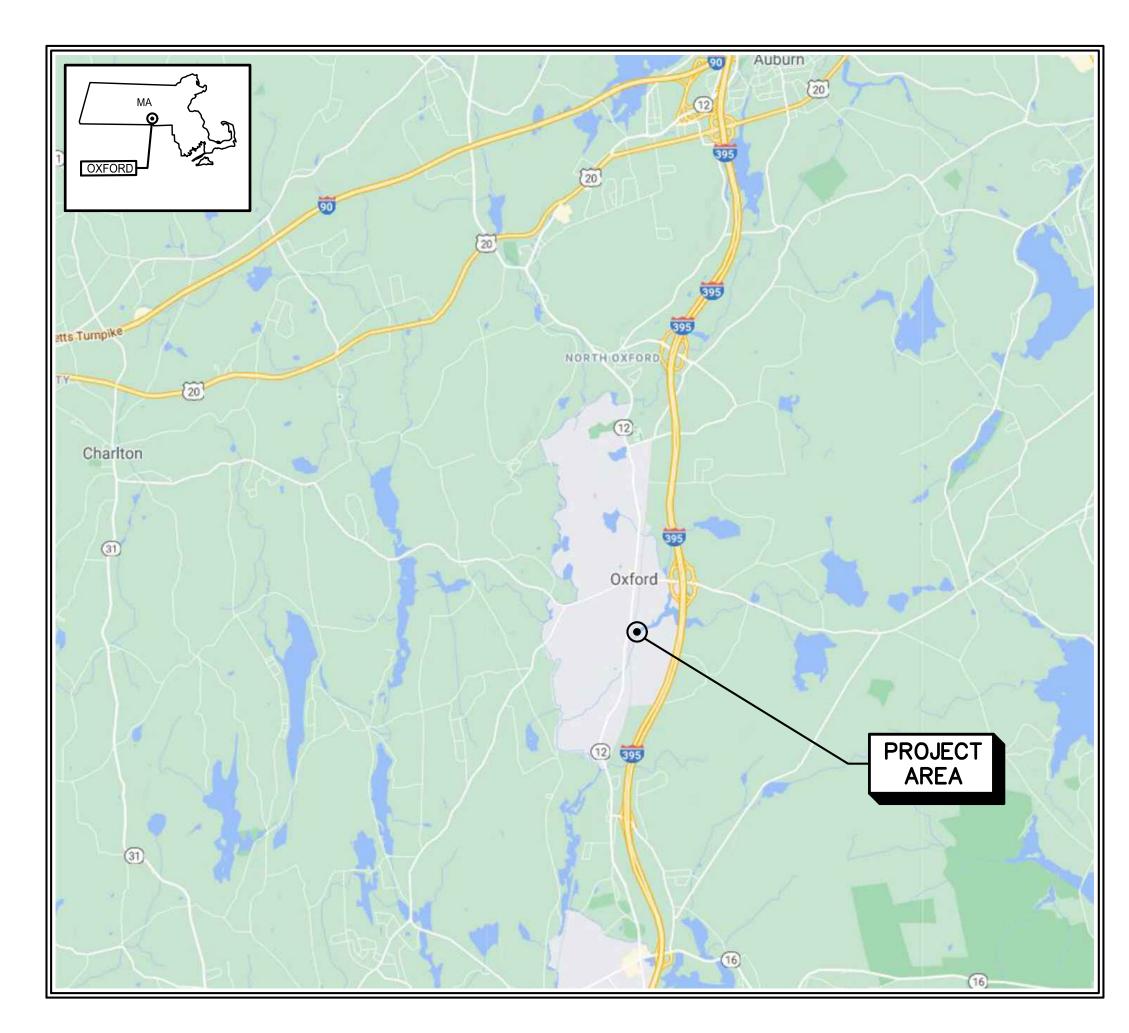
MASSACHUSETTS DEPARTMENT OF CONSERVATION AND RECREATION

LOWES POND DAM REHABILITATION OXFORD, MA

DCR PROJECT NO. FOR CONSTRUCTION P24-3541-C1A

MARCH 2024



LOCATION PLAN APPROXIMATE SCALE: 1" = 1 Mile

0 1 2 SCALE: 1" = 1 MILE

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CIVIL

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 10 C-101 EXISTING CONDITIONS AND EROSION PROTECTION PLAN
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AECOM

PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620
Boston, MA 02116
617.626.1250 tel 617.626.1351 fax
www.mass.gov/orgs/department-of-conservation-recreation

CONSULTANT

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REGISTRATION

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ISSUE/REVISION

I/R	DATE	DESCRIPTION

PROJECT NUMBER

60604936

Designed By:	J.P. MINOIS
Drawn By:	M. THIBODEAU
Dept Check:	C. BENZIGER
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

DISCIPLINE

GENERAL SHEET TITLE

LOWES POND

COVER SHEET, LOC. PLAN AND INDEX OF DRAWINGS SHEET NUMBER

00 G-001

GENERAL PLAN NOTES

- 1. TOPOGRAPHIC SURVEY IS BASED ON AN ON-THE-GROUND SURVEY PERFORMED ON JUNE 4TH AND 5TH, 2019, BY ALPHA SURVEY GROUP, LLC.
- 2. THE HORIZONTAL DATUM FOR THIS PROJECT IS THE MASSACHUSETTS STATE PLANE COORDINATE SYSTEM REFERENCED TO THE NORTH AMERICAN DATUM OF 1983 (NAD83), CORS ADJUSTMENT (NA2011/GEOID 12a) AS DETERMINED BY REDUNDANT GPS OBSERVATIONS MADE ON MAY 20TH, 2019 UTILIZING KEYSTONE PRECISION INSTRUMENTS' KEYNET GPS VIRTUAL REFERENCE SYSTEM (VRS) NETWORK.
- 3. THE VERTICAL DATUM FOR THIS PROJECT IS REFERENCED TO THE RECOVERED SITE BENCHMARKS (AS SHOWN ON PLAN ENTITLED "PLAN OF LAND IN, OXFORD, MA, PREPARED FOR, FUSS & O'NEILL" PREPARED BY SHERMAN & FRYDRYCK, INC., DATED 7/22/2014) PROVIDED BY THE CLIENT.
- 4. RESOURCE AREA DELINEATION WAS PERFORMED BY AECOM ON MARCH 18, 2021.
- 5. NO UTILITY INVESTIGATION WAS PERFORMED FOR THIS PROJECT. UTILITY LOCATION SHALL BE PERFORMED IN THE FIELD PRIOR TO THE COMMENCEMENT OF ANY WORK. CALL "DIG SAFE" AT 811.
- 6. THE CONTRACTOR SHALL RESTORE ALL DISTURBED AREAS TO APPROXIMATE EXISTING GRADES UPON COMPLETION OF THE CONSTRUCTION, EXCEPT WHERE PERMANENT GRADE CHANGES ARE SPECIFICALLY NOTED OR ORDERED.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF THE EXISTING FEATURES AND STRUCTURES WITHIN AND ADJACENT TO THE WORK. IN THE EVENT OF DAMAGE, THE REPAIRS OR REPLACEMENT SHALL BE COMPLETED AT THE CONTRACTOR'S EXPENSE AS APPROVED BY THE ENGINEER.
- 8. ALL PIPES OR OTHER UTILITIES DAMAGED DURING THE CONTRACTOR'S OPERATIONS SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR OR REPLACE AT NO COST TO THE OWNER.
- 9. EXISTING CONDITIONS ARE SHOWN ON THE PLANS IN A SCREENED (LIGHTER) PENWEIGHT. PROPOSED WORK IS SHOWN IN BOLDER PENWEIGHT.
- 10. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING ALL WORK AS INDICATED ON THE DRAWINGS, IN THE SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER IN CONFORMANCE WITH ALL APPLICABLE CODES AND IN A PROPER AND WORKMANLIKE MANNER.
- 11. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL WASTE BUILDING MATERIAL, CONCRETE, MASONRY, TREES, SHRUBS, DEBRIS AND OTHER MATERIALS NECESSARY FOR THE SATISFACTORY COMPLETION OF THE WORK AND AS REQUIRED BY THE OWNER. CONSTRUCTION DEBRIS SHALL BE DISPOSED OF IN STRICT ACCORDANCE WITH ALL APPLICABLE LOCAL. STATE AND FEDERAL LAWS.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY LOCAL STREET OPENING OR BUILDING PERMITS AND FOR COORDINATING INSPECTIONS AS REQUIRED. PERMIT FEES SHALL BE PAID DIRECTLY BY THE CONTRACTOR AND SHALL BE INCLUDED IN THE APPROPRIATE ITEM OF THE BID. THE CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH THE PERMITS OBTAINED BY THE OWNER AND REFERENCED IN SPECIFICATION 0110.
- 13. MAINTAIN GRAVEL ROADS AT ALL TIMES DURING CONSTRUCTION. GRAVEL ROADS SHALL BE GRADED AND SHALL BE IN GOOD TRAFFICABLE CONDITIONS WITHOUT RUTS OR WASHOUTS AT THE COMPLETION OF THE PROJECT.
- 14. TREE CLEARING MAY OCCUR UP TO THE LIMIT OF WORK LINE TO FACILITATE EQUIPMENT STAGING AND VEHICULAR MOVEMENT.

DEMOLITION NOTES

- 1. TAKE MEASURES TO PREVENT DAMAGE TO EXISTING STRUCTURES TO REMAIN DURING DEMOLITION. DAMAGE THAT DOES OCCUR SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT NO ADDITIONAL COST TO THE OWNER.
- 2. SHORE EXISTING STRUCTURES TO UNLOAD MEMBERS BEFORE REMOVING STRUCTURAL ELEMENTS.
- 3. PATCH AND FINISH EXPOSED SURFACES TO MATCH THE ADJACENT AREA UNLESS OTHERWISE INDICATED OR SPECIFIED.

TRAFFIC MANAGEMENT

- 1. CONTRACTOR SHALL COORDINATE A TRAFFIC MANAGEMENT PLAN WITH LOCAL AUTHORITIES PRIOR TO THE START OF THE PROJECT PER SPECIFICATION 01063.
- 2. CONTRACTOR TO PROVIDE ALL REQUIRED SIGNAGE, FLAGGING, AND TRAFFIC CONTROL DEVICES AS PART OF THE TRAFFIC MANAGEMENT PLAN.
- 3. ACCESS FOR LOCAL RESIDENTS MUST BE MAINTAINED THROUGHOUT THE PROJECT.
- 4. MAINTAIN ONE LANE PASSABLE AT ALL TIMES.

<u>LEGEND</u>

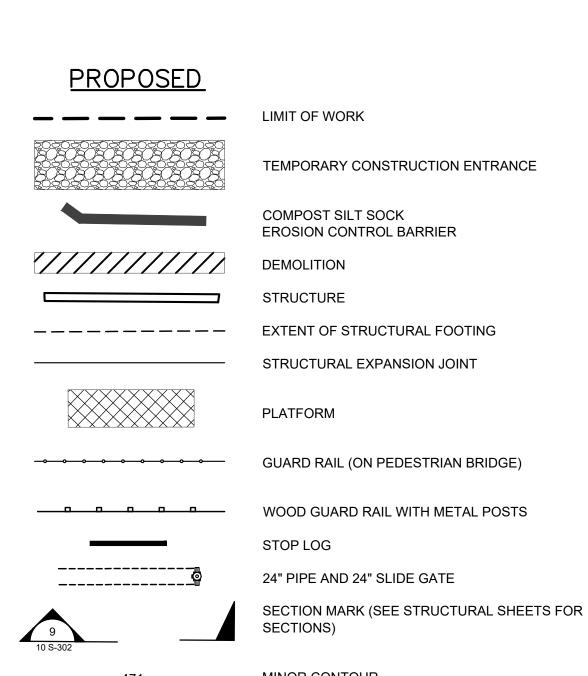
EXISTING ----- 471 -----MINOR CONTOUR _ _ _ _ _ 470 — _ _ _ MAJOR CONTOUR EDGE OF PAVEMENT EDGE OF GRAVEL __ _ _ _ _ _ _ _ _ EDGE OF WATER ORDINARY HIGH WATER _ · · _ · · _ · · _ · · _ · · _ · · _ · · _ 100-FOOT BUFFER ZONE 200-FOOT RIVERFRONT AREA WOOD GUARD RAIL WITH METAL POSTS TREE LINE OVERHEAD WIRE ____OHW____ 100' RESOURCE BUFFER -----WETLAND FLAG - BANK LIMIT RIGHT OF WAY ______ **DIRECTIONAL FLOW ARROW BENCHMARK** SIGN ___ SPOT GRADE X 474.8 RESOURCE AREA FLAG BOULDER **BORING** WATER GATE FIRE HYDRANT UTILITY POLE DRAIN MANHOLE CATCH BASIN TREE (SIZE IN INCHES) TRAVERSE (CONTROL) POINT CONCRETE

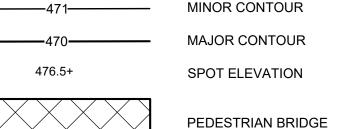
RIPRAP

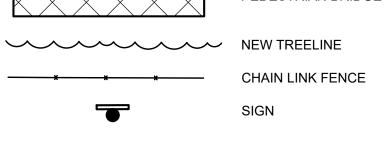
LEDGE

ABBREVIATIONS

BIT.	BITUMINOUS
CONC.	CONCRETE
DH	DRILL HOLE
E.J.	EXPANSION JOINT
НВ	HARD BOTTOM
MAG	MAG NAIL
RET.	RETAINING
R.O.W.	RIGHT-OF-WAY
RR	RAILROAD
S&M	STONE AND MORTAR
SPK	SPIKE
VGC	VERTICAL GRANITE CURB
W.S.	WATER STOP

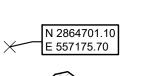








PARKING SPACE WITH ACCESSIBLE AISLE

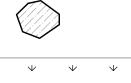


GRAVEL PATH CENTERLINE

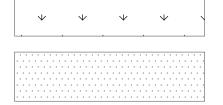
SEED MIX

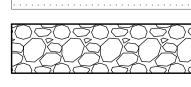
RIPRAP AREA

COORDINATE LOCATION



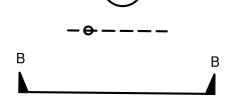
EXISTING BOULDER RELOCATION



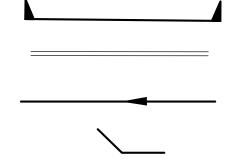


HYDRODYNAMIC SEPARATOR, DRAIN PIPE AND RIPRAP AT OUTLET, WITH FLOW DIRECTION

GRAVEL PATHWAY AND PARKING AREA

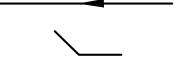


SECTION MARK AT BIOINFILTRATION BASINS



VERTICAL GRANITE CURB

UNDERDRAIN PIPE WITH CLEANOUT



DRAIN PIPE WITH FLOW DIRECTION

DRAIN PIPE BEND

LIMIT OF EMBANKMENT (IMPERVIOUS FILL AREA)

PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620 Boston, MA 02116 617.626.1250 tel 617.626.1351 fax www.mass.gov/orgs/department-of-conservation-recreation

CONSULTANT

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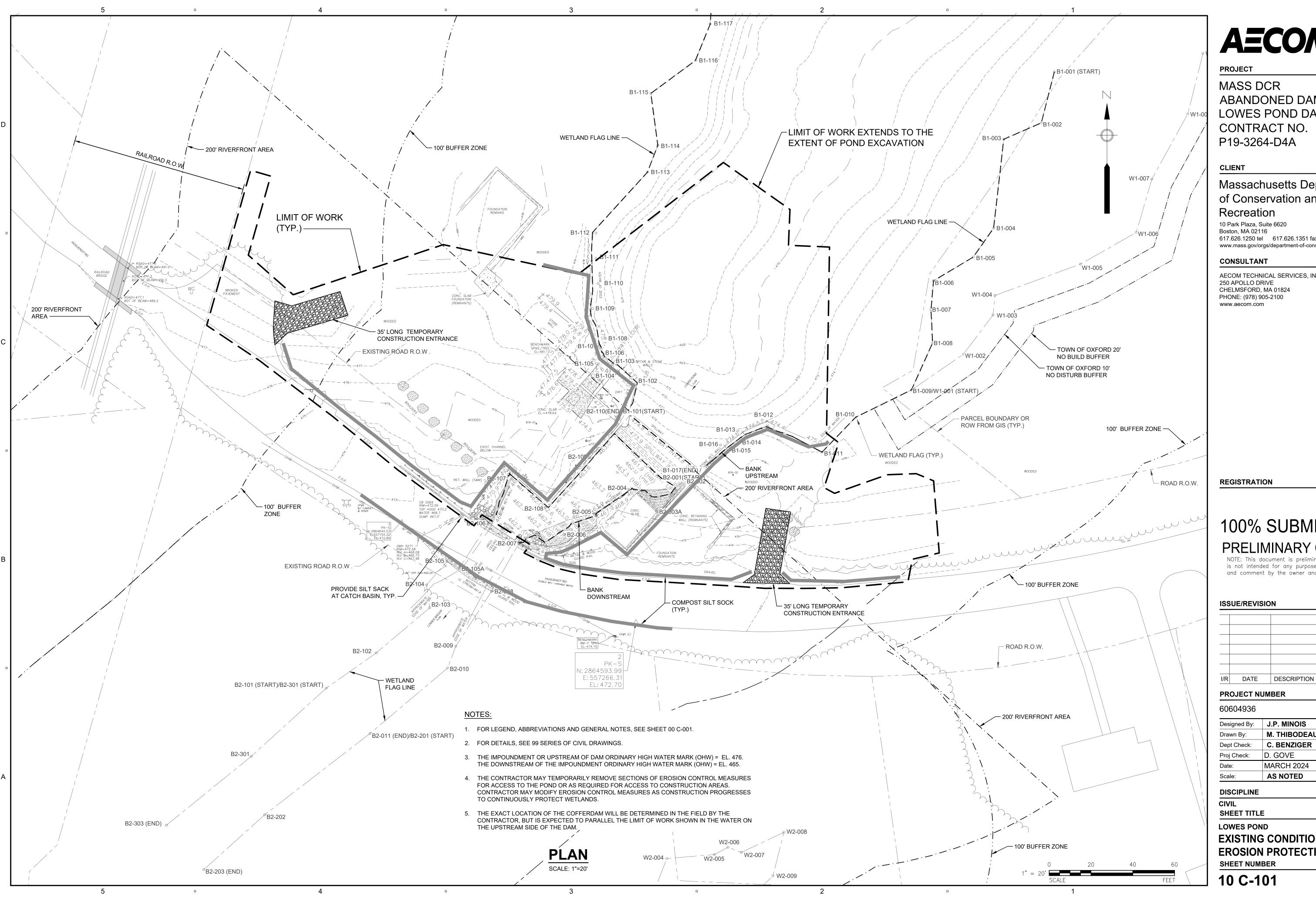
J.P. MINOIS Designed By: M. THIBODEAU C. BENZIGER Dept Check: D. GOVE Proj Check: MARCH 2024 **AS NOTED** Scale:

DISCIPLINE

CIVIL SHEET TITLE

LOWES POND

LEGEND, ABBREVIATIONS **AND GENERAL NOTES** SHEET NUMBER



MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO.

Massachusetts Department of Conservation and

617.626.1250 tel 617.626.1351 fax www.mass.gov/orgs/department-of-conservation-recreation

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EXISTING CONDITIONS AND EROSION PROTECTION PLAN

AREA (ACRES) 20 12 28 24 RESERVOIR AREA (ACRES) 474 IMPOUNDED WATER (ACRE-FEET) — 466 464 200 100 120 160 180 140 IMPOUNDED WATER NOTE: (ACRE-FEET) THE AREA - CAPACITY CURVES ARE BASED ON THE BATHYMETRIC SURVEY OF JULY 2019 PRESENTED IN THE PHASE II REPORT (APRIL 2020).

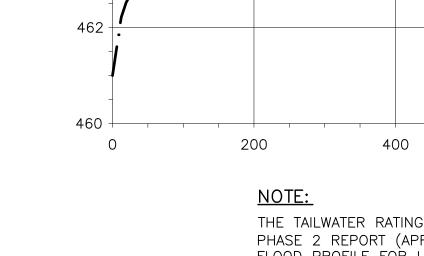
LOWES POND AREA - CAPACITY CURVES

50 - EXCEEDED 10% OF THE TIME 40 5 30 ⊒ EXCEEDED 50% OF THE TIME -EXCEEDED 90% OF THE TIME FEB DEC NOV

AVERAGE DAILY FLOWS

THE AVERAGE DAILY FLOW STATISTICS WERE ESTIMATED BASED ON THE FLOW RECORDS AT THE USGS GAGING STATION NO. 01124500 ON LITTLE RIVER NEAR OXFORD AND ADJUSTED FOR DRAINAGE AREA.

479 EXISTING SPILLWAY PROPOSED SPILLWAY WITH STOP LOGS IN PLACE PROPOSED SPILLWAY WITH STOP LOGS REMOVED 200 400 600 800 1000 1200 1400 1600 1800 2000 DISCHARGE (CFS)



TAILWATER RATING CURVE -600 800 1000 1200 DISCHARGE (CFS)

THE TAILWATER RATING CURVE WAS DEVELOPED FOR THE PURPOSE OF PHASE 2 REPORT (APRIL 2020) AND IS DERIVED FROM THE FEMA FLOOD PROFILE FOR LOWES BROOK PRESENTED IN FEMA FLOOD INSURANCE STUDY FOR WORCESTER COUNTY, MA DATED JULY 16, 2014.

AECOM

PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

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FLOOD DISCHARGE PROBABILITIES

2-YEAR FLOOD

10-YEAR FLOOD

50-YEAR FLOOD

100-YEAR FLOOD

NOTE:

PEAK

DISCHARGE (CFS)

107

374

753

THE FLOOD DISCHARGE PROBABILITIES WERE DEVELOPED

DURING THE HYDROLOGY AND HYDRAULICS STUDY COMPLETED FOR THE PHASE II REPORT (APRIL 2020).

RUNOFF

VOLUME

(AC-FT)

490

1307



PROJECT NUMBER

60604936

Designed By:	J.P. MINOIS
Drawn By:	M. THIBODEAU
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Proj Check:	D. GOVE
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DISCIPLINE CIVIL

SHEET TITLE

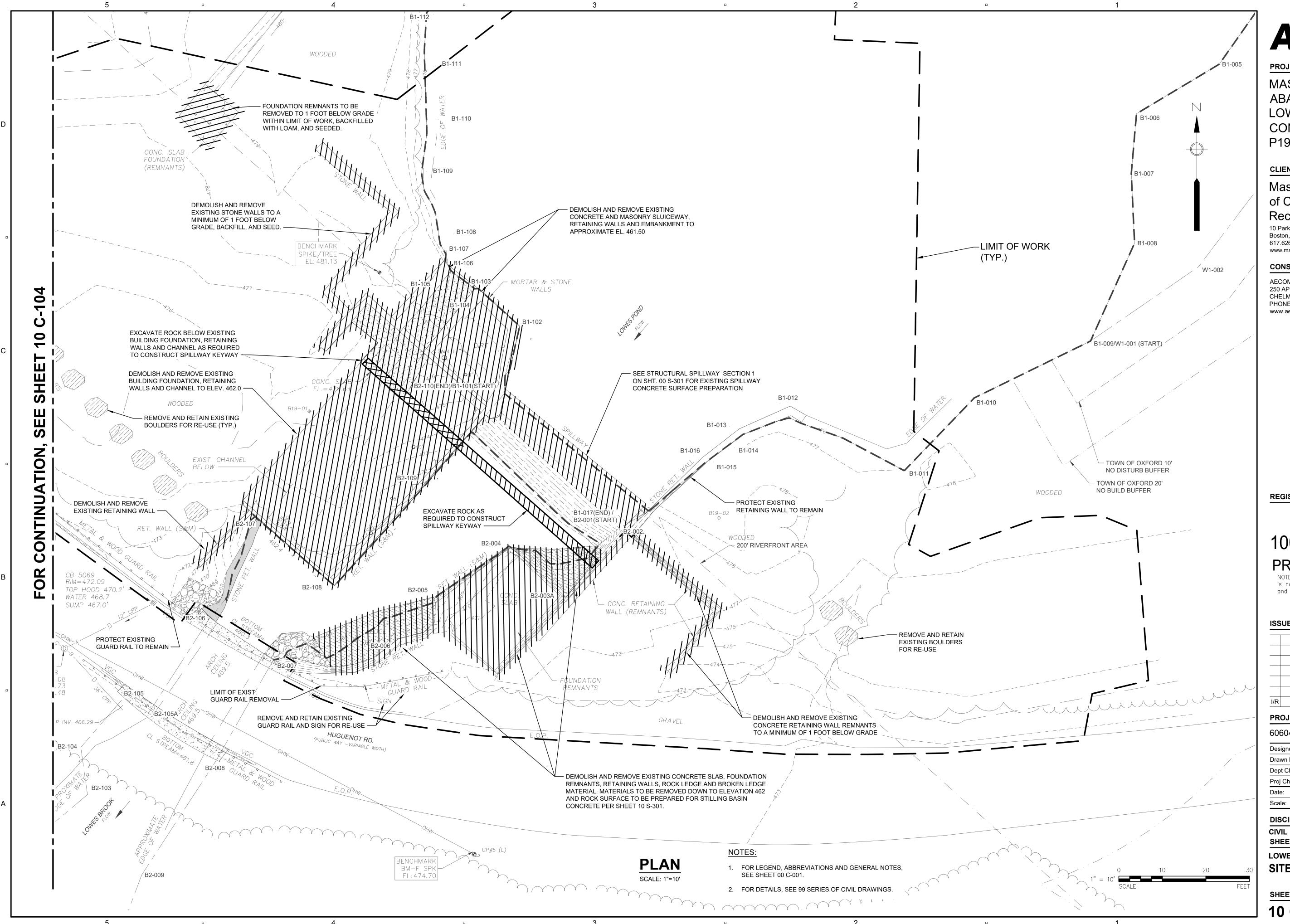
LOWES POND

PROJECT HYDROLOGY AND **HYDRAULIC DATA** SHEET NUMBER

10 C-102

NOTE: THE SPILLWAY DISCHARGE RATING CURVES WERE DEVELOPED FOR THE PURPOSE OF THE LOWES POND DAM REHABILITATION DESIGN, AND ARE BASED ON A SPILLWAY CREST ELEVATION AT 474.8 FT—NAVD88, AND A STOP LOG INVERT ELEVATION AT 471.8 FT—NAVD88. SPILLWAY DISCHARGE RATING CURVES

TAILWATER RATING CURVE UPSTREAM OF HUGUENOT ROAD



MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

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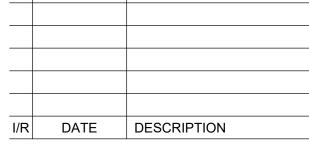
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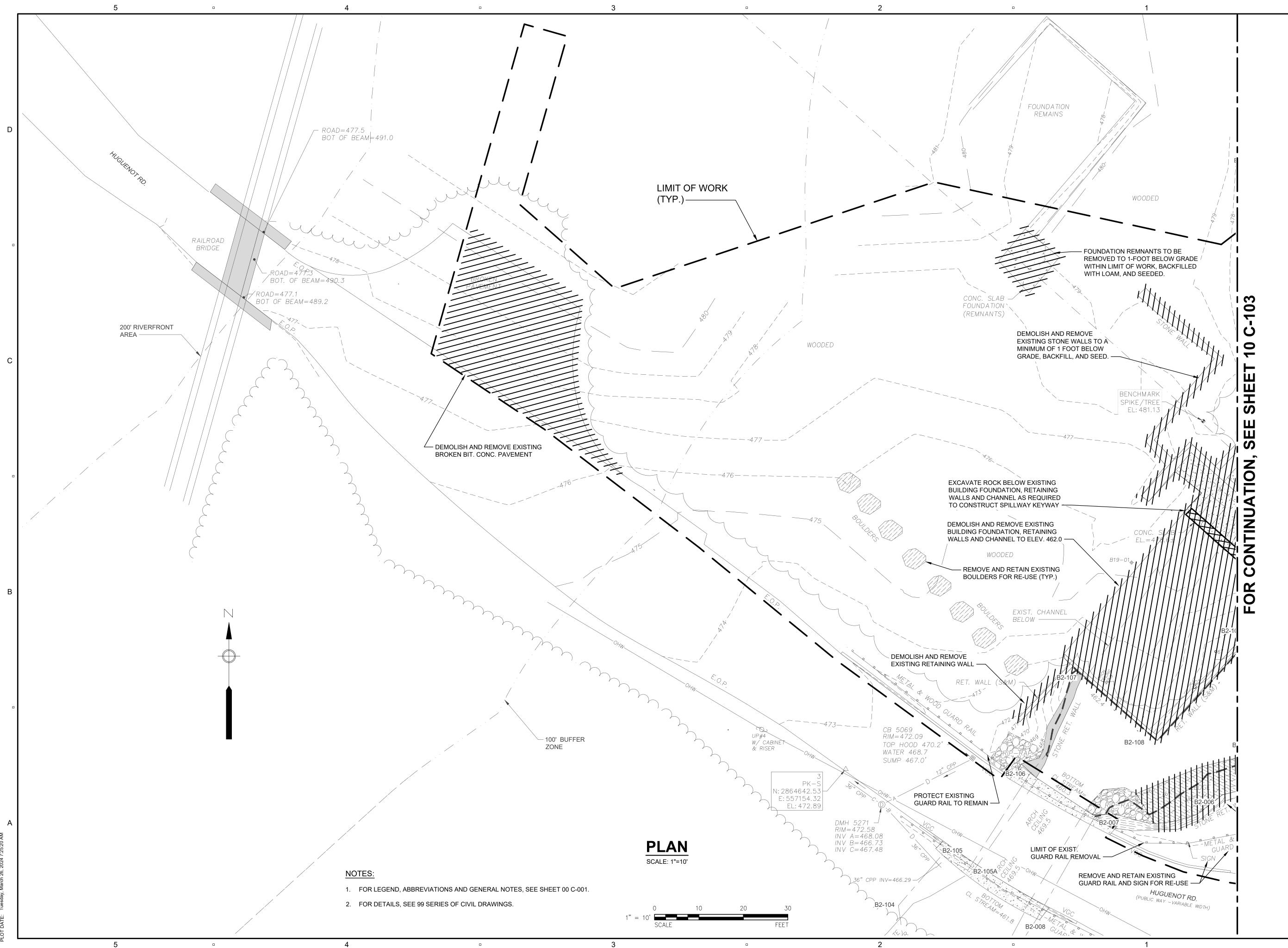
DISCIPLINE

