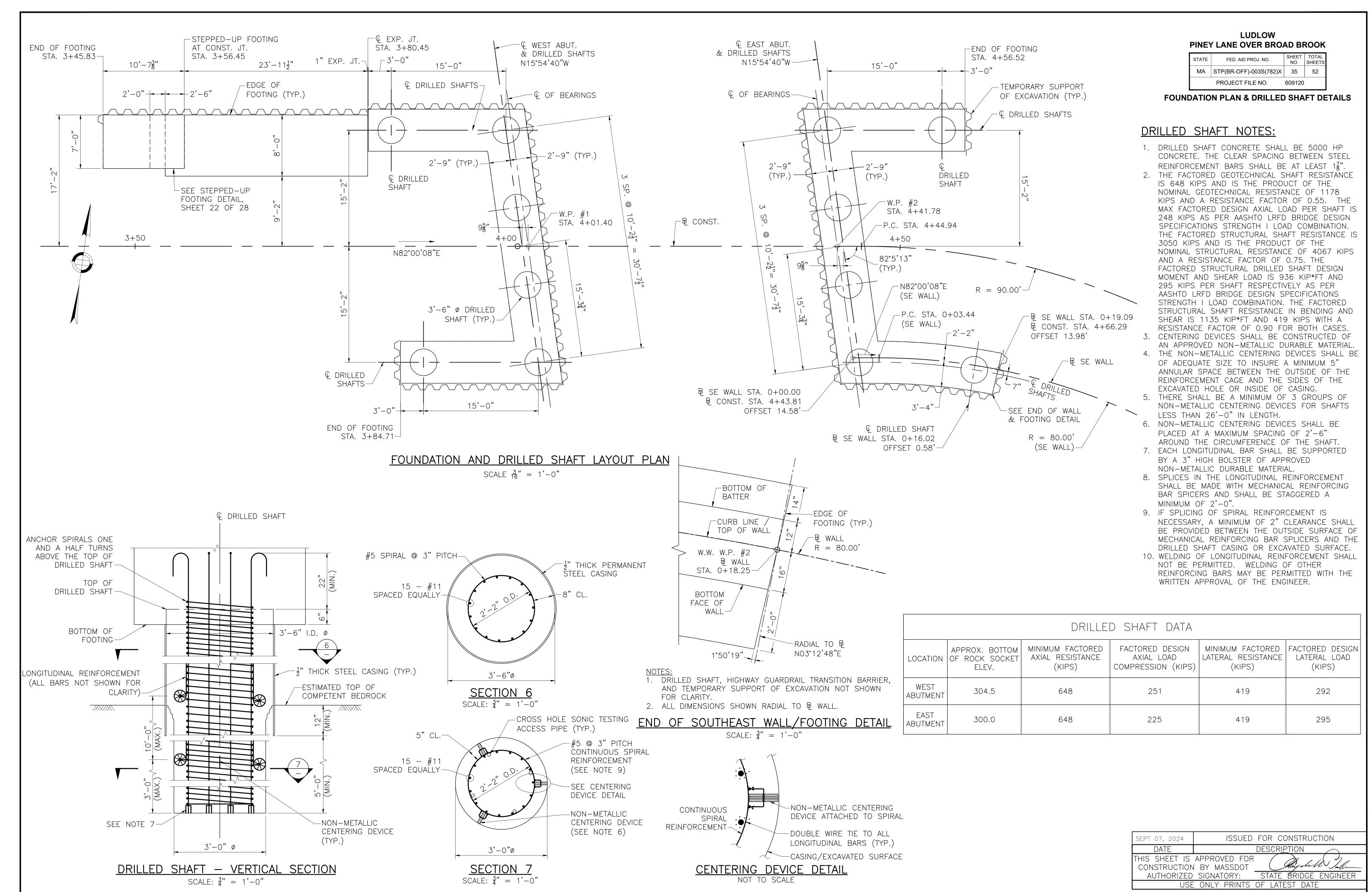
- 2. THE FACTORED STRUCTURAL PILE RESISTANCE PER PILE IS 397.9 KIPS AND IS THE PRODUCT OF THE NOMINAL STRUCTURAL RESISTANCE OF 530.6 KIPS AND A RESISTANCE FACTOR OF 0.75.
- 3. THE FACTORED GEOTECHNICAL PILE RESISTANCE IS 201.5 KIPS AND IS THE PRODUCT OF THE
- NOMINAL GEOTECHNICAL RESISTANCE OF 366.5 KIPS AND A RESISTANCE FACTOR OF 0.55.
- 4. THE FACTORED AXIAL TENSION DESIGN LOAD PER PILE IS 32.9 KIPS AS PER AASHTO LRFD
- BRIDGE DESIGN SPECIFICATIONS STRENGTH I LOAD COMBINATION.
- 5. THE FACTORED STRUCTURAL TENSION RESISTANCE PER PILE IS 113.6 KIPS AND IS THE PRODUCT OF THE NOMINAL STRUCTURAL RESISTANCE OF 162.3 KIPS AND A RESISTANCE FACTOR OF 0.70
- 6. THE ESTIMATED TIP ELEVATION SHALL BE AS SHOWN ON THE VERTICAL SECTION THROUGH PILE. 7. STEEL CASING SHALL BE PRIME STEEL AND MEET THE REQUIREMENTS OF API 5L PSL1 GRADE 52
- WITH SR 15 SUPPLEMENTAL REQUIREMENTS.
- 8. THREADED STEEL BAR SHALL BE CONTINUOUSLY THREADED FOR THE ENTIRE BAR LENGTH CONFORMING TO AASHTO M31, HAVING A MINIMUM YIELD STRENGTH OF 75 KSI.
- 9. THREADED CASING JOINTS ARE NOT ALLOWED WITHIN 3'-0" OF THE PILE CAP.
- 10. NUT AND BAR COUPLING SHALL BE PROVIDED FROM THE SAME MANUFACTURER AS THE THREADED
- STEEL BAR. 11. BAR COUPLING SHALL BE FULLY ENGAGED ON THE THREADED STEEL BAR AND SHALL NOT BE
- LOCATED IN THE TOP THIRD OF THE MICROPILE LENGTH.
- 12. ANCHOR PLATE SHALL MEET THE REQUIREMENTS OF AASHTO M270 GRADE 50. 13. GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4000 PSI AND CEMENT
- SHALL CONFORM TO AASHTO M85 TYPE I. 14. GROUT SHALL BE PLACED USING TREMIE METHODS.
- 15. THE CONTRACTOR SHALL SUBMIT A MICROPILE SCHEDULE, MICROPILE INSTALLATION, AND MICROPILE TESTING PLAN FOR REVIEW AND APPROVAL BY THE ENGINEER.
- 16. SEE SPECIAL PROVISION ITEM 945.10 DRILLED MICROPILES, ITEM 948.60 MICROPILE VERIFICATION LOAD TEST, AND ITEM 948.61 MICROPILE PROOF LOAD TEST FOR ADDITIONAL MICROPILE SPECIFICATIONS.

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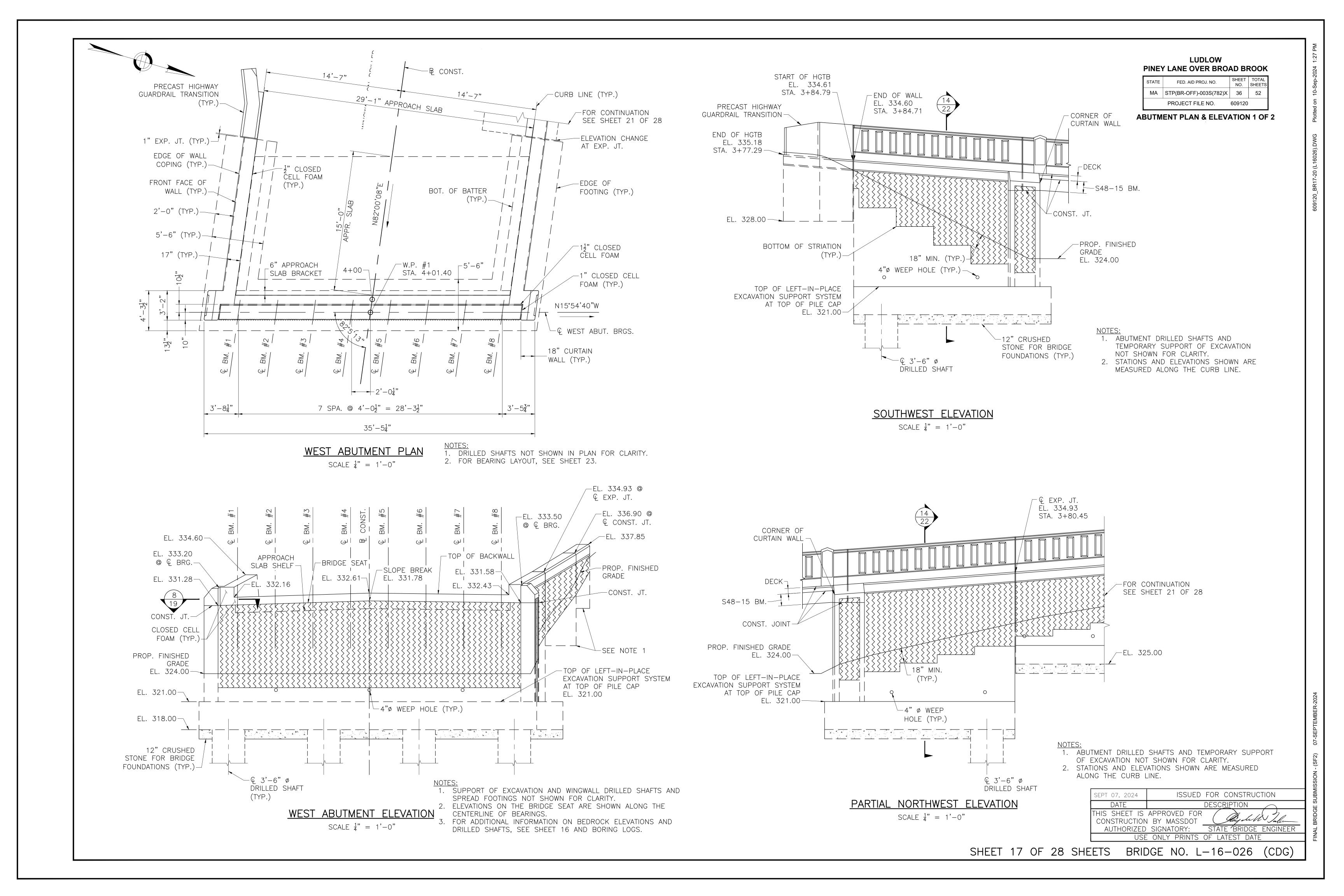
LUDLOW PINEY LANE OVER BROAD BROOK STATE FED. AID PROJ. NO. MA STP(BR-OFF)-003S(782)X 34 52 PROJECT FILE NO. 609120 **CHANNEL SECTION** ISSUED FOR CONSTRUCTION SEPT 07, 2024 DESCRIPTION DATE THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER USE ONLY PRINTS OF LATEST DATE