SHEET TITLE

LOWES POND

SITE DEMOLITION PLAN I

SHEET NUMBER



PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

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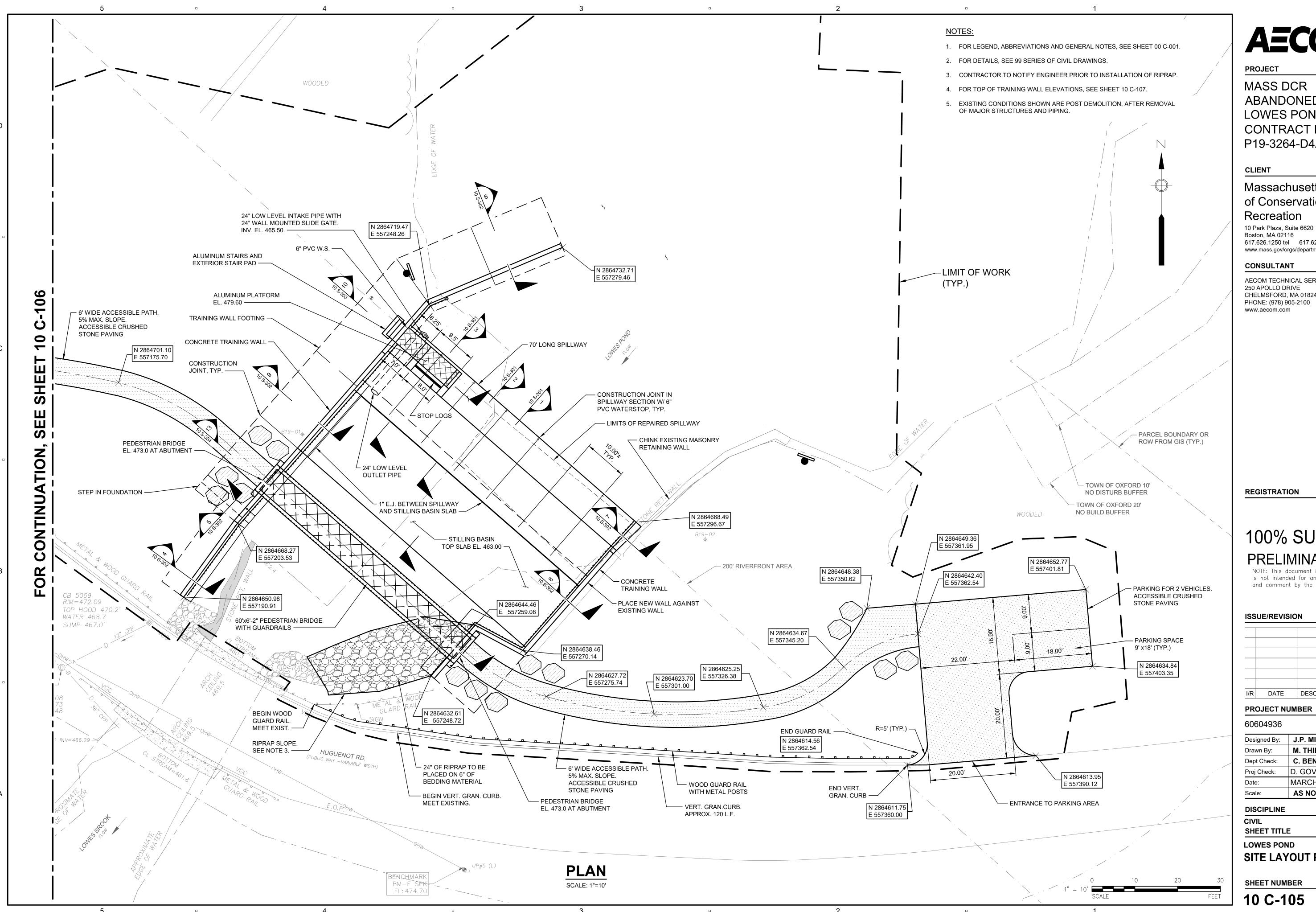
CIVIL SHEET TIT

SHEET TITLE

LOWES POND

SITE DEMOLITION PLAN II

SHEET NUMBER



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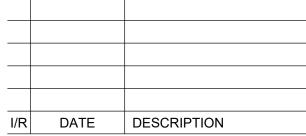
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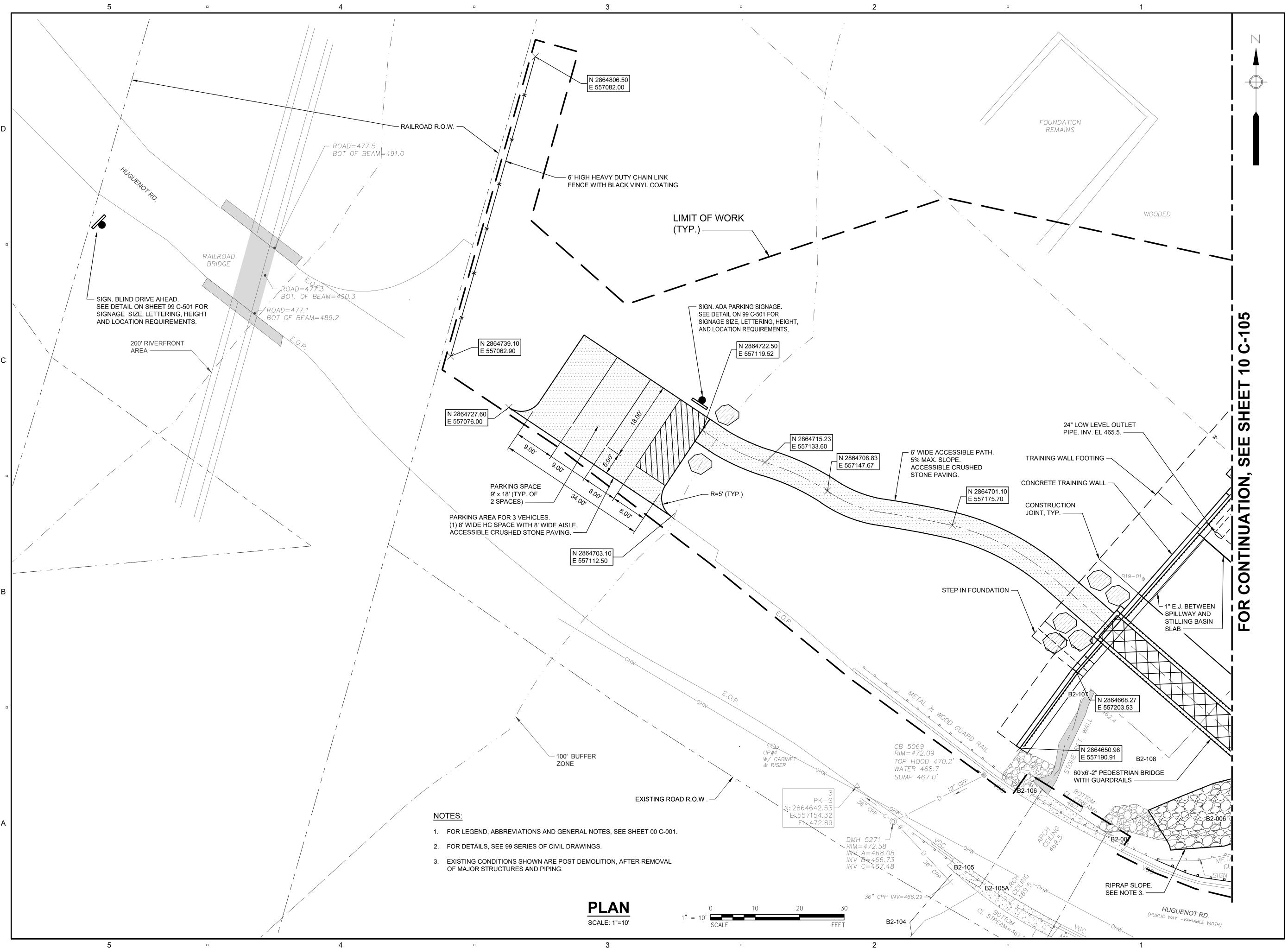
DISCIPLINE

SHEET TITLE

LOWES POND

SITE LAYOUT PLAN I

SHEET NUMBER



PROJECT

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ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
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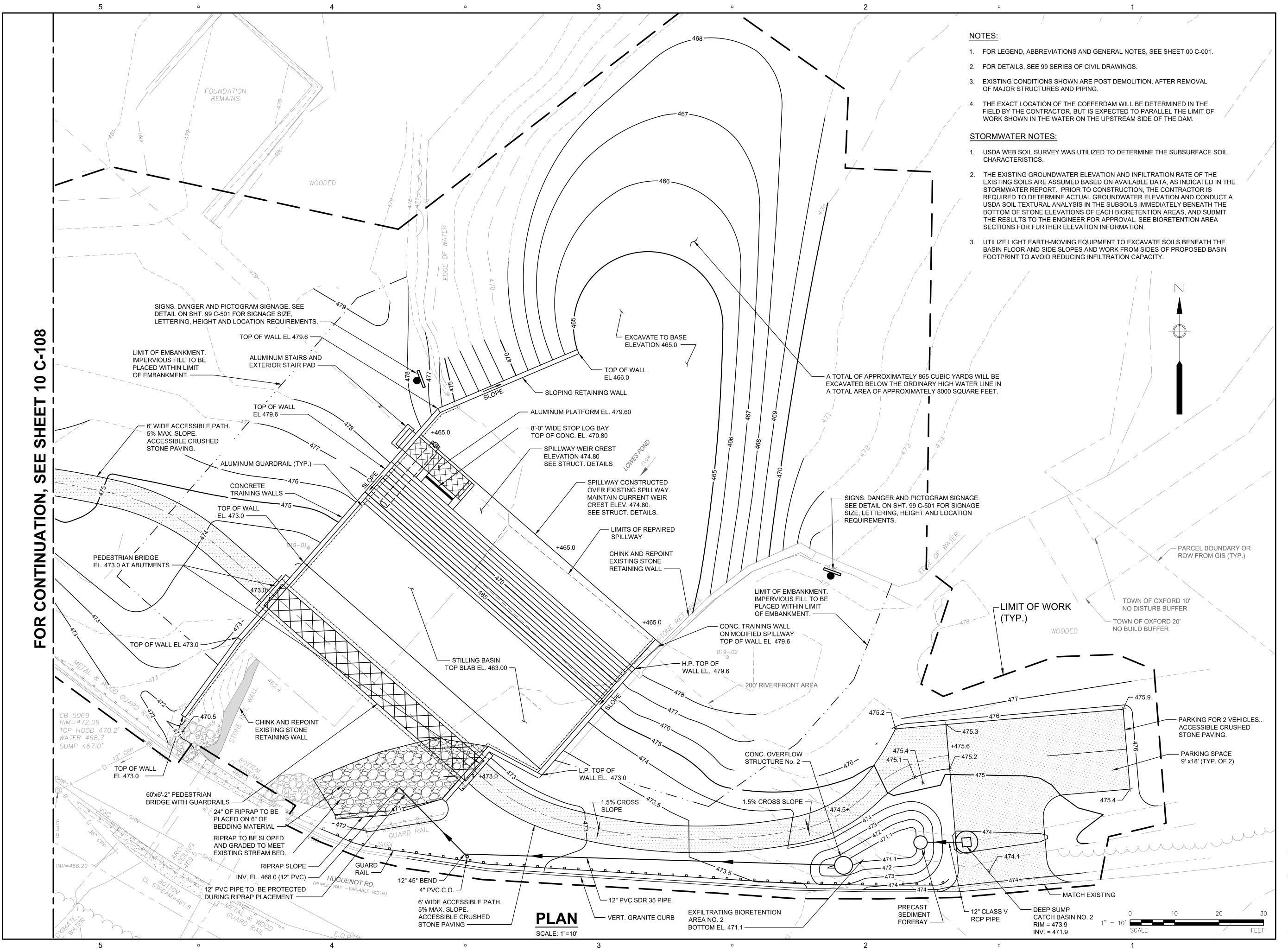
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LOWES POND