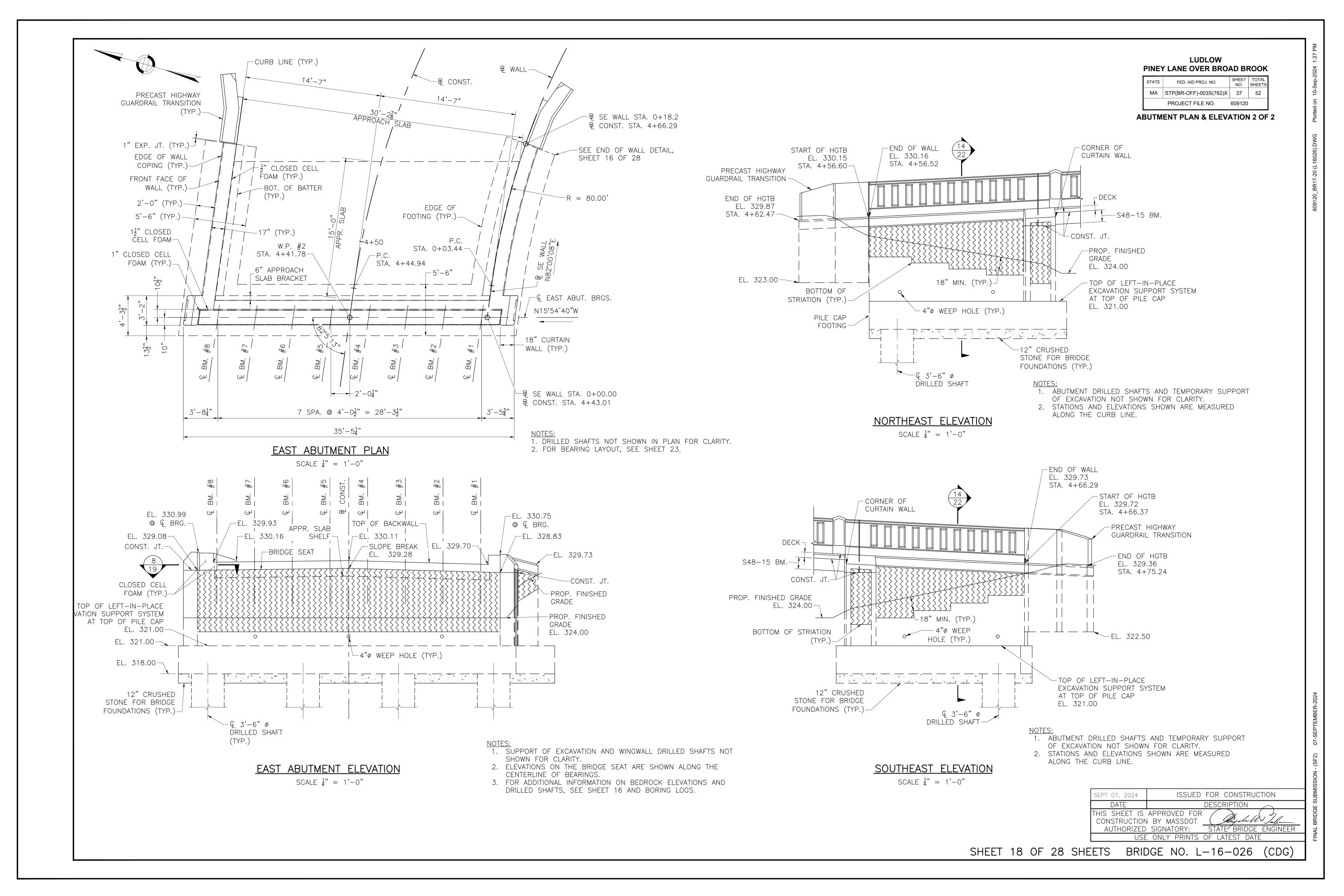
SHEET 15 OF 28 SHEETS BRIDGE NO. L-16-026 (CDG)

₽ BRG. ₽ BRG. W. ABUT. E. ABUT. CLEAR SPAN 38'-83" (SKEWED) 38'-4" (SQUARE) MODIFIED ROCKFILL (M2.02.4) TOP OF LEFT-IN-PLACE EXCAVATION AND FILL VOIDS WITH CRUSHED SUPPORT SYSTEM FOR WINGWALL STONE (M2.01.4) — AND ABUTMENT TO BE CUT AT TOP - ABUTMENT STEM (TYP.) OF PILE CAP CONTINUE 12" SEDIMENT EL. VARIES, 3'-0" DESIGN (10 YEARS) FROM STREAMBED (TYP.) 3 FT THICK MIN. - PROPOSED GEOTEXTILE FABRIC FOR SEPARATION EL. 320.2± AT NORTH 18" PROPOSED SEDIMENT FROM PILE CAP (TYP.) — FASCIA AND EL. 319.5± EXISTING STREAMBED OVER PROP. 1.8 AT SOUTH FASCIA -4'-0" THICK RIPRAP (M2.02.0) OVER 12" CRUSHED STONE FOR CHANNEL RECONSTRUCTION (M2.01.2)BOT. OF PILE CAP EL. 318.00 (TYP.) 3'-6"Ø DRILLED — SHAFT (TYP.) ─ 12" CRUSHED STONE (M2.01.4) FOR BRIDGE FOUNDATION (TYP.) GEOTEXTILE FABRIC FOR SEPARATION SOUTH FASCIA LOOKING NORTH (NORTH FASCIA LOOKING SOUTH SIMILAR) SCALE: $\frac{3}{8}$ = 1'-0"



SHEET 16 OF 28 SHEETS BRIDGE NO. L-16-026 (CDG)







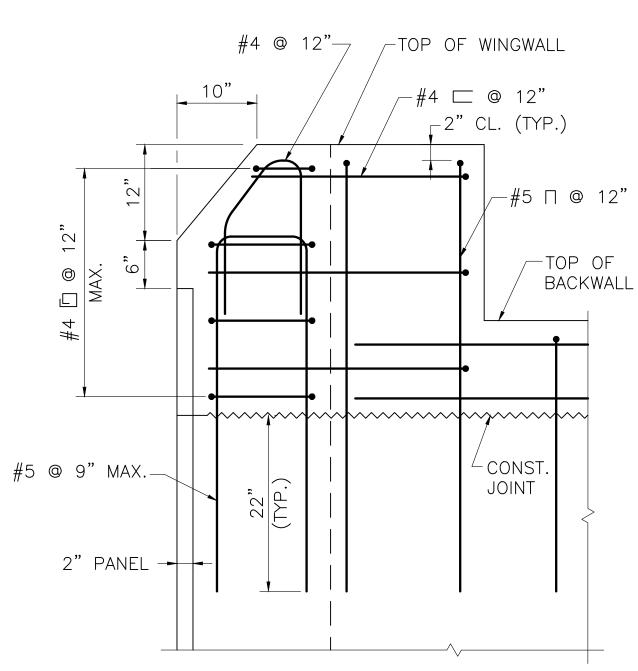
- 2. ALL REINFORCING SHOWN IN THIS DETAIL SHALL REINFORCEMENT.
- 3. ATTACH CLOSED CELL FOAM TO BACK OF PRECAST BEAM WITH ADHESIVE.
- 4. BACKWALL CONCRETE SHALL BE 5000 HP CONCRETE AND SHALL BE PLACED AFTER ALL BEAMS HAVE BEEN ERECTED.
- 5. DRAPE MEMBRANE WATERPROOFING OVER CLOSED CELL FOAM BACKER ROD.
- 6. FOR BEARING PAD LAYOUT AND DIMENSIONS, SEE SHEET 23.

┌1" RADIUS $-1\frac{1}{2}$ " ø FOAM BACKER ROD (SEE NOTE 5) PAVEMENT SAWCUT DETAIL (SEE DETAIL, THIS SHEET) -- HMA WEARING SURFACE DECK THICKNESS VARIES PROTECTIVE COURSE SEE SHEET 25) (SEE NOTE 1) -1" CHAMFER $1\frac{1}{2}$ " CLOSED CELL FOAM (SEE NOTE 3) — #4 @ 12" (TYP.) END OF MEMBRANE -2" CL. (TYP.) WATERPROOFING 1" CHAMFER 10" APPROACH SLAB -SLOPE #"/FT. BETWEEN BEARINGS 3-#6 CONST. JT. (RAKE FINISH)--#4 @ 12" #6 @ 18", 18" LONG #5 @ 12" 11<u>1</u>" **Q** BEARINGS

BRIDGE SEAT DETAIL SCALE: 1" = 1'-0"

BRIDGE SEAT NOTES:

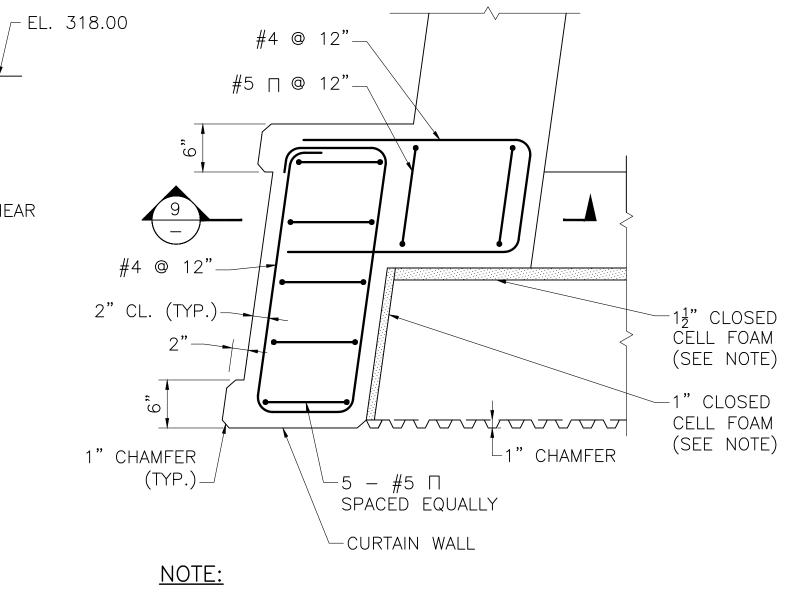
- 1. PROTECTIVE COURSE TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER WITHIN 12 HOURS AFTER PLACING MEMBRANE WATERPROOFING.
- BE COATED BARS, EXCEPT FOR APPROACH SLAB



NOTE:

REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

> SECTION 9 SCALE: 1" = 1'-0"



ATTACH CLOSED CELL FOAM TO THE BACK AND SIDE OF THE EXTERIOR PRECAST BEAM PRIOR TO PLACING THE CONCRETE FOR THE BACKWALL AND CURTAIN WALL.

> SECTION 8 SCALE: 1" = 1'-0"

PAVEMENT SAWCUT DETAIL NOT TO SCALE

HOT POURED 3" JOINT SEALER	
2 -1,2	
1"	

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TYPICAL ABUTMENT SECTION SCALE: $\frac{1}{2}$ " = 1'-0"

5'-6"

3'-1"

CONST. JT.

2" CL. TO VALLEY -

#6 @ 12" -

SEE NOTE 1

້າດ 5"___

#6 @ 12", 2'-6" LONG ~

6"EMBEDMENT—

19½" - 6"

- SEE BRIDGE

SEAT DETAIL

-#6 @ 12**"**

- #6 @ 12**"**

-#6 @ 9**"**

┌ 7" CL.

-#6 @ 12**"**

TOP AND BOT.

WITH 12"x12"x2" SHEAR

KEYS @ 3'-0" O.C.

* CONST. JT.

3" CL. (TYP.)

- APPROACH SLAB

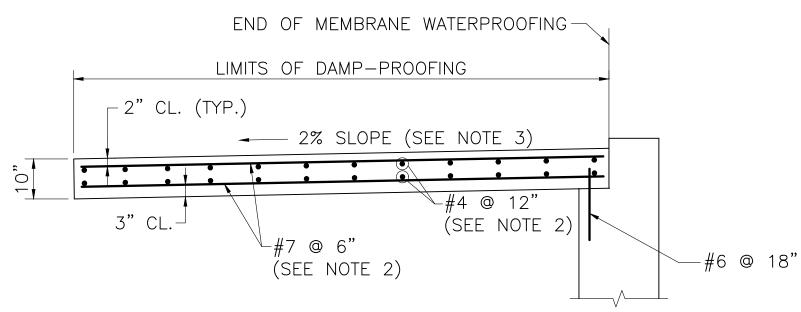
ABUTMENT SECTION NOTES:

1. 4" Ø WEEP HOLES 10'-0" O.C. (JUST ABOVE PROTECTIVE COURSE). PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE.

2'-9"

€ FOOTING & DRILLED SHAFTS

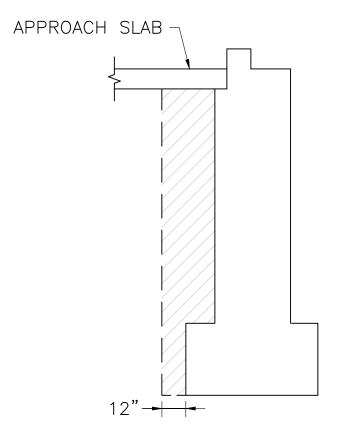
2. ALL CONCRETE SHALL BE 5000 HP CONCRETE.



NOTES:

- 1. APPROACH SLAB TO BE 5000 HP CONCRETE.
- 2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO ROADWAY ALIGNMENT. PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.
- 3. WEST APPROACH SLAB SHOWN. SET EAST APPROACH SLAB SLOPE AT 6%.

APPROACH SLAB DETAILS SCALE: ½" = 1'-0"



NOTE:

HATCHED AREA INDICATES LIMITS OF GRAVEL BORROW FOR BACKFILLING.

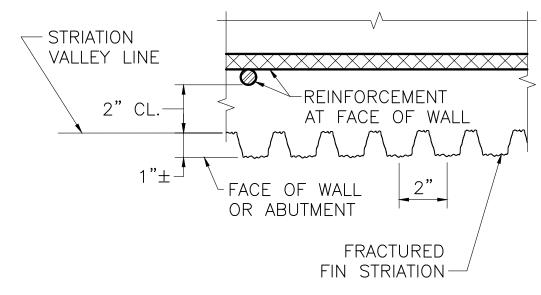
LIMITS OF GRAVEL BORROW FOR BACKFILLING AT ABUTMENT

SCALE: $\frac{1}{4}$ " = 1'-0"

LIMITS OF GRAVEL BORROW FOR BACKFILLING AT WINGWALL

SCALE: \(\frac{1}{4} \) = 1'-0"

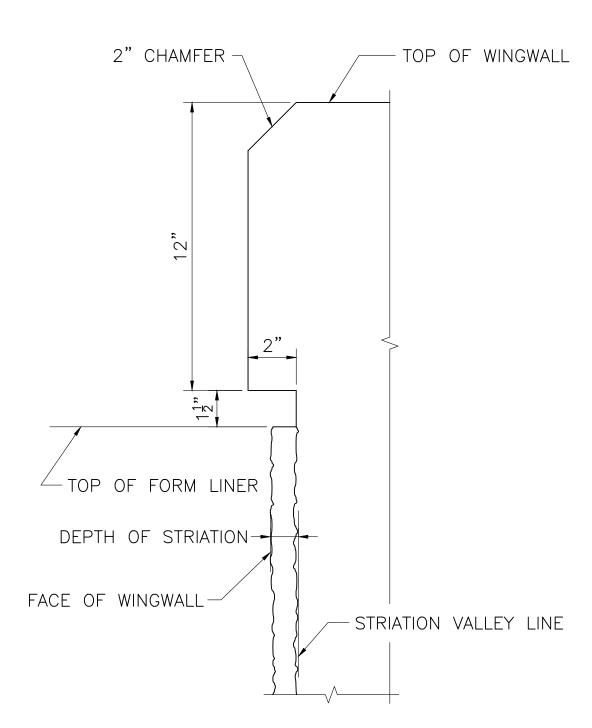
- PROPOSED SLOPE



STRIATION NOTES:

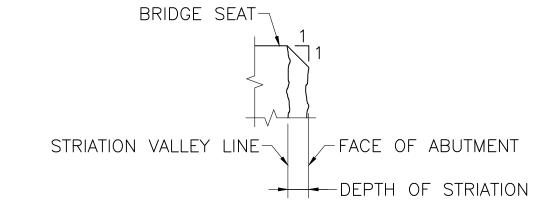
- 1. THE CONTRACTOR SHALL MAKE SURE THAT THE STRIATION FINS ARE PLUMB AND LINED UP VERTICALLY FROM PANEL TO PANEL FOR THE FULL HEIGHT OF THE WALL.
- 2. THE HORIZONTAL JOINT MAY BE OMITTED IF THE CONTRACTOR CAN DEMONSTRATE THAT THE FORM LINER PANELS CAN BE INSTALLED END TO END WITHOUT CREATING A VISIBLE SEAM IN THE FINAL CAST CONCRETE.

TYPICAL STRIATION DETAIL SCALE: 3" = 1'-0"

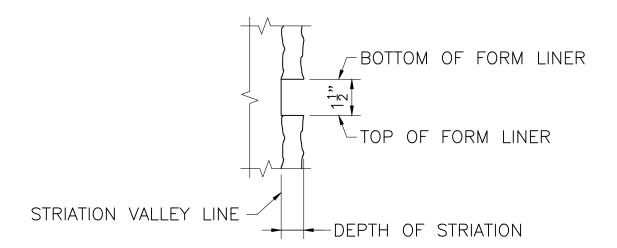


DETAIL AT TOP OF WINGWALL

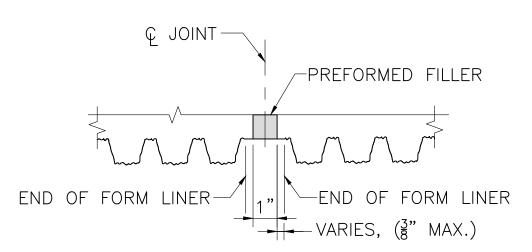
SCALE: 3" = 1'-0"



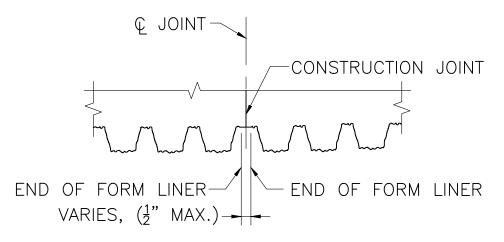
DETAIL AT BRIDGE SEAT SCALE: 3" = 1'-0"



HORIZONTAL PANEL JOINT SCALE: 3" = 1'-0"