SITE LAYOUT PLAN II

SHEET NUMBER



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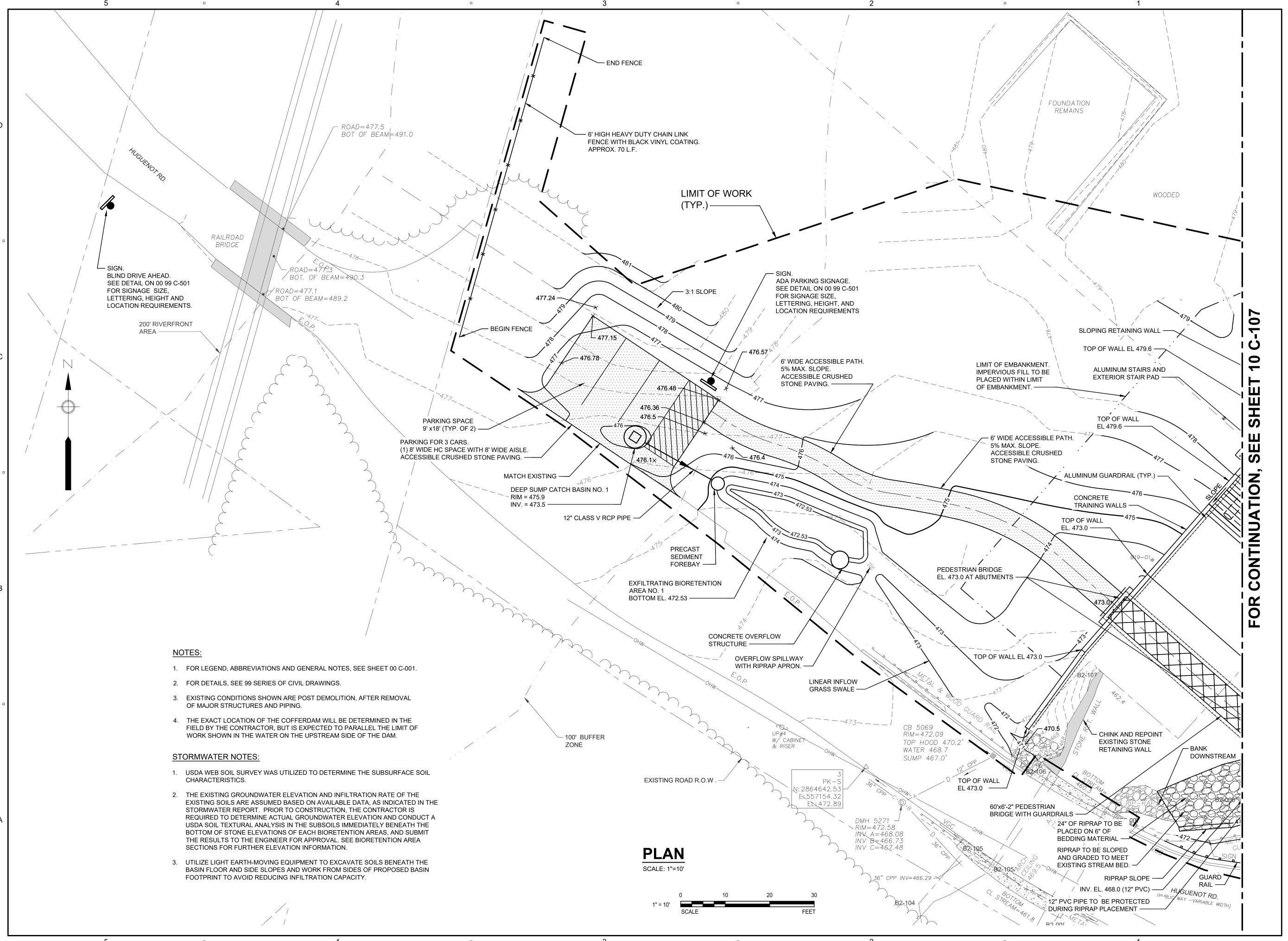
CIVIL

SHEET TITLE

LOWES POND

SITE GRADING PLAN I

SHEET NUMBER



PROJECT

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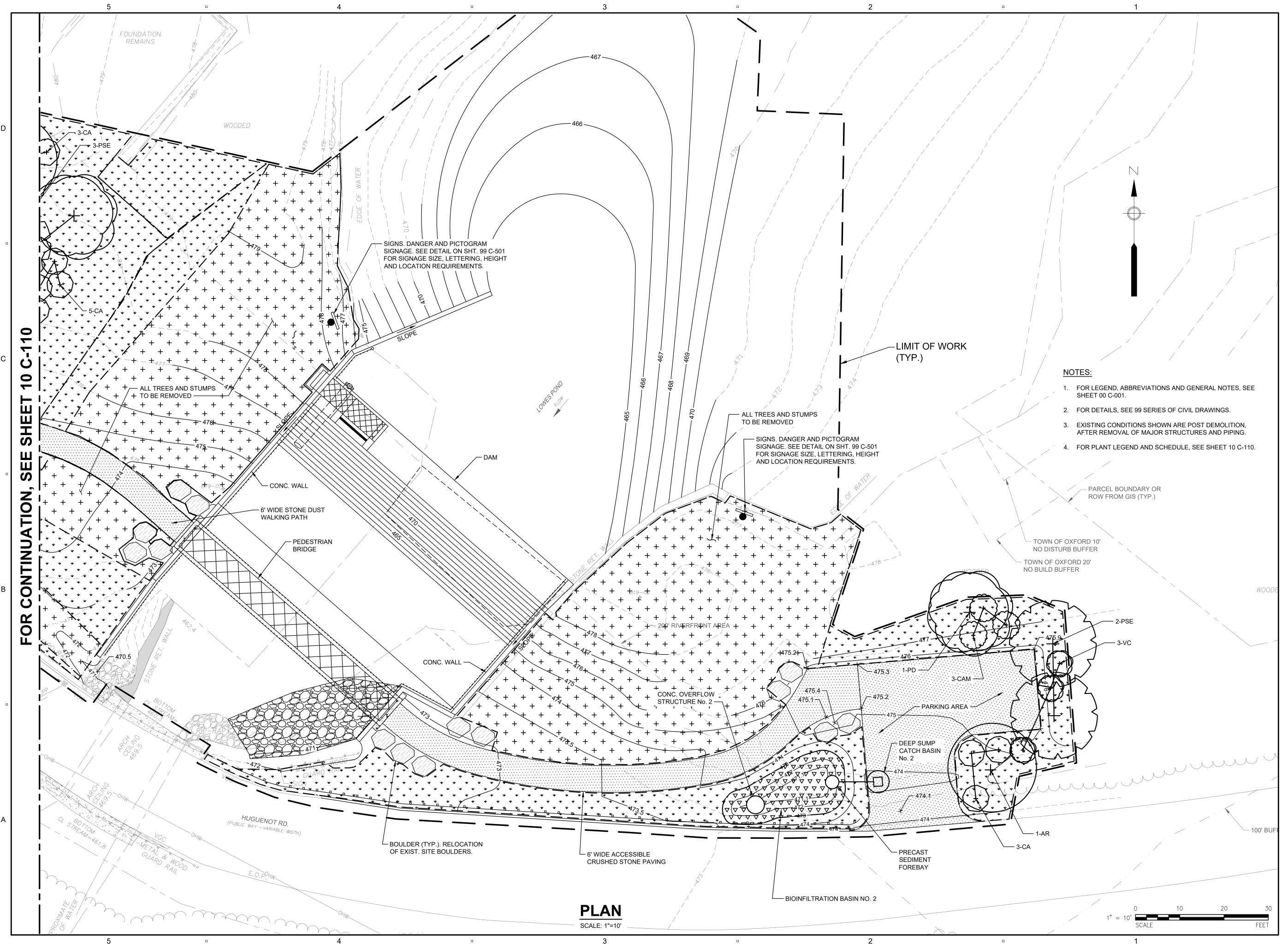
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SITE GRADING PLAN II

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Designed By:	L. DECKER
Drawn By:	M. THIBODEAU
Dept Check:	C. BENZIGER
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

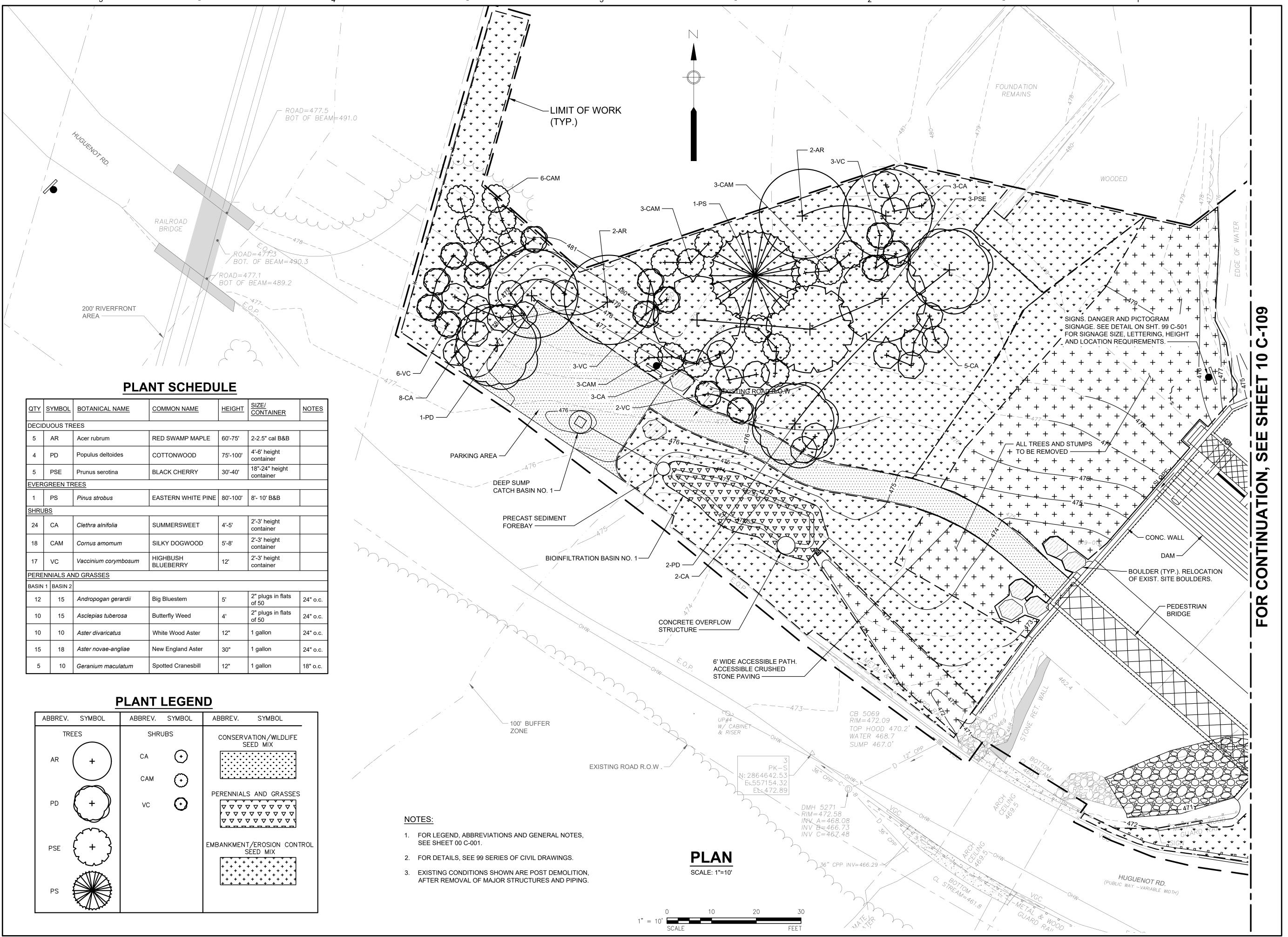
DISCIPLINE

CIVIL SHEET TITLE

LOWES POND

LANDSCAPE PLAN I

SHEET NUMBER



PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620
Boston, MA 02116
617.626.1250 tel 617.626.1351 fax
www.mass.gov/orgs/department-of-conservation-recreation

CONSULTANT

AECOM TECHNICAL SERVICES, INC. 250 APOLLO DRIVE CHELMSFORD, MA 01824 PHONE: (978) 905-2100 www.aecom.com

REGISTRATION

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								_
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I/R	DATE	DESCRIPTION

PROJECT NUMBER

60604936

Designed By:	L. DECKER
Drawn By:	M. THIBODEAU
Dept Check:	C. BENZIGER
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DISCIPLINE

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01450 0015

LANDSCAPE PLAN II

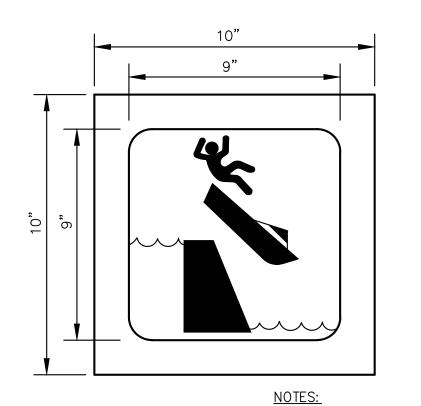
LANDOUALLILA

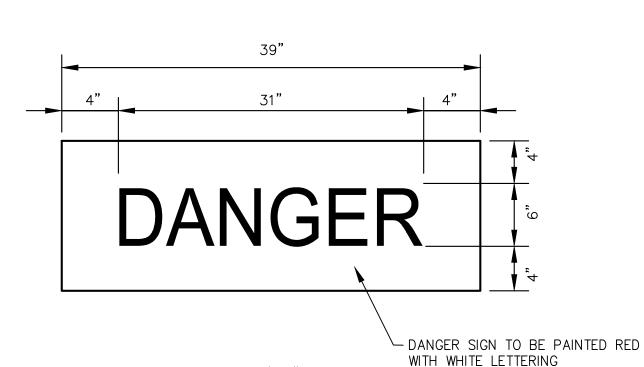
SHEET NUMBER

NOTE: DETAIL APPLIES TO THE RELOCATION OF EXISTING SITE BOULDERS.

BOULDER SETTING

NOT TO SCALE



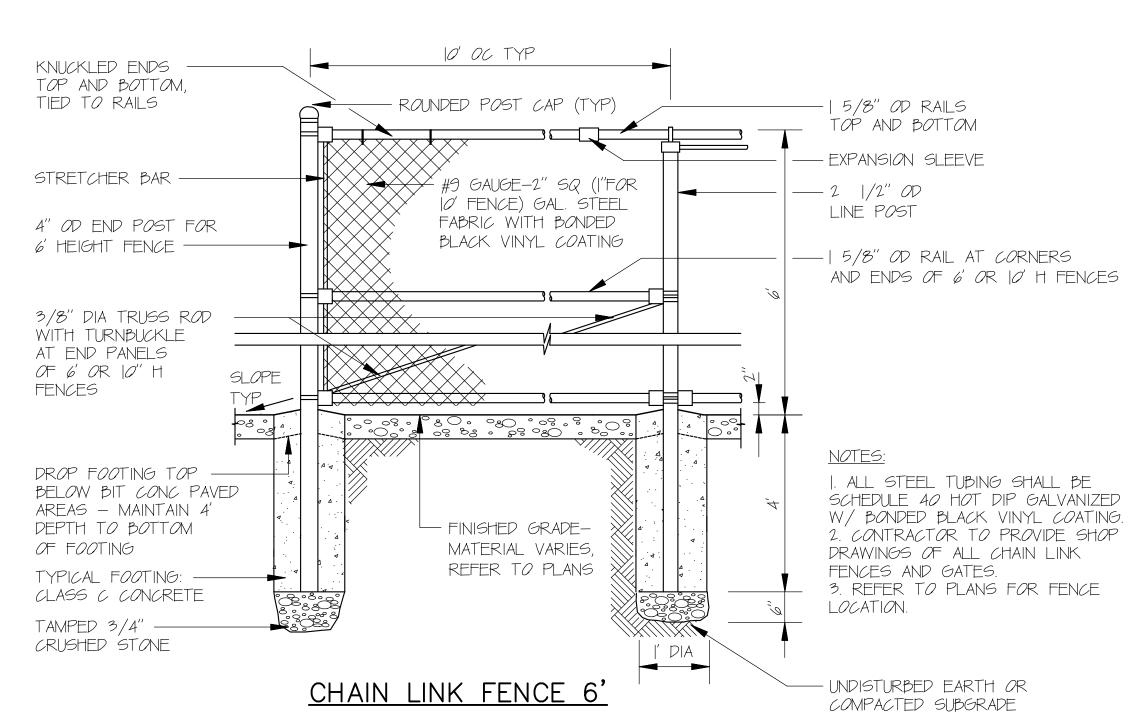


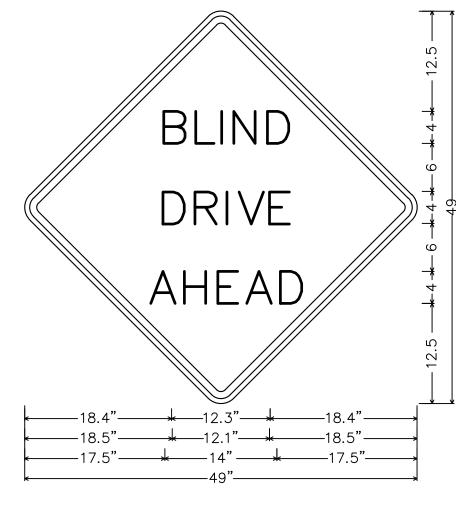
- 1. SIGN MOUNTING HEIGHT SHALL BE A MINIMUM OF 4'-0" AND MAXIMUM OF 5'-0" FROM GRADE TO BOTTOM OF SIGN.
- 2. DANGER SHALL BE IN HELVETICA BOLD FONT. ALL OTHER LETTERING SHALL BE IN HELVETICA FONT.
- 3. SIGN MATERIAL SHALL BE HIGH DENSITY OVERLAY (HDO) PLYWOOD, MARINE QUALITY 3/4" THICKNESS.
- 4. SIGNS (2 TOTAL) SHALL BE MOUNTED ON EAST AND WEST SIDES OF DAM AS SHOWN ON SHEET OO C-107.