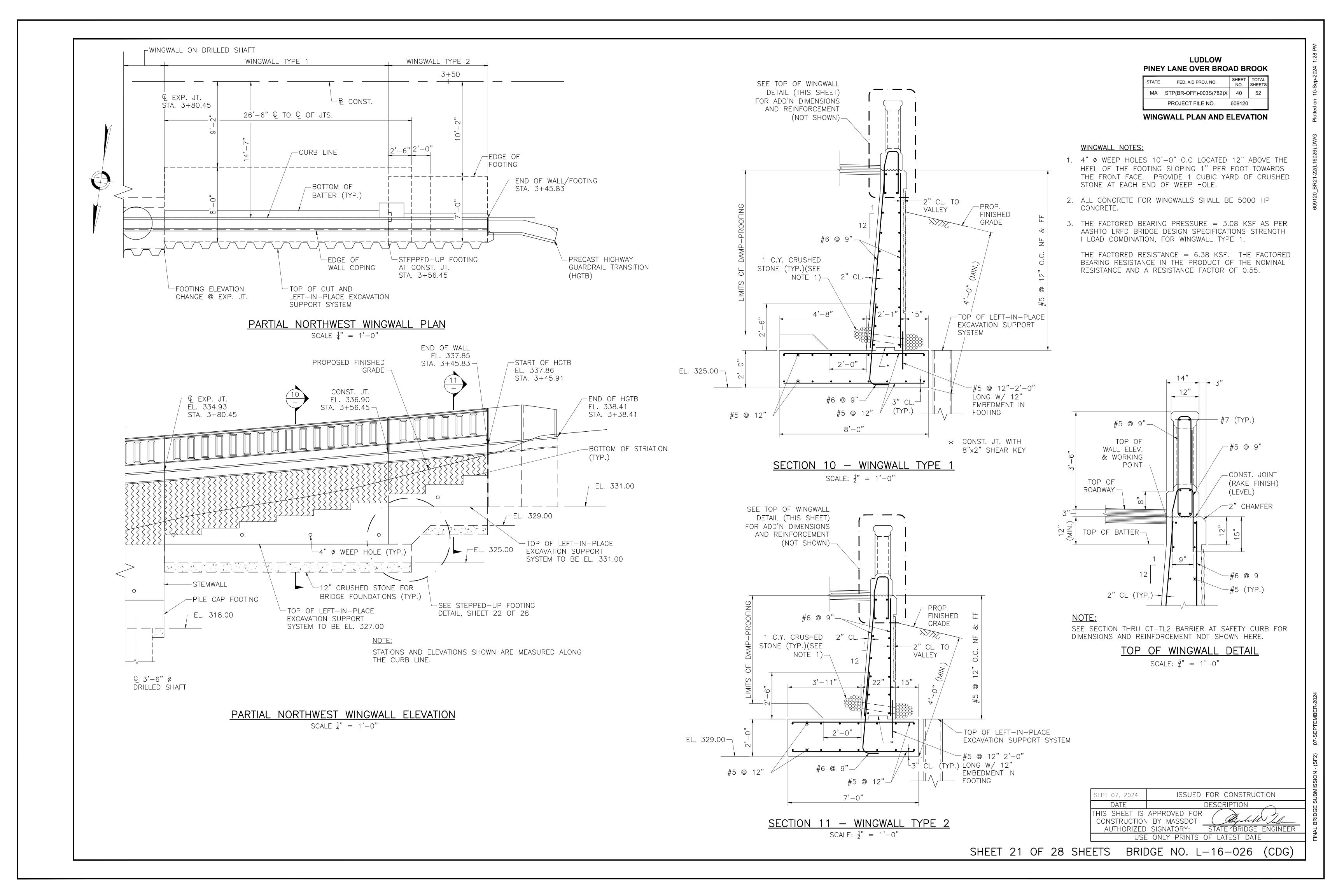


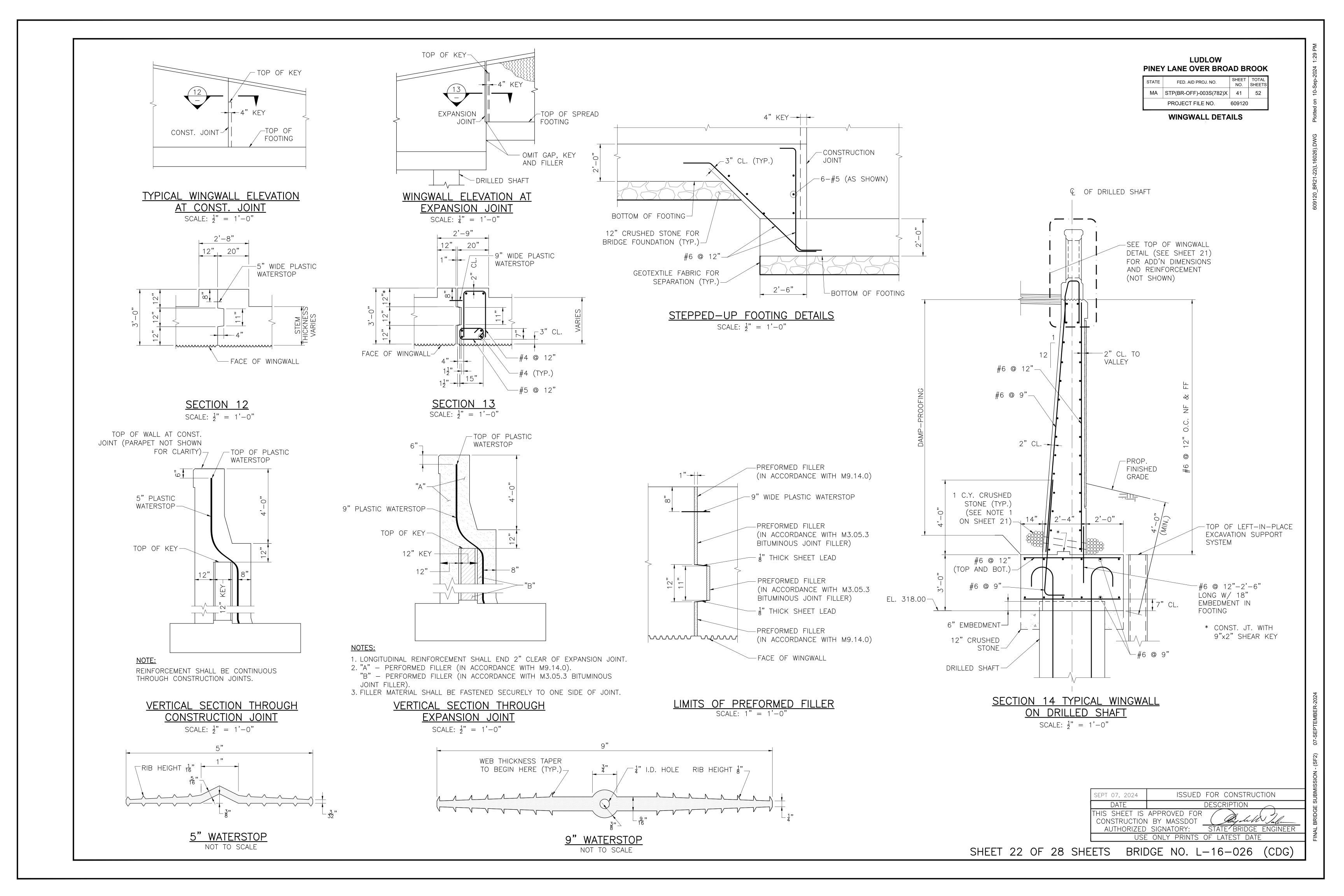
EXPANSION JOINT SCALE: 3" = 1'-0"



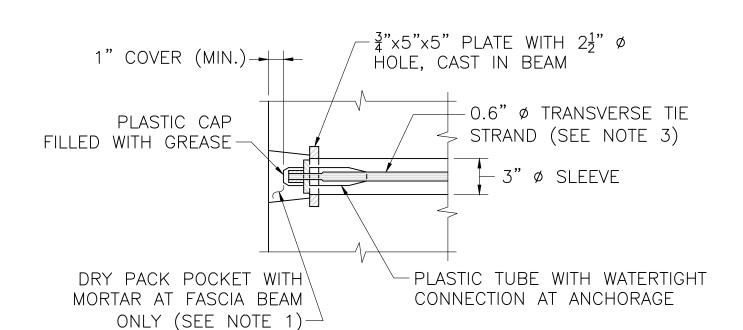
CONSTRUCTION JOINT SCALE: 3" = 1'-0"

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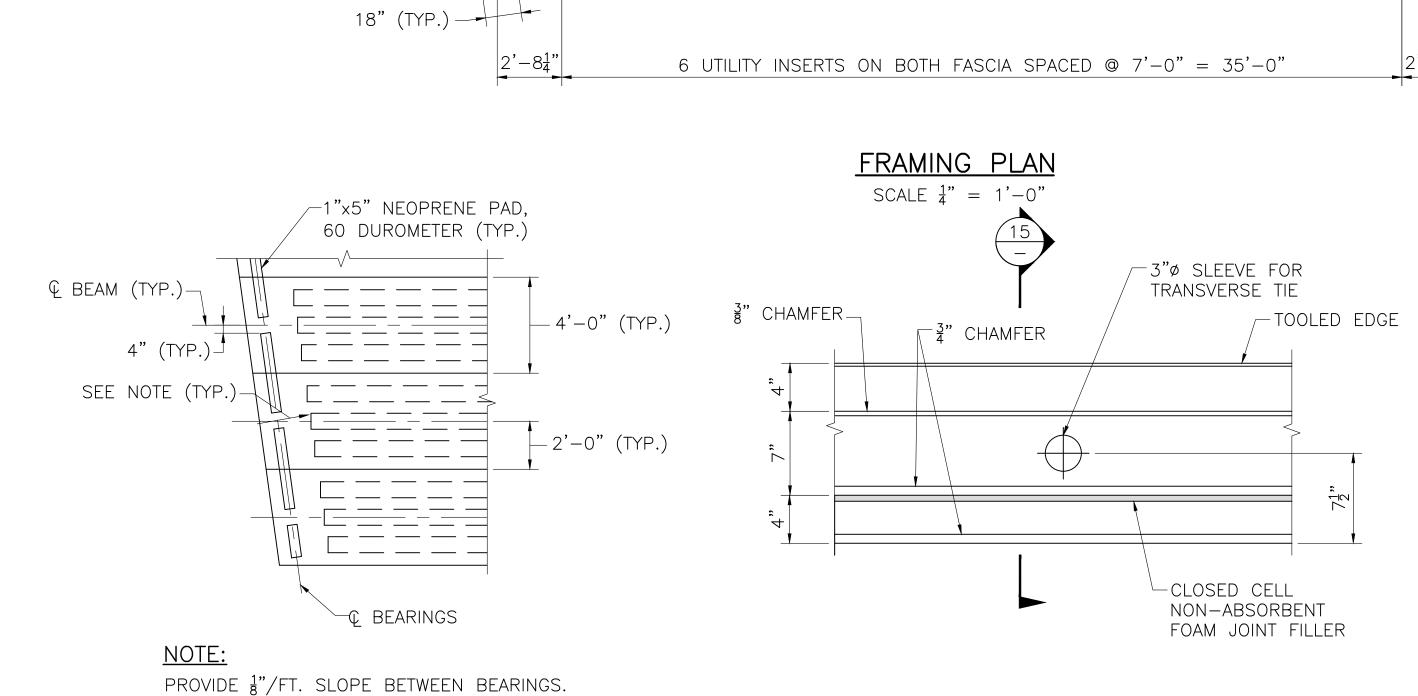
FRAMING PLAN



NOTES:

- 1. MORTAR FOR EXTERIOR POCKETS SHALL CONFORM TO M4.02.15 AND SHALL BE THE SAME COLOR AND TEXTURE AS THE BEAM CONCRETE.
- 2. OTHER ANCHORAGE SYSTEMS MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. ALTERNATE ANCHORAGE SYSTEMS SHALL BE WATERTIGHT AND CORROSION PROOF.
- 3. TRANSVERSE TIES SHALL BE COVERED BY A SEAMLESS POLYPROPYLENE SHEATH (WITH CORROSION INHIBITING GREASE BETWEEN THE STRAND AND SHEATH) FOR THE FULL LENGTH OF THE STRAND, EXCEPT AT THE ANCHORAGE LOCATION.
- 4. SEE SPECIAL PROVISIONS ITEM 995.01.

TRANSVERSE TIE ANCHORAGE SCALE: $1\frac{1}{2}$ " = 1'-0"



TYPICAL BEAM ELEVATION AT

TRANSVERSE TIE LOCATIONS

SCALE: $1\frac{1}{2}$ " = 1'-0"

 $41'-10\frac{3}{4}"$

7" (TYP.)

 $19' - 5\frac{1}{4}$ "

N82°00'08"E

W.P. #1

4+01.40-

LAYOUT OF BEARINGS

SCALE: $\frac{1}{4}$ " = 1'-0"

Q BRG. STA.