WARNING SIGN AT DAM

NOT TO SCALE

NOT TO SCALE





W42-5;

Sign Type: Warning; 36.0" across sides 2.3" Radius, 0.9" Border, 0.6" Indent, Black on, Yellow; "BLIND", C 2K; "DRIVE", C 2K; "AHEAD", C 2K;

NOTES:

- 1. SIGN MOUNTING HEIGHT SHALL BE 7'-0" MIN. FROM GRADE TO BOTTOM OF SIGN.
- 2. SIGN TO BE INSTALLED APPROX. 200' FROM BEGINNING OF WEST SIDE PARKING AREA.

SIGNS

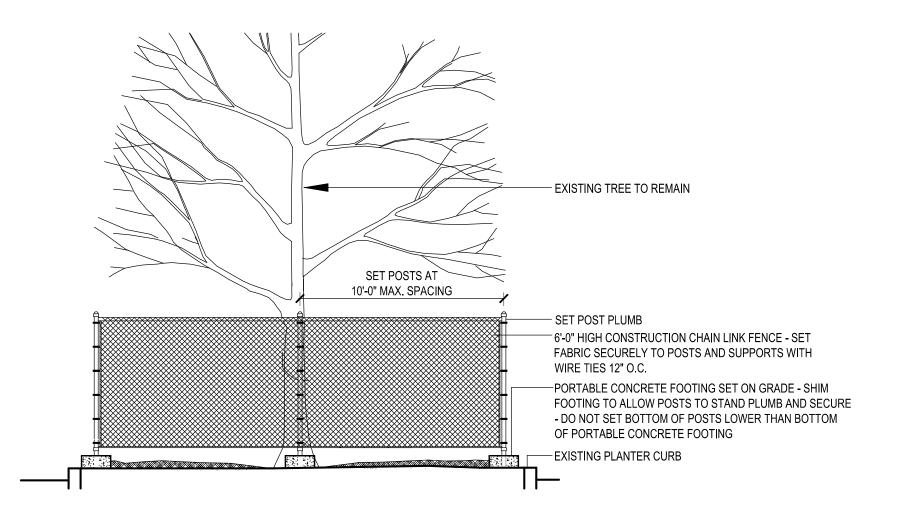
ACCESSIBLE

RESERVED

PARKING

& →

SIGNAGE NOT TO SCALE



NOTE: CUT BRANCHES BELOW TOP OF FENCE PRIOR TO INSTALLING FENCE.

DEPTH OF PIPE INVERT

BELOW GROUND SURFACE

12' TO 20'

7'-0"

D + 5'-0"

TRENCH WIDTH Ws OR Wu

0 TO 12'

5'-0"

D + 3'-0"

NOMINAL PIPE

DIAMETER

24" AND SMALLER

OVER 24"

TREE PROTECTION FENCING

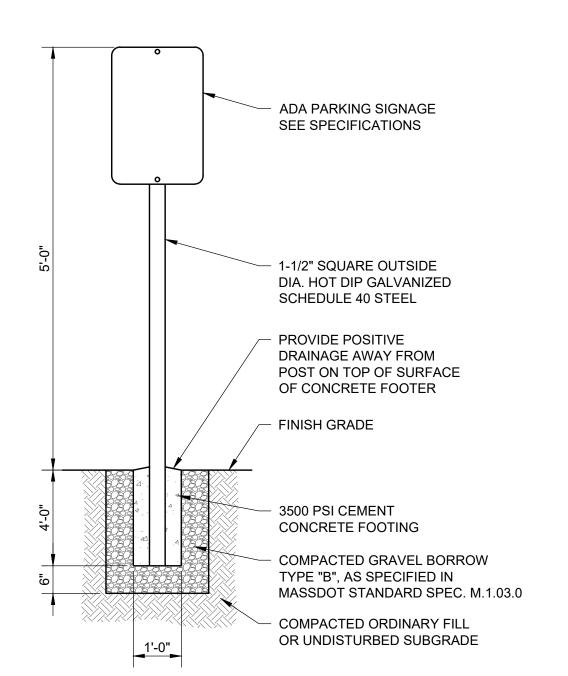
NOT TO SCALE

GENERAL NOTES FOR PIPE TRENCHES

- PIPE TRENCHES MAY BE EXCAVATED WIDER THAN TRENCH WIDTH Ws (SHEETED) OR Wu (UNSHEETED) ABOVE THE TOP OF PIPE ZONE.
- TRENCHES SHALL NOT BE EXCAVATED BEYOND THE TRENCH WIDTH Wu BELOW THE TOP OF PIPE ZONE.
- SHEETING MUST BE USED IF EXCAVATION AND BACKFILL, BELOW NORMAL DEPTH, IS REQUIRED. SHEETING SHALL BE LEFT IN PLACE BELOW A LINE 1'-0" ABOVE THE TOP OF PIPE.
- ALL ROCK WITHIN 3'-0" HORIZONTALLY OF THE ENDS OF BUILDING CONNECTIONS, BRANCHES OR STUBS AND DOWN TO A HORIZONTAL PLANE 6" BELOW THE BOTTOMS OF SUCH CONNECTIONS, BRANCHES OR STUBS, SHALL BE EXCAVATED.

GENERAL NOTES FOR PIPE TRENCHES

NOT TO SCALE 2-1.1.11 (REV. 10-23-95)



ADA PARKING SIGNAGE

-NO LEDGE OR UNEXCAVATED MATERIAL SHALL TRENCH WIDTH Ws OR Wu SHEETING IF USED PROJECT BEYOND SHALL BE LEFT IN THIS LINE Ws/ OR Wu/ PLACE BELOW TOP OF PIPE ZONE, EXCEPT WHERE OTHERWISE INDICATED OR DIRECTED -SELECTED EXCAVATED MATERIAL, COMPACTED AS SPECIFIED PAYMENT LIMITS FOR NORMAL - PAYMENT LIMITS FOR **EXCAVATION** -ROCK EXCAVATION COMPACTED SCREENED GRAVEL UNDISTURBED AGAINST UNDISTURBED MATERIAL MATERIAL OR SHEETING HALF SECTION HALF SECTION IN ROCK IN EARTH

> TRENCH SECTION FOR DI OR PVC PIPE 18 INCH DIAMETER AND SMALLER

> > NOT TO SCALE 2-1.1.1 (REV. 03-15-95)

AECOM

PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

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CONSULTANT

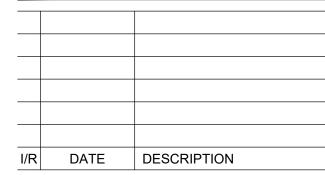
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Drawn By:	M. THIBODEAU
Dept Check:	C. BENZIGER
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

DISCIPLINE

CIVIL SHEET TITLE

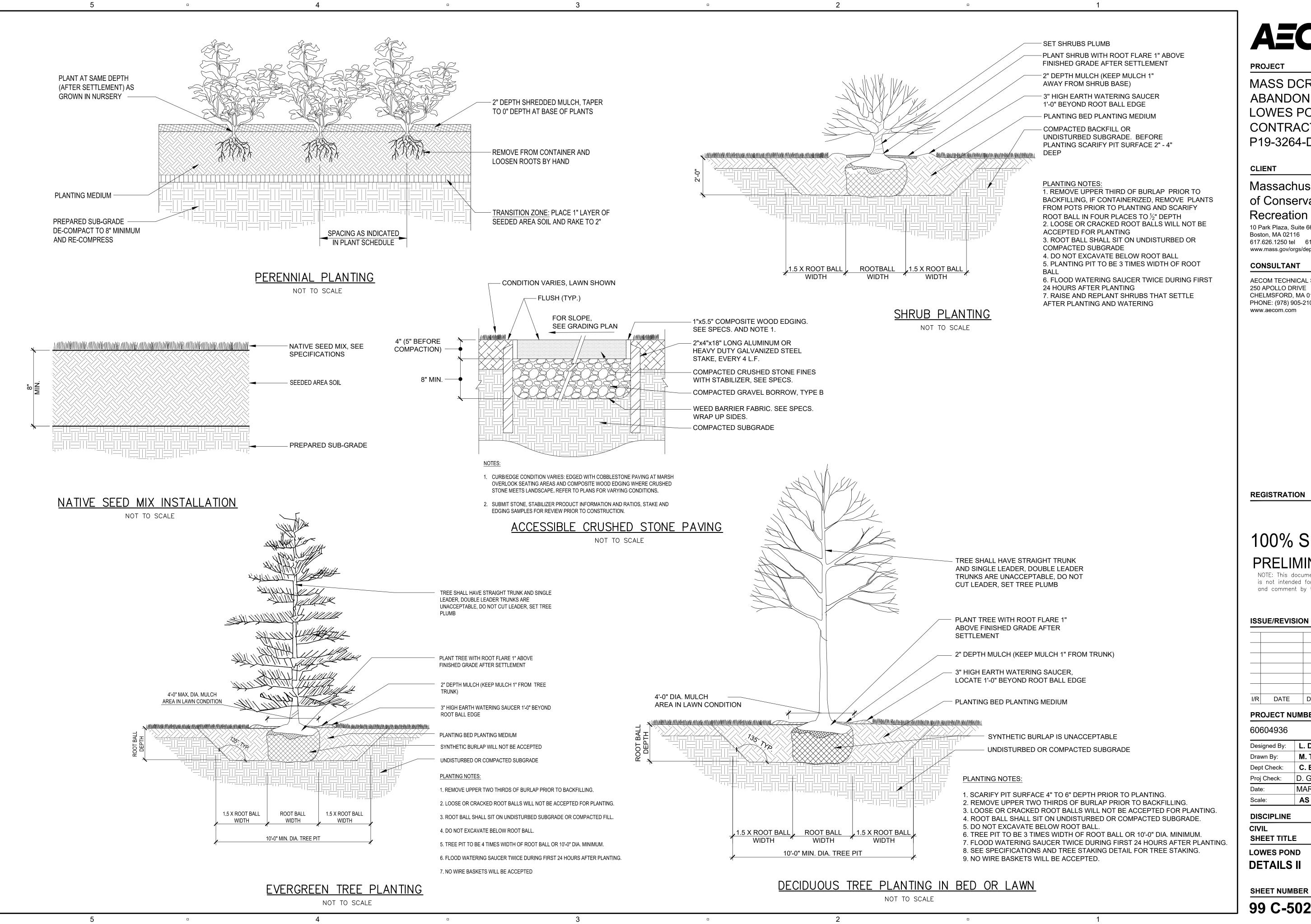
DETAILS I

LOWES POND

99 C-501

SHEET NUMBER

NOT TO SCALE



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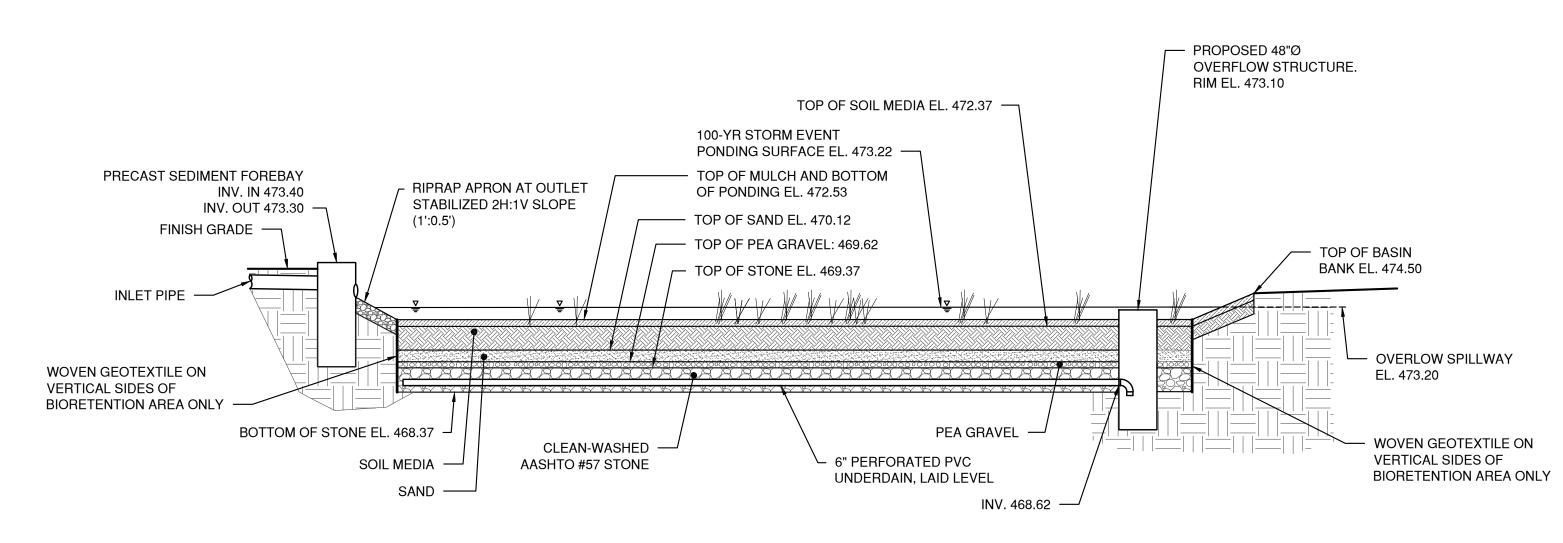
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DISCIPLINE