18<u>1</u>"

-SPACING OF F'S OF

3" Ø SLEEVES FOR TRANSVERSE TIES

> © BEARINGS EAST ABUTMENT

> > −見 CONST.

−W.P. #2

4+41.78

9" (TYP.)

Q BRG. STA.

PINEY LANE

 $19'-5\frac{1}{4}"$

SECTION 15 - SHEAR KEY DETAIL

SCALE: $1\frac{1}{2}$ " = 1'-0"

CONSTRUCTION SEQUENCE NOTES:

- 1. AFTER ALL BEAMS HAVE BEEN ERECTED, TENSION EACH TRANSVERSE TIE TO 5 KIPS.
- 2. FILL ALL KEYWAYS WITH MORTAR (M4.04.0). IF THE KEYWAYS ARE NOT FILLED WITHIN FIVE (5) DAYS AFTER THE BEAMS ARE ERECTED, THE CONTRACTOR SHALL COVER AND PROTECT THE KEYWAYS FROM WEATHER AND DEBRIS UNTIL THEY ARE FILLED.
- 3. AFTER THE MORTAR HAS CURED (24 HOURS MINIMUM), TENSION EACH TRANSVERSE TIE TO 44 KIPS.
- 4. CONCRETE FOR DECK SLAB SHALL BE 5000 HP CONCRETE AND SHALL BE PLACED AFTER THE TRANSVERSE TIES HAVE BEEN FULLY TENSIONED.
- 5. NO TRAFFIC OR HEAVY EQUIPMENT WILL BE PERMITTED ON THE BRIDGE UNTIL ALL TRANSVERSE TIES HAVE BEEN PROPERLY TENSIONED AND THE DECK HAS BEEN CAST AND CURED PER THE STANDARD SPECIFICATIONS.

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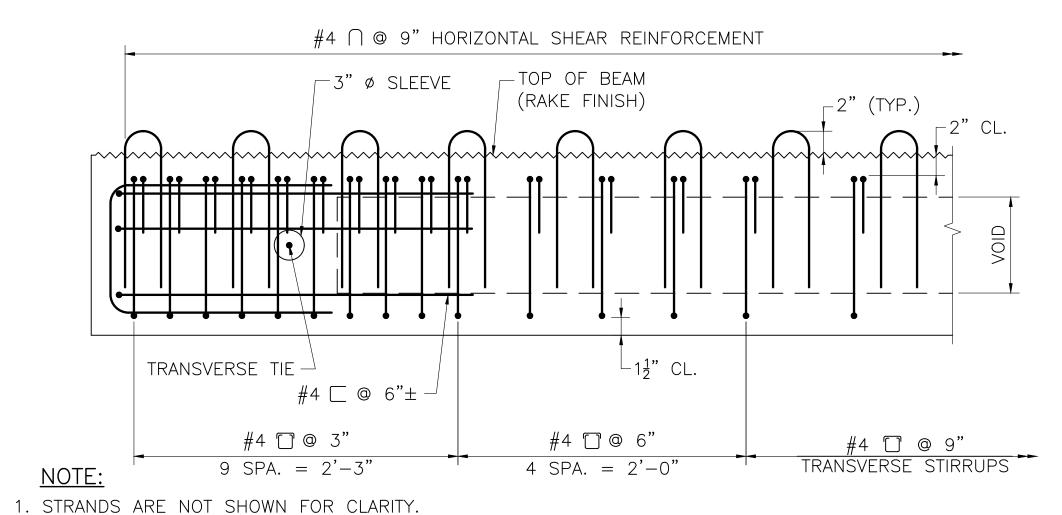
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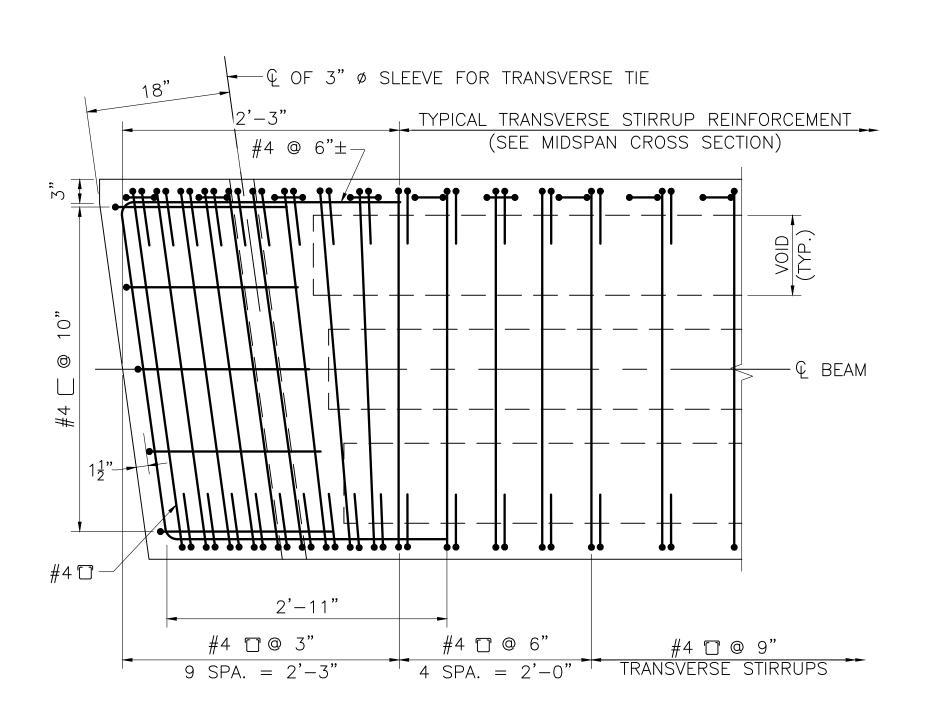
_#4 □ (TYP.)

─#4 *□*



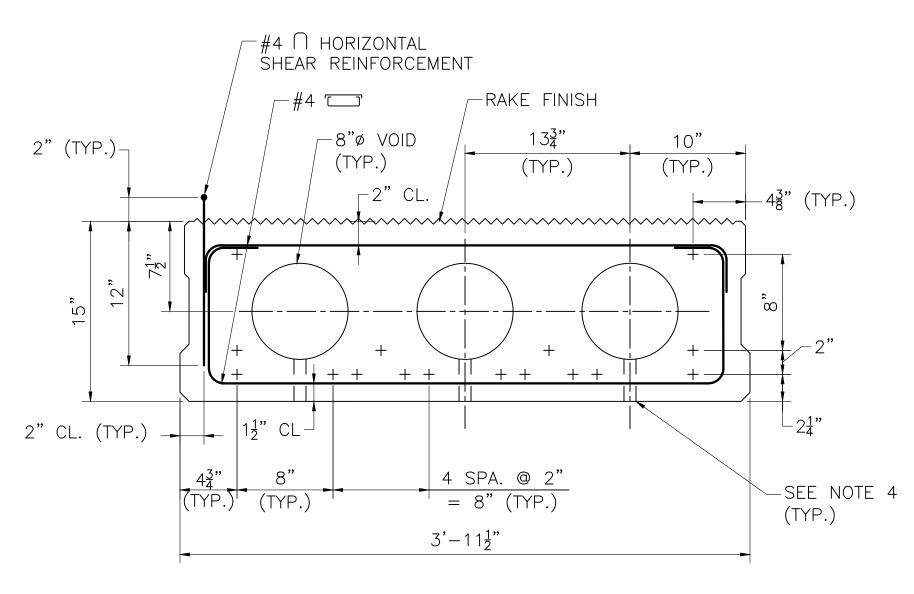
LONGITUDINAL SECTION

SCALE $1\frac{1}{2}$ " = 1'-0"

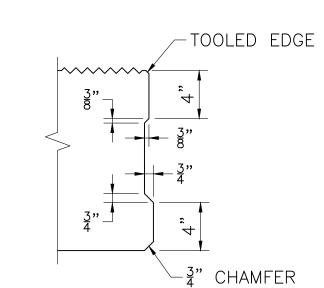


END OF BEAM PLAN

SCALE 1" = 1'-0"



TYPICAL MIDSPAN SECTION SCALE $1\frac{1}{2}$ " = 1'-0"

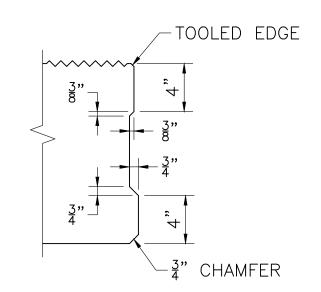


2'-2"

NOTES:

- 1. CONTRACTOR MAY SUBMIT ABOVE STIRRUP PATTERN TO THE ENGINEER FOR APPROVAL PROVIDED THAT THE ABOVE CRITERIA IS MET.
- 2. MAINTAIN ALL CLEARANCES AS SHOWN ON THE PLANS.

ALTERNATE STIRRUP PATTERN SCALE: $1\frac{1}{2}$ " = 1'-0"



SHEAR KEY DETAIL SCALE: $1\frac{1}{2}$ " = 1'-0"

END OF BEAM SECTION SCALE $1\frac{1}{2}$ " = 1'-0"

 $3'-11\frac{1}{2}"$

4 SPA. @ 2"

= 8" (TYP.)

 $-3-\#4 \square AS SHOWN$

(SEE NOTE 7)

BEAM SECTION NOTES:

(TYP.) (TYP.)

1. + DENOTES STRAIGHT STRANDS.

-- #4 ∩ HORIZONTAL SHEAR REINFORCEMENT

RAKE FINISH-

2" (TYP.)-

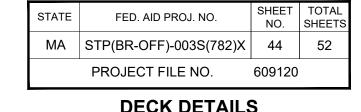
2" CL. (TYP.) $\frac{1}{2}$ " CL-

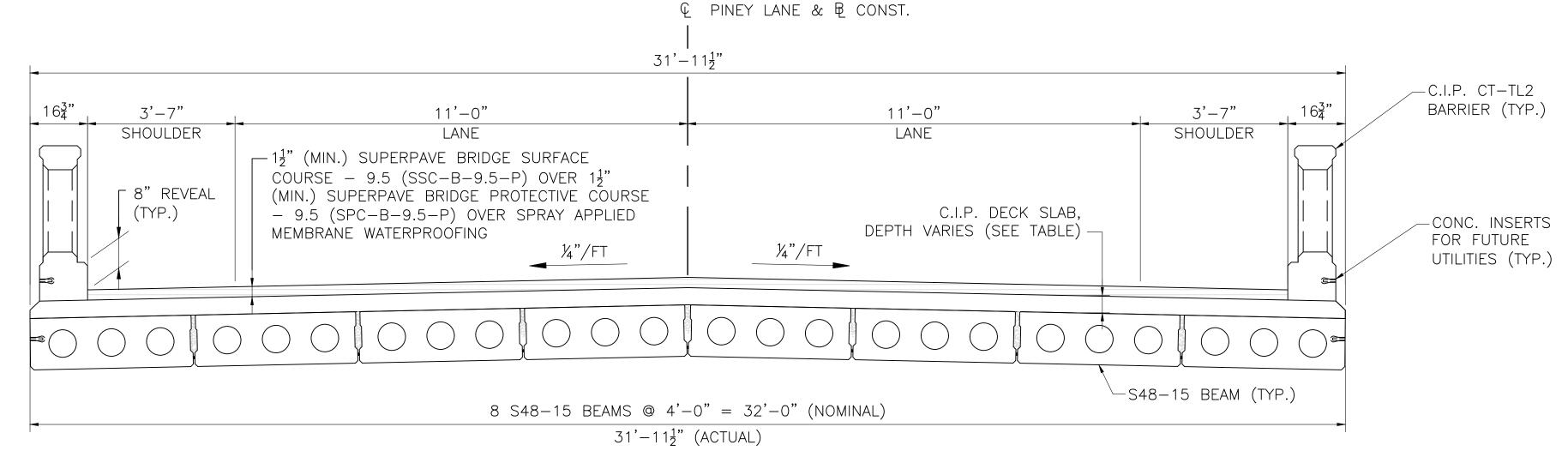
- 2. SEE SHEAR KEY DETAIL BELOW.
- 3. SEE END OF BEAM PLAN FOR STIRRUP SPACING.
- 4. 1" Ø DRAIN, PLACED AT BOTH ENDS OF EACH VOID.
- 5. MAINTAIN ALL CLEARANCES AS SHOWN ON THE PLANS.
- 6. SECTIONS SHOWN LOOKING UP STATION.
- 7. ADJUST REINFORCEMENT SPACING AS NECESSARY TO AVOID CONFLICT WITH TRANSVERSE TIE.

PRESTRESS NOTES:

- 1. ALL PRETENSIONING ELEMENTS SHALL BE 0.6" Ø, UNCOATED, SEVEN-WIRE, LOW RELAXATION STEEL STRANDS AND SHALL CONFORM TO AASHTO M 203.
- 2. THE TENSILE STRENGTH OF THE PRETENSIONING STRANDS SHALL BE 270 KSI.
- 3. THE INITIAL TENSION PER 0.6" Ø STRAND SHALL BE 44 KIPS.
- 4. THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 6500 PSI.
- 5. NO PRESTRESS SHALL BE TRANSFERRED TO THE CONCRETE UNTIL IT HAS ATTAINED A COMPRESSIVE STRENGTH, AS SHOWN BY CYLINDER TEST, OF AT LEAST 4500 PSI.
- 6. THE TOP OF ALL BEAMS SHALL BE GIVEN A RAKE FINISH $\binom{1}{4}$ " AMPLITUDE) ACROSS THE WIDTH (PERPENDICULAR TO THE BEAM'S AXIS).
- 7. THE FABRICATOR IS FULLY RESPONSIBLE FOR THE DESIGN OF THE LIFTING DEVICES WHICH SHALL BE ADEQUATE FOR THE SAFETY FACTORS REQUIRED BY THE ERECTION PROCEDURE.

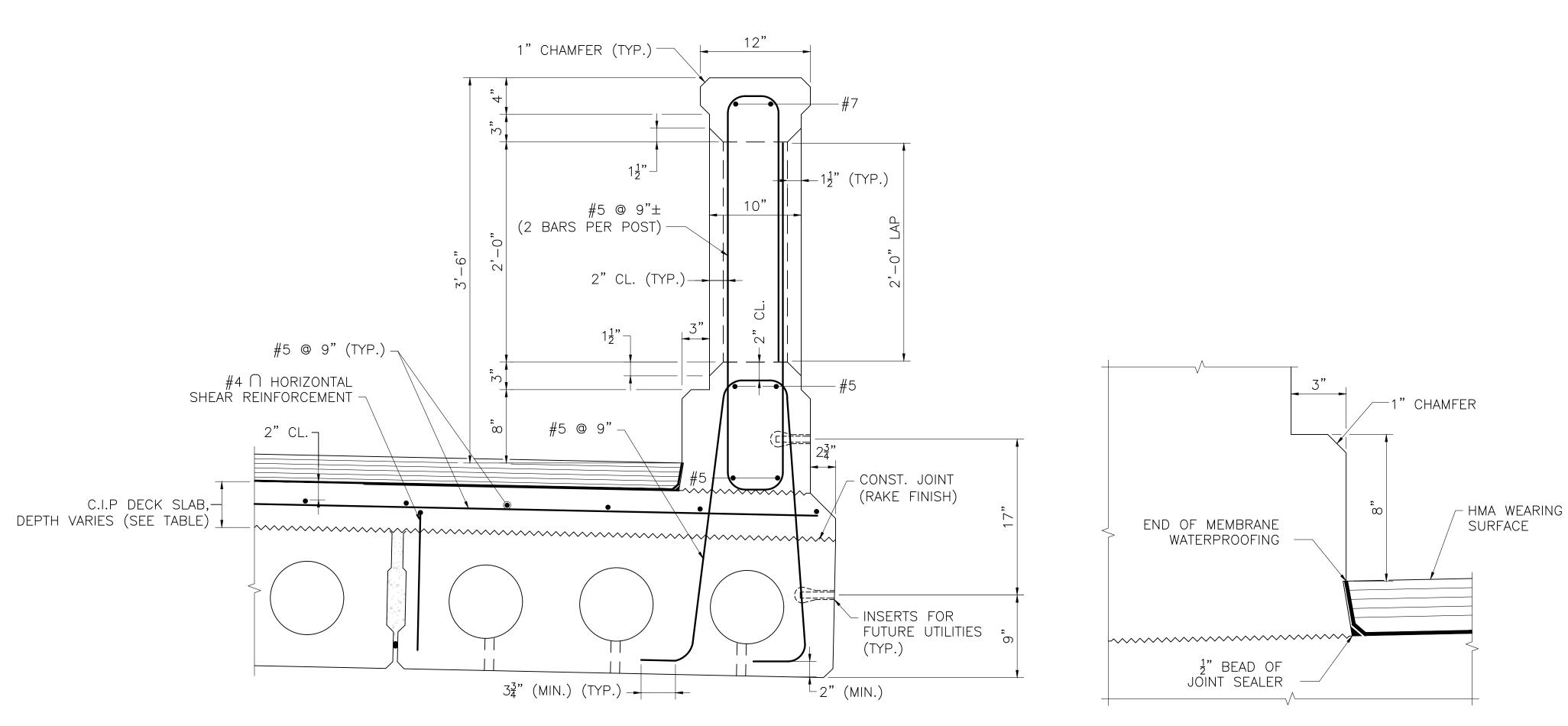
SEPT 07, 2024	ISSUED FOR CONSTRUCTION
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TRANSVERSE CROSS SECTION

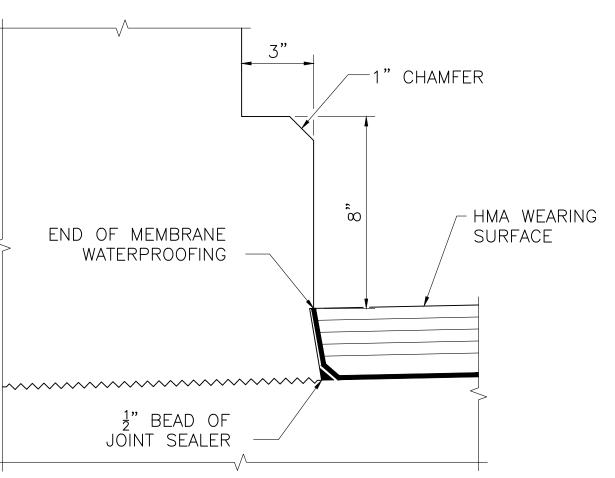
SCALE: $\frac{1}{2}$ " = 1'-0"



SECTION THRU SAFETY CURB SCALE: $1\frac{1}{2}$ " = 1'-0"

NOTE:

1. DECK SLAB SHALL BE 5000 PSI HP CONCRETE. CONCRETE FOR CT-TL2 BARRIER SHALL BE 5000 PSI §"IN, 710 HP CONCRETE. SEE SHEET 2, GENERAL



FACE OF SAFETY CURB DETAILS SCALE: 3'' = 1'-0''

	TOP (OF ROADWAY	ELEVATIONS		
LOCATION	₽ W. ABUT.	1/4 POINT	1/2 POINT	3/4 POINT	€ E. ABUT.
NORTH CURBLINE	333.77	333.08	332.44	331.83	331.26
CROWN	333.93	333.25	332.62	332.02	331.46
SOUTH CURBLINE	333.49	332.82	332.19	331.60	331.05

LOCATION	LEFT EDGE OF DECK SLAB	PROFILE GRADE LINE/CROWN	RIGHT EDGE OF DECK SLAB
© BRGS. W ABUT.	7.11"	6.85"	7.02"
MIDSPAN	5.44"	5.35"	5.47"
€ BRGS. E ABUT.	6.96"	7.11"	7.11"