CIVIL SHEET TITLE

LOWES POND

DETAILS II



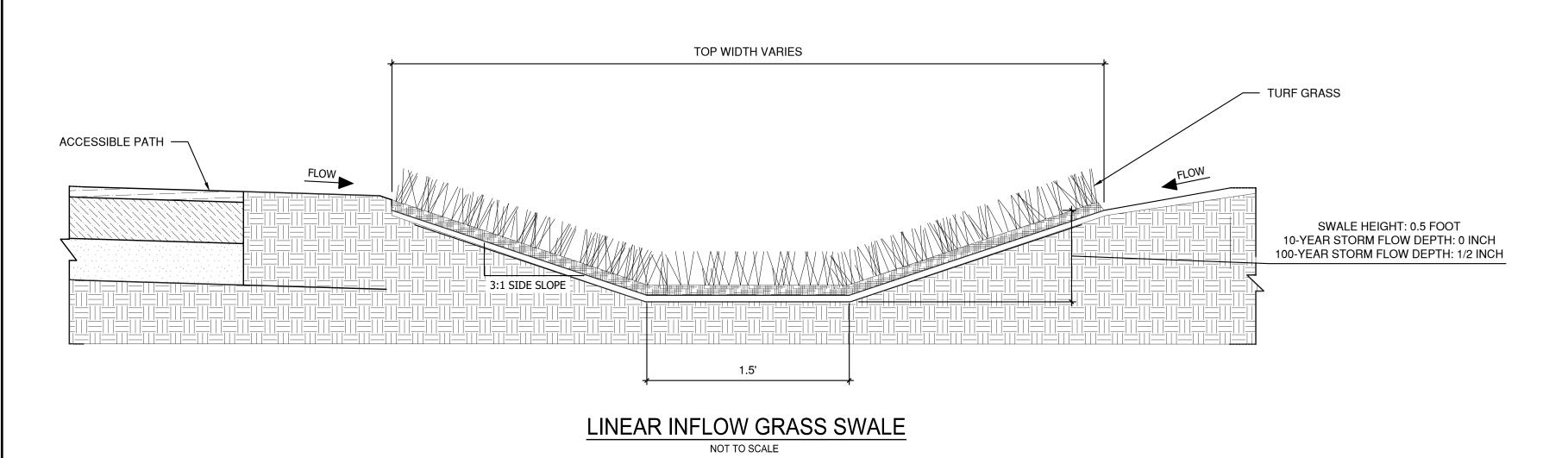
SCHEMATIC - EXFILTRATING BIORETENTION AREA NO. 1

NOT TO SCALE

2" MULCH -PROPOSED 48"Ø OVERFLOW STRUCTURE. - 100-YR STORM EVENT RIM EL. 471.60 PONDING SURFACE EL. 471.64 - TOP OF MULCH AND BOTTOM OF ___ 2" MULCH PONDING EL. 470.93 — 4" TOPSOIL — TOP OF SOIL MEDIA EL. 470.77 PRECAST SEDIMENT FOREBAY ON SLOPES INV. IN 471.80 - RIPRAP APRON AT OUTLET — TOP OF SAND EL. 468.52 INV. OUT 471.70 -STABILIZED 2H:1V SLOPE (1':0.5')— TOP OF PEA GRAVEL: 468.02 - TOP OF BASIN FINISH GRADE BANK EL. 473.00 - TOP OF STONE EL. 467.77 <u>Amala Jen kan kan Bandan ka Takk</u> **WOVEN GEOTEXTILE ON** VERTICAL SIDES OF BIORETENTION AREA ONLY **WOVEN GEOTEXTILE ON** VERTICAL SIDES OF PEA GRAVEL BOTTOM OF STONE EL. 466.77 **BIORETENTION AREA ONLY** CLEAN-WASHED -- 6" PERFORATED PVC AASHTO #57 STONE SOIL MEDIA -- OUTLET PIPE UNDERDAIN, LAID LEVEL SAND -INV. 469.10 · INV. 467.02

SCHEMATIC - EXFILTRATING BIORETENTION AREA NO. 2

NOT TO SCALE



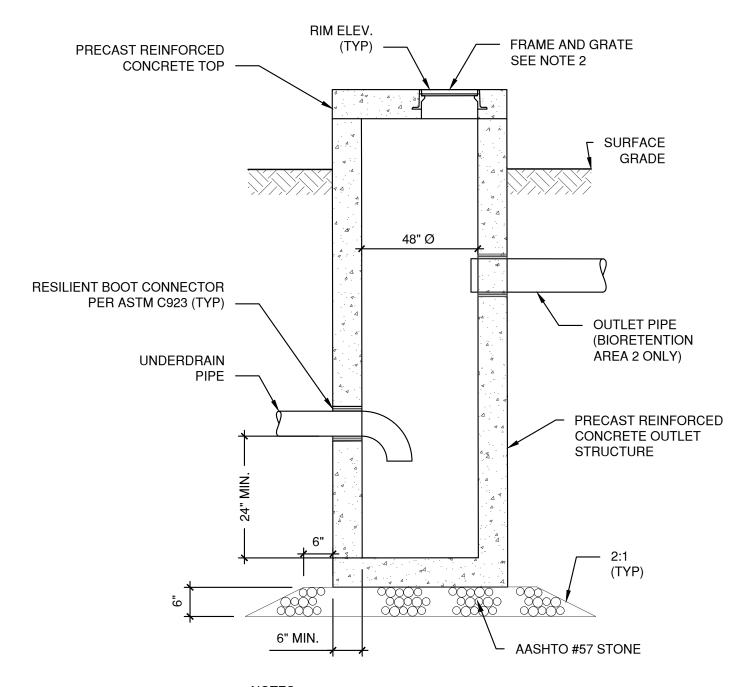
MH FRAME AND COVER - MORTAR FINISH GRADE PRECAST REINFORCED CONCRETE MH FLAT ساساس SLAB TOP **BUTYL RUBBER** SEALANT RESILIENT BOOT CONNECTOR PER ASTM C923 (TYP) 12" OUTLET **INLET PIPE** CORE HOLE 2'-6" DIA. PRECAST REINFORCED CONCRETE MH BARREL NO CEMENT BRICK OR BARREL BLOCK ALLOWED. 3'-0" DIA. **AASHTO #57 STONE** - 12" Ø OPENING

NOTE:

1. PRECAST SEDIMENT FOREBAY, INCLUDING SLAB TOP AND MH FRAME AND COVER, TO BE DESIGNED FOR AASTHO H-20 LOADING.

SECTION

PRECAST SEDIMENT FOREBAY NOT TO SCALE



NOTES:

1. PIPE SIZES, MATERIALS, INVERT ELEVATIONS AND ORIENTATION AS INDICATED ON PLANS AND DETAILS.

2. H20 LOADING IS NOT REQUIRED. ALL JOINTS, ADJUSTMENTS, AND PIPE CONNECTIONS MUST BE

CONCRETE OVERFLOW STRUCTURE

NOT TO SCALE

AECOM

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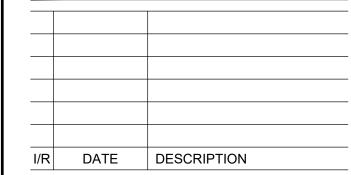
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Designed By:	S. HE
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Dept Check:	C. BENZIGER
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

DISCIPLINE

CIVIL

SHEET TITLE

LOWES POND

DETAILS III

SHEET NUMBER

99 C-503

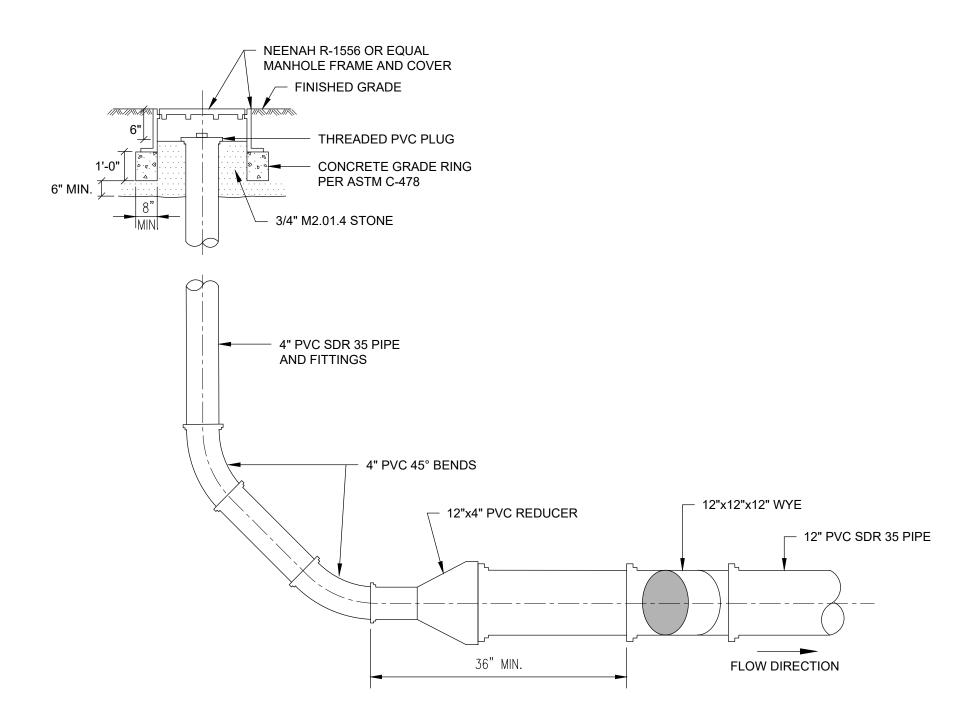
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5

1. PRECAST CONCRETE DEEP SUMP CATCH BASIN, INCLUDING SLAB TOP AND FRAME & GRATE, MUST BE DESIGNED FOR AASHTO H-20 LOADING

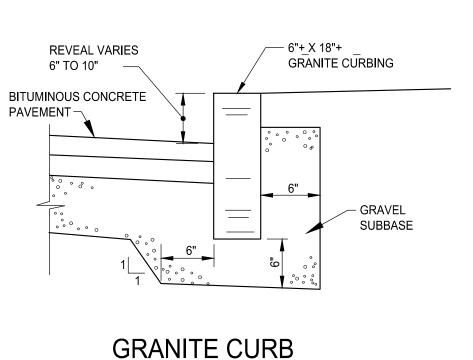
DEEP SUMP CATCH BASIN

NOT TO SCALE



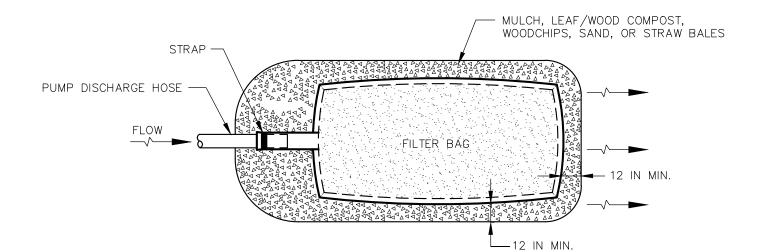
CLEANOUT

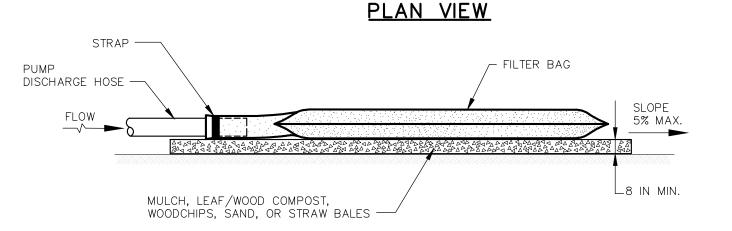
NOT TO SCALE



NOT TO SCALE

2-1.42.1 (REV. 12-15-94)





ELEVATION

CONSTRUCTION SPECIFICATIONS

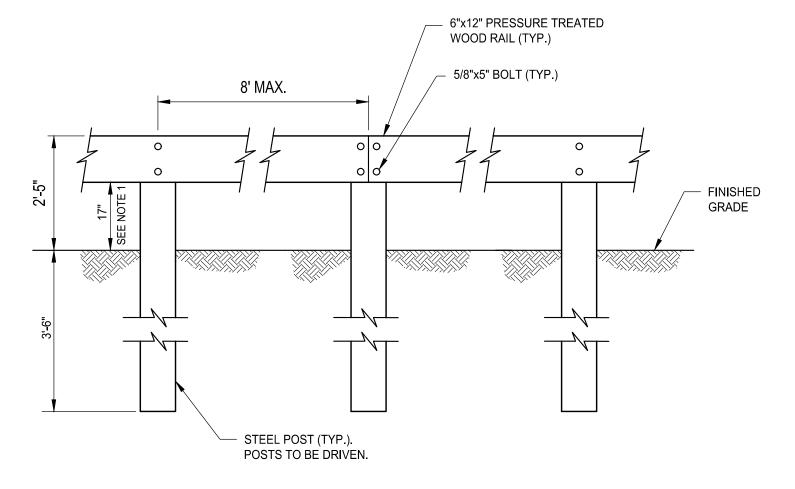
- 1. TIGHTLY SEAL SLEEVE AROUND THE PUMP DISCHARGE HOSE WITH A STRAP OR SIMILAR DEVICE.
- 2. PLACE FILTER BAG ON SUITABLE BASE (E.G., MULCH, LEAF/WOOD COMPOST, WOODCHIPS, SAND, OR STRAW BALES) LOCATED ON A LEVEL OR 5% MAXIMUM SLOPING SURFACE. DISCHARGE TO A STABILIZED AREA. EXTEND BASE A MINIMUM OF 12 INCHES FROM EDGES OF BAG.
- 3. CONTROL PUMPING RATE TO PREVENT EXCESSIVE PRESSURE WITHIN THE FILTER BAG IN ACCORDANCE WITH THE MANUFACTURER RECOMMENDATIONS. AS THE BAG FILLS WITH SEDIMENT, REDUCE PUMPING
- 4. REMOVE AND PROPERLY DISPOSE OF FILTER BAG UPON COMPLETION OF PUMPING OPERATIONS OR AFTER BAG HAS REACHED CAPACITY, WHICHEVER OCCURS FIRST. SPREAD THE DEWATERED SEDIMENT FROM THE BAG IN AN APPROVED UPLAND AREA AND STABILIZE WITH SEED AND MULCH BY THE END OF THE WORK DAY. RESTORE THE SURFACE AREA BENEATH THE BAG TO ORIGINAL CONDITION UPON REMOVAL OF THE DEVICE.
- 5. USE NONWOVEN GEOTEXTILE WITH DOUBLE STITCHED SEAMS USING HIGH STRENGTH THREAD. SIZE SLEEVE TO ACCOMMODATE A MAXIMUM 4 INCH DIAMETER PUMP DISCHARGE HOSE. THE BAG MUST BE MANUFACTURED FROM A NONWOVEN GEOTEXTILE THAT MEETS OR EXCEEDS MINIMUM AVERAGE ROLL VALUES (MARV) FOR THE FOLLOWING:

GRAB TENSILE	250 LB	ASTM D-4632
PUNCTURE	150 LB	ASTM D-4833
FLOW RATE	70 GAL/MIN/FT²	ASTM D-4491
PERMITTIVITY (SEC ⁻¹)	1.2 SEC ⁻¹	ASTM D-4491
UV RESISTANCE	70% STRENGTH @ 500 HOURS	ASTM D-4355
APPARENT OPENING SIZE (AOS)	0.15-0.18 MM	ASTM D -4751
SEAM STRENGTH	90%	ASTM D-4632

6. REPLACE FILTER BAG IF BAG CLOGS OR HAS RIPS, TEARS, OR PUNCTURES. DURING OPERATION KEEP CONNECTION BETWEEN PUMP HOSE AND FILTER BAG WATER TIGHT. REPLACE BEDDING IF IT BECOMES DISPLACED.