THEORETICAL DECK SLAB THICKNESS TABLE

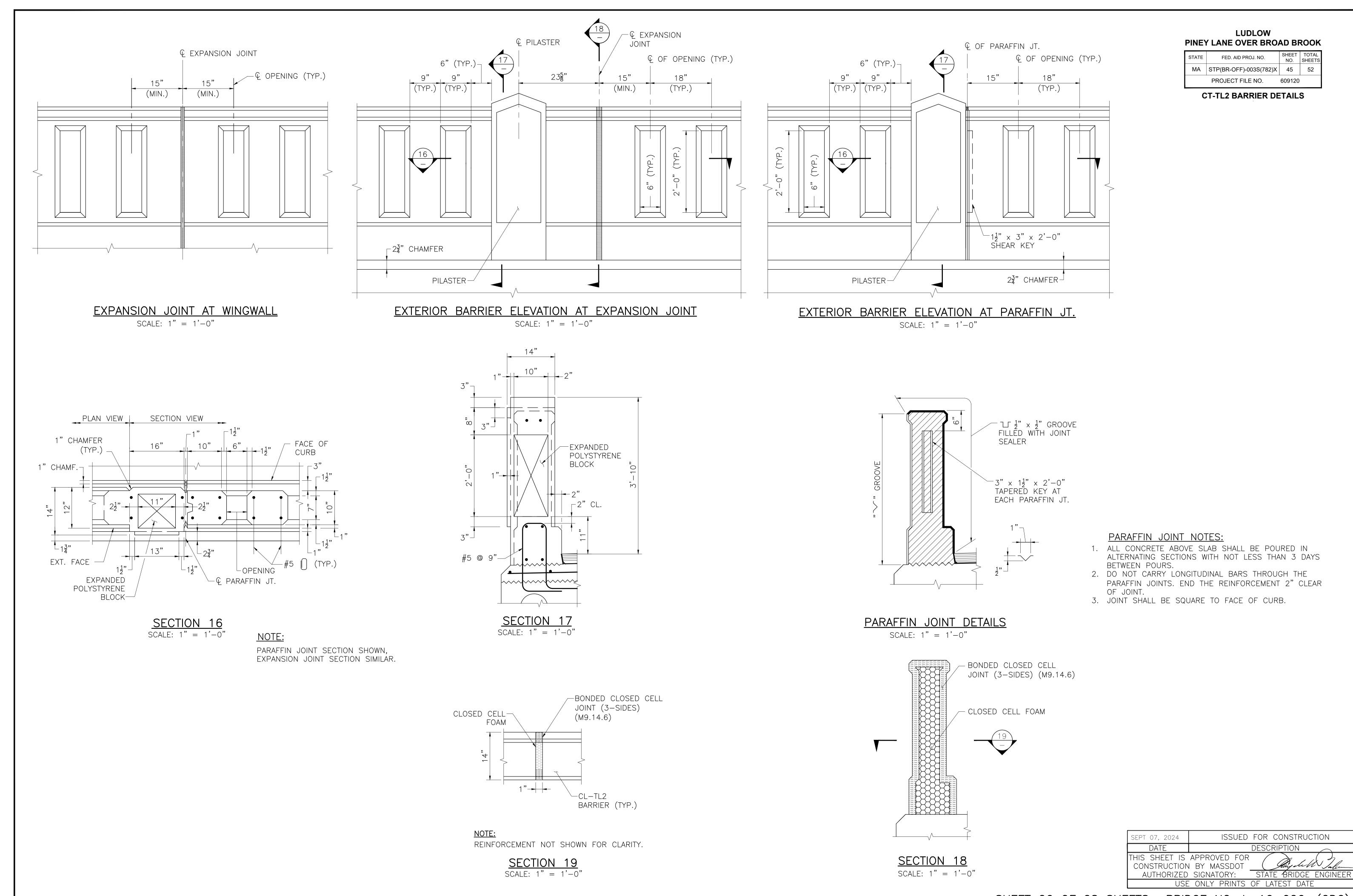
NOTES:

- 1. THIS TABLE INDICATES THE THEORETICAL THICKNESS OF THE DECK SLAB IN INCHES BASED UPON ASSUMED BEAM CAMBERS AT ERECTION.
- 2. TABLE IS PROVIDED TO ASSIST IN ESTIMATING THE REQUIRED CONCRETE VOLUME.
- 3. THE ACTUAL DECK THICKNESSES WILL BE AS REQUIRED

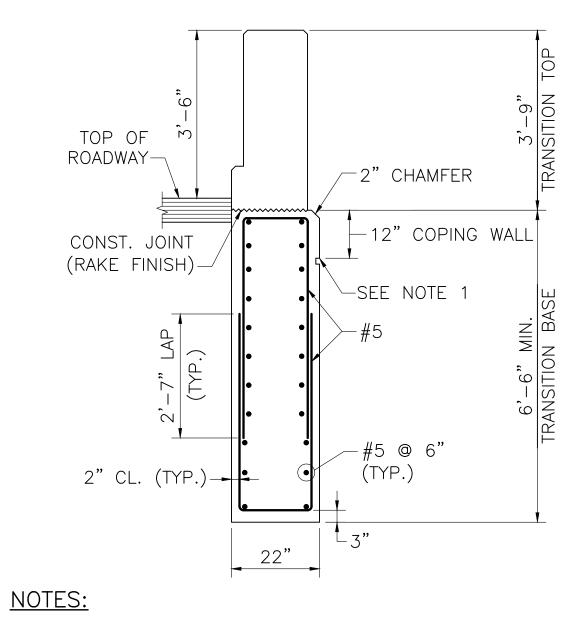
UTILITY SUPPORT NOTES:

- 1. UTILITY SUPPORT THREADED INSERTS HAVE BEEN PROVIDED FOR THE INSTALLATION OF FUTURE UTILITIES. STRUCTURE HAS BEEN DESIGNED TO ACCOMMODATE AN UNFACTORED 250 PLF UTILITY LOAD ALONG EACH FASCIA.
- 2. THE $\frac{3}{4}$ " Ø THREADED INSERTS FOR $\frac{3}{4}$ " Ø BOLTS SHALL BE CAST INTO THE PRECAST BEAMS BY THE FABRICATOR AND SHALL PROVIDE A MINIMUM NOMINAL TENSILE RESISTANCE OF 6.0 KIPS AND A MINIMUM NOMINAL SHEAR RESISTANCE OF 6.0 KIPS IN 3000 PSI CONCRETE.

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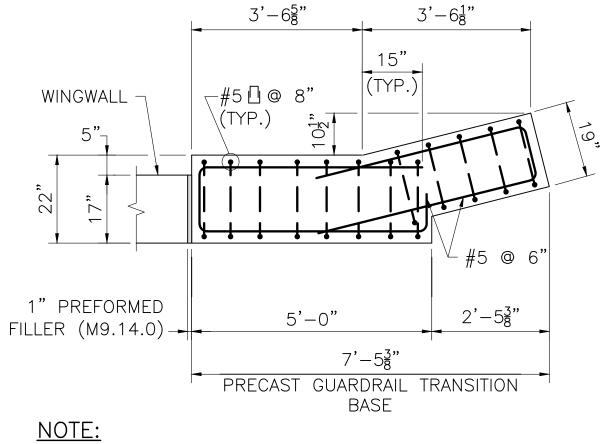
SHEET 26 OF 28 SHEETS BRIDGE NO. L-16-026 (CDG)





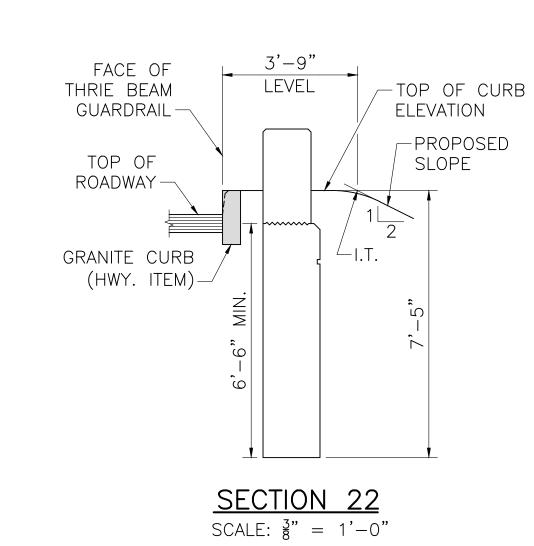
- 1. $1\frac{1}{2}$ " H x 1" D GROOVE. ALIGN WITH GROOVE AT TOP OF STRIATIONS.
- 2. REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY.

SECTION 20 SCALE: $\frac{1}{2}$ " = 1'-0"



WINGWALL REINFORCEMENT AND STRIATIONS NOT SHOWN FOR CLARITY.

SECTION 21 SCALE: $\frac{1}{2}$ " = 1'-0"



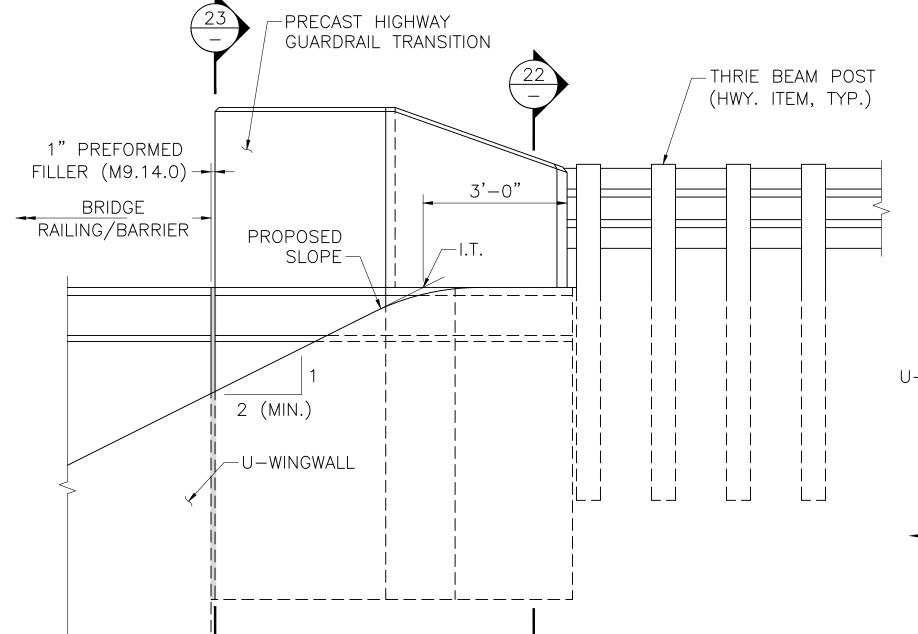
LUDLOW

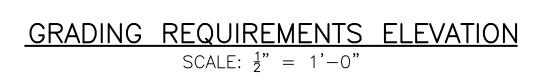
PINEY LANE OVER BROAD BROOK

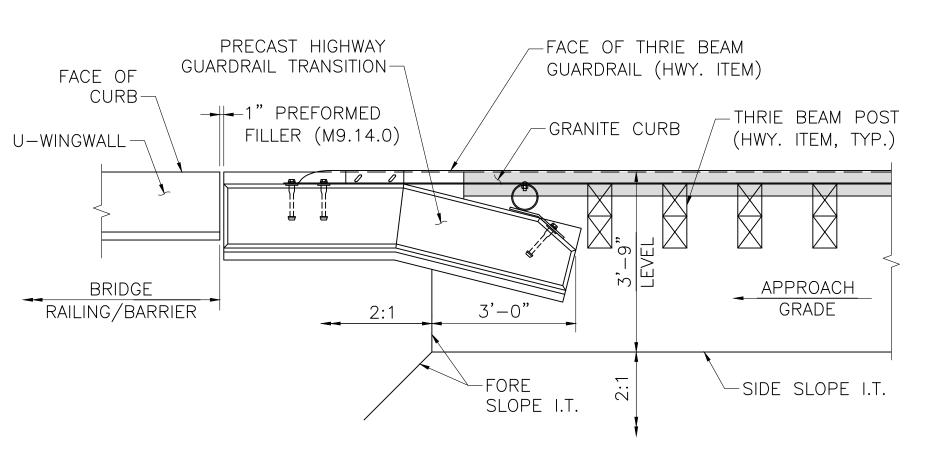
HIGHWAY GUARDRAIL TRANSITION DETAILS 1 OF 2

MA STP(BR-OFF)-003S(782)X 46

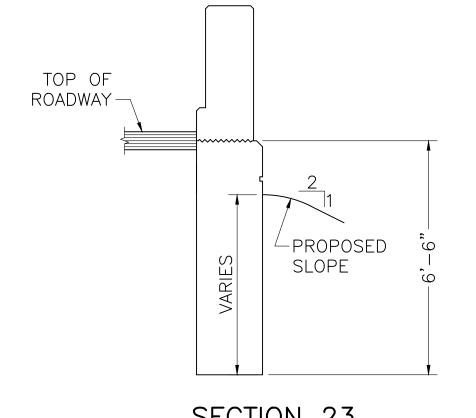
PROJECT FILE NO.







GRADING REQUIREMENTS PLAN SCALE: $\frac{1}{2}$ " = 1'-0"



SECTION 23 SCALE: $\frac{3}{8}$ " = 1'-0"

PRECAST CONCRETE TRANSITION NOTES:

- 1. PRECAST GUARDRAIL TRANSITION SHALL BE 5000 HP CONCRETE AND PAID FOR UNDER ITEM NO. 995.01 BRIDGE LUMP SUM.
- 2. GRAVEL BORROW SHALL BE PLACED AND THOROUGHLY COMPACTED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE PRECAST GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION. WHERE NO GRAVEL BORROW IS REQUIRED BELOW THE BASE, IT SHALL BE PLACED ON UNDISTURBED SOIL.
- 3. CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKILL PRECAST GUARDRAIL TRANSITION WITH CONTROLLED DENSITY FILL (NON-EXCAVATABLE) TO THE ELEVATION SHOWN.
- 4. SEE ADDITIONAL HIGHWAY TRANSITION BARRIER NOTES AND DETAILS ON SHEET 28 OF 28.

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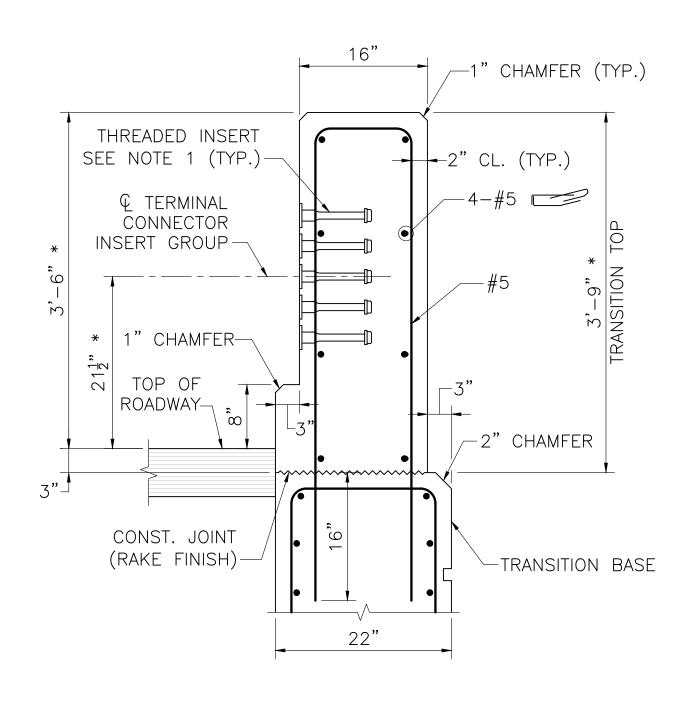
SHEET 27 OF 28 SHEETS BRIDGE NO. L-16-026 (CDG)

NOTES:

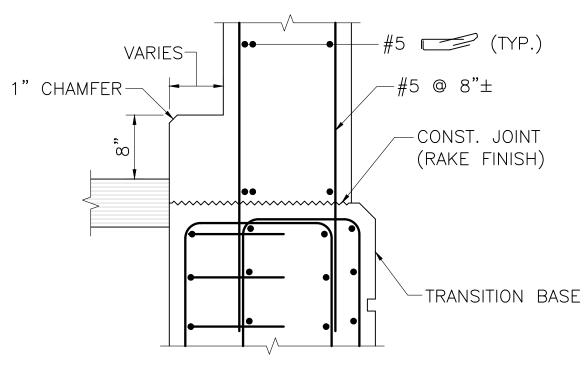
- 1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING A NOMINAL SHEAR RESISTANCE OF 20 KIPS PER 7 \$\infty\$ S.S. BOLT. S.S. BOLTS SHALL BE $\frac{7}{8}$ " $\phi \times 1\frac{1}{2}$ " LONG FULLY THREADED AISI TYPE 304N STAINLESS STEEL. INSERTS FOR $\frac{7}{8}$ " S.S. BOLTS SHALL BE GALVANIZED AND CAST INTO THE TRANSITION.
- 2. FOR AN APPROACH GRADE UP TO 3%, THE TRANSITION MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE POST.

FOR AN APPROACH GRADE IN EXCESS OF 3%, THE TRANSITION TOP AND THE TOP OF THE BRIDGE BARRIERS SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE TRANSITION TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE BOTTOM OF THE TRANSITION BASE SHALL BE SET LEVEL WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE.

- 3. USE LATEST CONTRACT COMPLETION YEAR IN EFFECT WHEN THE FIRST GUARDRAIL TRANSITION IS CAST. USE THIS YEAR FOR ALL GUARDRAIL TRANSITIONS.
- 4. ALL CONCRETE FOR THE PRECAST HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 HP CONCRETE.
- 5. LIFTING DEVICES (NOT SHOWN), INCLUDING THEIR NUMBER AND LOCATION, SHALL BE DESIGNED AND DETAILED BY THE PRECASTER. THEY SHALL BE GALVANIZED AND SHALL BE PLACED AND RECESSED IN POCKETS TO PROVIDE $1\frac{1}{2}$ " CLEAR COVER TO THE FACE OF THE TRANSITION CONCRETE. THESE DEVICES SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS ALONG WITH ALL SUPPORTING CALCULATIONS AND/OR CATALOG CUTS. ONCE THE PRECAST TRANSITION IS SET IN PLACE, THE LIFTING DEVICE POCKETS SHALL BE FILLED WITH A NON-SHRINK GROUT THAT MATCHES THE COLOR OF THE TRANSITION CONCRETE WHEN CURED AND THE FILLED POCKETS SHALL BE RUBBED WITH A CORUNDUM STONE TO BLEND OUT THE JOINTS.
- 6. THE DATE IN THE BARRIER SHALL BE CAST LEVEL AND APPROXIMATELY 5 INCHES (MINIMUM) FROM THE TOP OF THE BARRIER.



SECTION 25 AT SAFETY CURB SCALE: 1" = 1'-0"



SECTION 26 AT SAFETY CURB

SCALE: 1" = 1'-0"

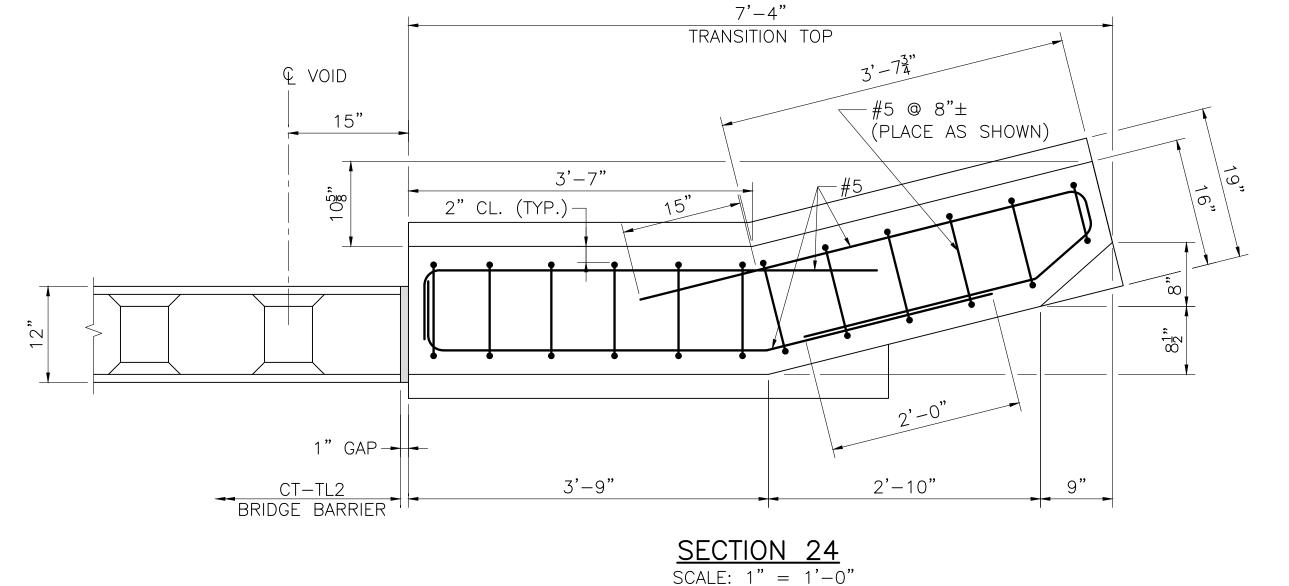
7'-4"

211,

THREADED INSERTS EACH

WITH $\frac{7}{8}$ Ø H.S. BOLT

* SEE NOTE 2



7'-4"

TRANSITION TOP

Q THREADED INSERTS

PLAN AT SAFETY CURB

SCALE: 1" = 1'-0"

7'-4"

TRANSITION TOP

ELEVATION AT SAFETY CURB

SCALE: 1" = 1'-0"

EACH WITH $\frac{7}{8}$ ø S.S.

BOLT (SEE NOTE 1)

5'-0"

3'-9"

YEAR CENTERED ON

FACE (SEE NOTE 3)

20XX 50

└ 5" MIN., SEE NOTE 6

---THREADED INSERT WITH

7" ø S.S. BOLT (SEE

NOTE 1, TYP.)

SEE

NOTE 2 -

2'-4"

Q THREADED INSERT WITH $\frac{7}{8}$ " ϕ H.S. BOLT -

2'-4"

3**'**-7"

- BEND TOP #5

TO MATCH TAPER

/ 1" CHAMFER

— € TERMINAL CONNECTOR

INSERT GROUP

TOP OF SAFETY CURB —

3'-6⁵"

CURB BELOW

1" CHAMFER-

1" CHAMFER-

CT-TL2
BRIDGE BARRIER

Q VOID

15"

1" GAP -

CT-TL2 BRIDGE BARRIER

2" CHAMFER-

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TOP OF PRECAST HIGHWAY GUARDRAIL TRANSITION FOR CT-TL2 BARRIER

BRIDGE NO. L-16-026 (CDG) SHEET 28 OF 28 SHEETS

PERMANENT