SEDIMENTATION FILTER BAG

NOT TO SCALE



WOOD GUARD RAIL MATERIAL SPECIFICATIONS:

POSTS: 6"x4"x8.5 STEEL "H" POST, GALVANIZED

RAILS: 6"x12" PRESSURE TREATED SOUTHERN YELLOW PINE

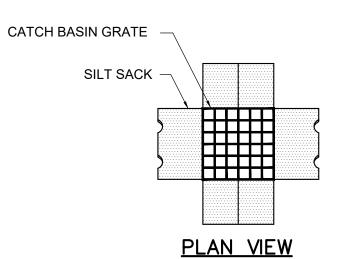
BOLTS: 5/8"x5" GALVANIZED CARRIAGE BOLT WITH NUT AND WASHER

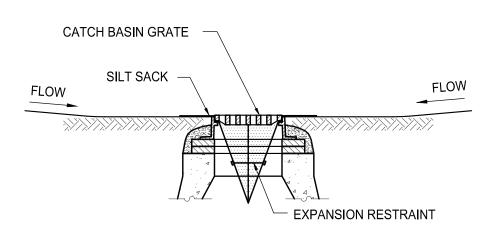
WOOD TREATMENT: .60 ACQ

WOOD GUARD RAIL NOTE:

- 1. MATCH EXISTING GUARDRAIL HEIGHT.
- 2. GUARD RAIL TO BE LOCATED AT SAME OFFSET AS EXISTING GUARD RAIL.

WOOD GUARD RAIL WITH STEEL POSTS





SECTION VIEW

- 1. INSTALL SILT SACK IN ALL CATCH BASINS BEFORE COMMENCING WORK OR IN PAVED AREAS AFTER BINDER COURSE IS PLACED AND STRAW BALES HAVE BEEN REMOVED. GRATE TO BE PLACED OVER SILT SACK.
- 2. SILT SACK SHALL BE INSPECTED PERIODICALLY AND AFTER ALL STORMS.
- 3. EVENTS AND CLEANING OR REPLACEMENT SHALL BE PERFORMED PROMPTLY AS NEEDED.

SILT SACK SEDIMENT TRAP

NOT TO SCALE

AECOM

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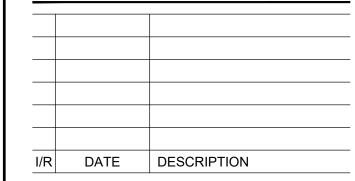
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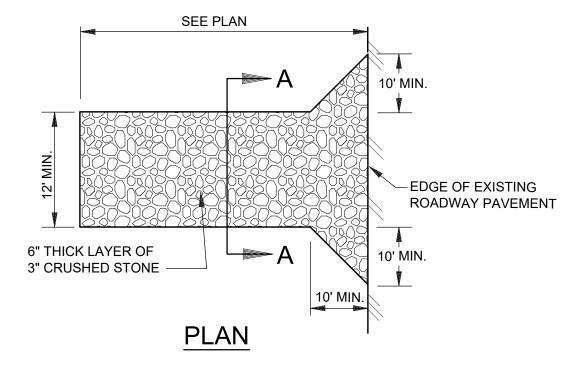
CIVIL SHEET TITLE

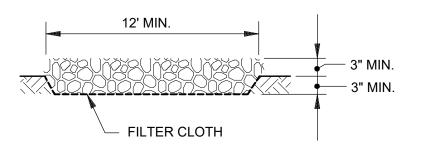
LOWES POND

DETAILS IV

SHEET NUMBER

- 1. SEDIMENT CONTROL MEASURES SHALL BE ADJUSTED TO MEET FIELD CONDITIONS AT THE TIME OF AND DURING ALL PHASES OF CONSTRUCTION AND BE CONSTRUCTED PRIOR TO AND IMMEDIATELY AFTER ANY GRADING OR DISTURBANCE OF EXISTING SURFACE MATERIAL ON THE SITE.
- 2. WEEKLY INSPECTION AND MAINTENANCE OF ALL SEDIMENT CONTROL STRUCTURES SHALL BE PROVIDED TO ENSURE THAT THE INTENDED PURPOSE IS ACCOMPLISHED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT NO SEDIMENT LEAVES THE LIMIT OF WORK. SEDIMENT CONTROL MEASURES SHALL BE IN WORKING CONDITION AT THE END OF EACH DAY.
- 3. THE CONTRACTOR SHALL PREVENT MUD/MATERIAL FROM BEING TRACKED ONTO HUGUENOT ROAD OR ONTO ANY AREAS OUTSIDE THE LIMIT OF WORK. IF DETECTED, DEBRIS MUST BE CLEANED UP IMMEDIATELY AND THE CAUSE MUST BE DETERMINED AND RECTIFIED.
- 4. ALL CONSTRUCTION INGRESS AND EGRESS POINTS SHALL BE PROTECTED WITH A STABILIZED CONSTRUCTION ENTRANCE/EXIT WITH WASH DOWN FACILITIES TO PREVENT TRACKING OF SEDIMENT ONTO THE ROAD.
- 5. ALL SEDIMENT MUST BE PREVENTED FROM ENTERING ANY STORMWATER COLLECTION SYSTEM, EXCEPT THOSE SYSTEMS SPECIFICALLY DESIGNED AS CONSTRUCTION—PERIOD SEDIMENT REMOVAL DEVICES.
- 6. CONTRACTOR SHALL PROVIDE COMPOST SILT SOCKS, AS NECESSARY, TO PREVENT SEDIMENT RUNOFF FROM ALL IMPACTED AREAS ASSOCIATED WITH CONSTRUCTION ACTIVITIES. THE SILT FENCE SHALL BE INSPECTED DAILY. THE CONTRACTOR SHALL REMOVE ACCUMULATED SEDIMENT AND REPAIR THE SILT FENCE AS DETERMINED DURING THE INSPECTION TO ORIGINAL CONDITION WITHIN THREE (3) DAYS OF INSPECTION.
- 7. ONLY DISTURB, CLEAR, OR GRADE AREAS NECESSARY FOR CONSTRUCTION. FLAG OR OTHERWISE DELINEATE AREAS NOT TO BE DISTURBED. EXCLUDE VEHICLES AND CONSTRUCTION EQUIPMENT FROM AREAS NOT NECESSARY FOR CONSTRUCTION.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR REMOVAL AND DISPOSAL OF ALL EROSION AND SEDIMENT CONTROLS AT THE COMPLETION OF CONSTRUCTION AND WHEN THE CONTRIBUTING DRAINAGE AREA HAS BEEN PERMANENTLY STABILIZED. REMOVE TEMPORARY EROSION CONTROL MEASURES WITHIN 30 DAYS AFTER THE TRIBUTARY AREA HAS BEEN PERMANENTLY STABILIZED. REMOVE ANY ACCUMULATED SEDIMENTS AND STABILIZE ALL DISTURBED
- 9. CONTRACTOR SHALL PROTECT AND MAINTAIN EXISTING DRAINAGE INLETS, DITCHES, SWALES, CULVERTS, AND DETENTION PONDS WITHIN THE LIMITS OF CONSTRUCTION AS WELL AS ANY OTHER AREAS IMPACTED BY CONSTRUCTION OPERATIONS. IF A DRAINAGE DITCH OR SWALE IS TO BE RELOCATED, THE RELOCATED DITCH SHALL BE IN PLACE PRIOR TO DISTURBANCE OF THE EXISTING DITCH.
- 10. PROVIDE AND STABILIZE STORMWATER TREATMENT AREAS, DITCHES AND SWALES AS EARLY AS PRACTICAL DURING CONSTRUCTION BUT PRIOR TO DISTURBING THE TRIBUTARY AREA AND PRIOR TO DIRECTING RUNOFF TO THE TREATMENT AREAS. DITCHES AND SWALES.
- 11. STABILIZE ACCESS DRIVES AND PARKING AREAS WITHIN 72 HOURS OF ACHIEVING FINISH GRADE.
- 12. INSPECT DISTURBED AREAS, STORAGE AREAS, AND EROSION CONTROL MEASURES WEEKLY UNLESS OTHERWISE INDICATED AND AFTER EVERY STORM EVENT OF 1/4 INCH OF RAINFALL OR MORE. ALL INSPECTIONS SHALL BE PERFORMED BY PERSONS FAMILIAR WITH EROSION AND SEDIMENT CONTROL PRACTICES.
- 13. AFTER ANY SIGNIFICANT RAINFALL (1/4 INCH OR GREATER), SEDIMENT CONTROL STRUCTURES SHALL BE INSPECTED FOR INTEGRITY. ANY DAMAGED DEVICES SHALL BE CORRECTED IMMEDIATELY.
- 14. DOCUMENT EACH INSPECTION IN A REPORT AND LOG, INCLUDING THE FOLLOWING: DATE, NAME(S) AND TITLE OF THE PERSONNEL MAKING THE INSPECTION, WEATHER AND DESCRIPTION OF ANY DISCHARGE OCCURRING AT THE TIME OF INSPECTION, LOCATION OF ANY DISCHARGE OF SEDIMENT OR POLLUTANTS, BEST MANAGEMENT PRACTICES THAT NEED TO BE MAINTAINED, BEST MANAGEMENT PRACTICES THAT FAILED OR OTHERWISE PROVED INADEQUATE, LOCATION WHERE ADDITIONAL BEST MANAGEMENT PRACTICES ARE NEEDED, AND CORRECTIVE ACTION REQUIRED. ALSO NOTE WHAT ACTION WAS TAKEN TO CORRECT DEFICIENCIES OR ENHANCE CONTROLS, INCLUDING THE DATE THE CORRECTIVE ACTION WAS TAKEN. PERFORM CORRECTIVE ACTION OF EROSION CONTROL MEASURES IMMEDIATELY.
- 15. ANY SEDIMENT TRACKED ONTO PAVED AREAS SHALL BE SWEPT UP AND REMOVED AT THE END OF EACH WORKING DAY AND THE CAUSE OF THE INCIDENT DETERMINED AND CORRECTED.
- 16. STABILIZED PORTIONS OF A SITE SHALL BE INSPECTED AT LEAST ONCE PER MONTH.
- 17. MAINTAIN EROSION CONTROL MEASURES FOR THE DURATION OF THE PROJECT AND UNTIL PERMANENT STABILIZATION OF THE ENTIRE SITE IS ESTABLISHED.
- 18. PROTECT STABILIZED AREAS FROM EROSION AND IMMEDIATELY REPAIR AND RE-VEGETATE ERODED AREAS.

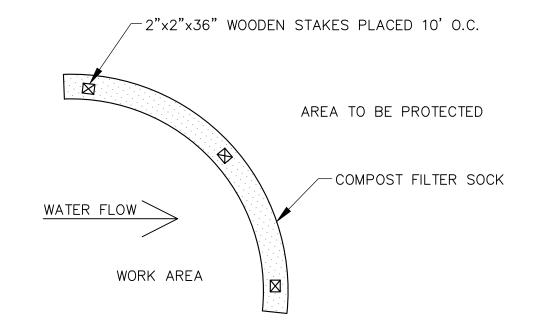




SECTION A-A

TEMPORARY
CONSTRUCTION ENTRANCE

NOT TO SCALE



PLAN

BLOWN/PLACED COMPOST — COMPOST FILTER SOCK (12" TYP.)

WORK AREA AREA TO BE PROTECTED

12" MIN.

SECTION

NOTE

1. SEE SPECIFICATION FOR COMPOST FILTER SOCK AND COMPOST FILL MATERIAL REQUIREMENTS.

COMPOST SILT SOCK

AECOM

PROJECT

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LOWES POND DAM
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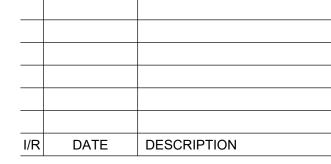
AECOM TECHNICAL SERVICES, INC. 250 APOLLO DRIVE CHELMSFORD, MA 01824 PHONE: (978) 905-2100 www.aecom.com

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ISSUE/REVISION



PROJECT NUMBER

60604936

Designed By:	S. HE
Drawn By:	S. NAPOLITANO
Dept Check:	C. BENZIGER
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

DISCIPLINE

CIVIL SHEET TITLE

LOWES POND
DETAILS V

SHEET NUMBER

- 2. DESIGN LOADS:
- A. FLOOR LIVE LOAD: N.A.
- B. ROOF LIVE LOAD: N.A.
- C. SNOW LOAD:

GROUND SNOW LOAD (PG): 50 PSF FLAT ROOF SNOW LOAD (PF): 35 PSF SNOW EXPOSURE FACTOR (CE): 1.00 SNOW LOAD IMPORTANCE FACTOR (IS): 1.1 SNOW THERMAL FACTOR (CT): 1.10

THE MORE STRINGENT REQUIREMENT SHALL GOVERN.

- D. WIND LOAD:
- ULTIMATE DESIGN WIND SPEED: 136 MPH RISK CATEGORY: IV
- E. SEISMIC LOAD:

SEISMIC IMPORTANCE FACTOR (IE): 1.25 RISK CATEGORY: IV MAPPED SPECTRAL RESPONSE COEFFICIENTS S(S): 0.190 S(1): 0.057 SPÉCTRAL RESPONSE COEFFICIENTS S(DS): 0.203 S(D1): 0.091 SITE MODIFIED PEAK GROUND ACCELERATION: 0.164 SITE CLASS: CLASS D SEISMIC DESIGN CATEGORY: C

- 3. PROTECT ALL STRUCTURES AGAINST HYDRAULIC UPLIFT UNTIL STRUCTURES ARE COMPLETED AND BACKFILLED. MAXIMUM FLOOD GROUNDWATER AT THE SITE IS ELEVATION 477.50 FEET
- 4. FROST DEPTH AT THE SITE IS 4'-0" BELOW FINISHED GRADE.
- 5. DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION. MISSING OR CONFLICTING DIMENSIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER.
- DIMENSIONS AND ELEVATIONS INDICATED ON EXISTING STRUCTURES HAVE BEEN OBTAINED FROM DRAWINGS OR FIELD SURVEYS. VERIFY ALL DIMENSIONS AND ELEVATIONS THAT ARE REQUIRED FOR FABRICATION AND INSTALLATION OF ADDITIONS TO EXISTING STRUCTURES WITH FIELD MEASUREMENTS.
- 7. REFER TO CIVIL DRAWINGS AND SPECIFICATIONS FOR LOCATIONS AND DIMENSIONS OF CHASES, SLOTS, INSERTS, CURBS, OPENINGS, SLEEVES, ANCHOR BOLTS, FLOOR PITCHES, ANGLE FRAMES, GATE FRAMES, AND OTHER PROJECT REQUIREMENTS NOT INDICATED ON STRUCTURAL DRAWINGS.
- 8. COORDINATE DIMENSIONS AND VERIFY LOCATION OF STRUCTURAL ELEMENTS WITH PURCHASED EQUIPMENT. ANCHOR BOLTS SHALL BE SIZED AND FURNISHED BY THE EQUIPMENT SUPPLIER UNLESS OTHERWISE INDICATED.
- 9. PROVIDE OPENINGS REQUIRED FOR PURCHASED EQUIPMENT. PROVIDE ANCHOR BOLTS, NUTS, NON-SHRINK NON-METALLIC GROUT, CONCRETE PADS AND REINFORCING STEEL REQUIRED FOR THE INSTALLATION OF EQUIPMENT.
- 10. STANDARD DETAILS AS SHOWN ON THE STRUCTURAL STANDARD DETAIL SHEETS ARE APPLICABLE TO ALL STRUCTURAL WORK EXCEPT WHERE A SPECIFIC SECTION OR DETAIL IS SHOWN OTHERWISE

<u>ALUMINUM RAILINGS AND GUARDRAILS</u>

- PROVIDE ALUMINUM RAILING WHERE INDICATED.
- 2. PROVIDE MOUNTINGS FOR REMOVABLE ALUMINUM RAILING AROUND OPENINGS WHERE INDICATED. COMPLETE WITH CAPS.
- 3. DESIGN CRITERIA
- A. RAILINGS AND GUARDRAILS SHALL BE DESIGNED FOR A LINE LOAD OF 100 PLF VERTICAL PLUS 50 PLF HORIZONTAL APPLIED CONCURRENTLY OR A CONCENTRATED LOAD OF 200 POUNDS APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE RAIL, WHICHEVER PRODUCES THE MOST EXTREME CONDITION.
- B. INTERMEDIATE RAILS, BALUSTERS AND PANELS OR FILLERS SHALL BE DESIGNED FOR A UNIFORM LOAD OF 25 PSF OVER THE GROSS AREA OF THE GUARD OF WHICH THEY ARE A PART. THIS LOADING NEED NOT BE ADDED TO THE LOADING OF THE MAIN MEMBERS PRESCRIBED ABOVE.
- C. HANDRAILS SHALL BE DESIGNED FOR A LINE LOAD OF 50 PLF APPLIED IN ANY DIRECTION OR A CONCENTRATED LOAD OF 200 POUNDS APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE HANDRAIL, WHICHEVER PRODUCES THE MOST EXTREME CONDITION.
- 4. ALUMINUM RAILING AND GUARDRAILS SHALL CONFORM TO ALLOY 6061-T6.
- 5. WELDING SHALL CONFORM TO THE STRUCTURAL WELDING CODE, AWS D1.2.
- 6. WELDING SHALL BE BY THE METAL INERT GAS (MIG) OR THE TUNGSTEN INERT GAS (TIG) PROCESS USING 5356 FILLER ALLOY.
- 7. FIELD WELDING OF ALUMINUM RAILING AND GUARDRAILS IS NOT PERMITTED UNLESS SPECIFICALLY INDICATED.
- 8. PROVIDE ASTM F 593 TYPE 316 STAINLESS STEEL BOLTS FOR BOLTED CONNECTIONS. PROVIDE DRILLED HOLES 1/16" LARGER THAN THE BOLT DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE ASTM F 594 STAINLESS STEEL NUTS WITH STAINLESS STEEL WASHERS.
- 9. PAINT ALUMINUM IN CONTACT WITH CONCRETE WITH ASPHALTIC PAINT OR PROVIDE SHEET NEOPRENE BARRIER.
- 10. PROVIDE DISSIMILAR METAL PROTECTION AT LOCATIONS WHERE DISSIMILAR METALS ARE IN CONTACT. PROTECT WITH A MINIMUM 4-MIL DRY THICKNESS COAT OF ZINC CHROMATE PRIMER ON THE ALUMINUM SURFACES AND A MINIMUM 2-MIL DRY THICKNESS COAT OF ALL-METAL PRIMER FOLLOWED BY ONE COAT OF MINIMUM 3-MIL DRY THICKNESS ALUMINUM PAINT TO THE DISSIMILAR METAL.
- 11. USE NON-SHRINK, NON-METALLIC GROUT UNDER BASE PLATES.

CONCRETE

- 1. CONCRETE WORK SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301) AND THE CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES (ACI 350).
- 2. USE NORMAL WEIGHT CAST-IN-PLACE CONCRETE WITH ASTM C 150. TYPE II CEMENT FOR ALL CONCRETE WORK.
- 3. CONCRETE SHALL HAVE A. A 28-DAY COMPRESSIVE STRENGTH OF 4,500 PSI;
 - B. A MINIMUM UNIT WEIGHT OF 150 POUNDS PER CUBIC FOOT. C. A MAXIMUM WATER-CEMENT RATIO OF 0.42:
- CONTAIN A SHRINKAGE REDUCING ADMIXTURE OR USE SHRINKAGE COMPENSATING CEMENT; E. A 21-DAY DRYING SHRINKAGE OF 0.028 PERCENT OR LESS AND A 28-DAY DRYING SHRINKAGE OF 0.032 PERCENT OR LESS WHEN TESTED

IN ACCORDANCE WITH ASTM C 157 AS MODIFIED BY THE PROJECT

- 4. PROVIDE CONCRETE UNDER FOUNDATIONS HAVING A 28-DAY COMPRESSIVE STRENGTH OF 1,500 PSI WHEN INDICATED OR REQUIRED.
- 5. AIR-ENTRAIN ALL CONCRETE.

SPECIFICATIONS.

- 6. REINFORCING BARS SHALL CONFORM TO ASTM A 615, GRADE-60.
- 7. WELDING REINFORCING BARS IS PROHIBITED EXCEPT WHERE SPECIFICALLY INDICATED ON THE DRAWINGS. REINFORCING BARS INDICATED TO BE WELDED SHALL CONFORM TO ASTM A 706, GRADE-60.
- 8. PROVIDE REINFORCING BAR SUPPORTS, SPACERS, AND ACCESSORIES AS RECOMMENDED IN ACI 315. PROVIDE PLASTIC BOOTED ACCESSORIES IN CONTACT WITH EXPOSED SURFACES. PROVIDE MINIMUM #5 SUPPORT BARS.
- 9. WAIT A MINIMUM OF 72 HOURS BEFORE PLACING ADJACENT CONCRETE SECTIONS.
- 10. SET AND MAINTAIN REINFORCEMENT AT THE CLEAR DISTANCES FROM THE SURFACE OF CONCRETE AS SHOWN IN THE STANDARD DETAIL UNLESS OTHERWISE INDICATED.
- 11. PROVIDE EMBEDMENT AND SPLICES OF REINFORCEMENT AS SHOWN IN THE STANDARD DETAILS UNLESS OTHERWISE INDICATED.
- 12. PROVIDE ADDITIONAL REINFORCEMENT ALONG EACH SIDE OF OPENINGS AS INDICATED IN THE STANDARD DETAILS UNLESS OTHERWISE INDICATED.
- 13. PROVIDE CONSTRUCTION AND EXPANSION JOINTS WHERE INDICATED. DO NOT OMIT OR ADD JOINTS.
- 14. INTENTIONALLY ROUGHEN SURFACE OF HORIZONTAL CONSTRUCTION JOINTS IN WALLS AND AT BASE OF WALL TO 1/4" AMPLITUDE.
- 15. PROVIDE 3/4-INCH CHAMFER ON ALL EXPOSED CORNERS OF CONCRETE ELEMENTS.
- 16. STEEL TROWEL FINISH THE TOP OF ALL WALLS AND SURFACES OF THE SPILLWAY UNLESS OTHERWISE INDICATED OR SPECIFIED.

CONCRETE ANCHORS

- 1. ADHESIVE TYPE ANCHORS SHALL CONFORM TO THE REQUIREMENTS OF ASTM E 488, 'STANDARD TEST METHODS FOR STRENGTH OF ANCHORS IN CONCRETE AND MASONRY ELEMENTS', ASTM 1512, 'STANDARD TEST METHODS FOR TESTING BOND PERFORMANCE OF ADHESIVE-BONDED ANCHORS', ICBO ES AC-308, 'ACCEPTANCE CRITERIA FOR POST-INSTALLED ADHESIVE ANCHORS' AND ACI 355.4 'QUALIFICATION OF POST-INSTALLED ANCHORS IN CONCRETE'.
- 2. PROVIDE SIZE, TYPE AND EMBEDMENT OF ANCHOR INDICATED INSTALLED TO DEVELOP THE MAXIMUM CAPACITY FOR THE EMBEDMENT, TYPE AND ANCHOR SIZE WITH A MINIMUM SAFETY FACTOR OF FOUR.
- 3. ANCHORS SHALL HAVE STANDARD UNC THREADS, UNLESS OTHERWISE INDICATED.
- 4. ANCHOR INSTALLATION SHALL CONFORM TO THE MANUFACTURER'S CURRENT PRINTED INSTRUCTIONS.
- 5. REMOVE DUST AND DEBRIS FROM DRILLED HOLES USING COMPRESSED AIR OR VACUUM AT BOTTOM OF HOLE. IMMEDIATELY REMOVE STANDING WATER AND THOROUGHLY DRY HOLES TO RECEIVE ADHESIVE ANCHORS.
- 6. DO NOT HAMMER IN ANCHORS. INSTALL ANCHORS USING A ROTARY HAMMER DRILL AS RECOMMENDED BY ANCHOR MANUFACTURER. THE USE OF HAMMER IN TYPE ADHESIVE CAPSULES IS PROHIBITED.
- 7. PROVIDE ADHESIVE TYPE ANCHORS CONSISTING INJECTABLE ADHESIVE CARTRIDGES AND THREADED ANCHOR BOLTS SUPPLIED AS A SYSTEM BY ONE MANUFACTURER. PROVIDE EACH ANCHOR WITH NUT AND WASHER. AFTER INSTALLATION, DO NOT DISTURB OR LOAD ANCHORS BEFORE THE SPECIFIED CURING TIME ELAPSES (MAY VARY WITH TEMPERATURE). ADHESIVE ANCHORING SYSTEM SHALL BE:
 - A. HILTI: HIT-HY 200R;
 - B. ITW REDHEAD/RAMSET: EPCON G5;
 - C. POWERS FASTENERS: PE1000+;
 - D. SIMPSON: STRONG—TIE SET—XP; E. OR APPROVED EQUAL

FOUNDATION

- 1. FOUNDATION DESIGNS ARE BASED UPON THE ALLOWABLE ROCK BEARING CAPACITIES OF 8,000 PSF.
- PERCENT COMPACTION IS DEFINED AS THE RATIO OF THE FIELD DRY DENSITY. DETERMINED BY ASTM D-1556, TO THE MAXIMUM DRY DENSITY, DETERMINED BY ASTM-D 1557 (MODIFIED PROCTOR).
- 3. ALL FOUNDATIONS SHALL BEAR ON COMPETENT BEDROCK.
- 4. SPILLWAY UNDULATING ROCK SURFACE SHALL BE PREPARED TO A MAXIMUM SLOPE OF 1H:1V WITH NO OVERHANGS. THE OVERALL ROCK SURFACE SHALL BE SLOPED UPWARD FROM HEEL (UPSTREAM) TO TOE (DOWNSTREAM) BETWEEN 1V:20H AND 1V:10H. ALL LOOSE OR UNSOUND ROCK SHALL BE REMOVED AND ALL DIRT AND DUST SHALL BE REMOVED BY AIR OR WATER BLASTING PRIOR TO CONCRETE OR GROUT PLACEMENT. OPEN JOINTS SHALL BE CLEANED OUT AND FILLED WITH GROUT PRIOR TO SPILLWAY CONCRETE PLACEMENT. ROCK SURFACE SHALL BE OBSERVED BY THE ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
- 5. TRAINING WALL FOUNDATION UNDULATING ROCK SURFACE SHALL BE CLEARED OF ALL LOOSE OR UNSOUND ROCK AND ALL DIRT AND DUST SHALL BE REMOVED BY AIR OR WATER BLASTING PRIOR TO CONCRETE PLACEMENT.
- 6. SPILLWAY UNDULATING ROCK SURFACE SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER OF RECORD OR A QUALIFIED INDIVIDUAL INSPECTING ON BEHALF OF THE GEOTECHNICAL ENGINEER OF RECORD.
- 7. PREPARED UNDULATING ROCK SURFACE SHALL BE COATED WITH A CONCRETE BONDING AGENT PRIOR TO CONCRETE PLACEMENT
- 8. DO NOT PLACE FOUNDATION CONCRETE IN WATER OR ON FROZEN OR DISTURBED GROUND.
- 9. A UNIT WEIGHT OF SOIL OF 130 PCF WAS USED IN THE DESIGN.
- 10. TRAINING WALLS HAVE BEEN DESIGNED FOR ACTIVE SOIL PRESSURES OF AN EQUIVALENT FLUID LATERAL EARTH PRESSURE OF 43 PCF ABOVE THE MAXIMUM GROUNDWATER ELEVATION AND AN EQUIVALENT FLUID LATERAL EARTH PRESSURE OF 85 PCF BELOW THE MAXIMUM GROUNDWATER ELEVATION PLUS A UNIFORM LATERAL PRESSURE OF 100 PSF TO ACCOUNT FOR A 300 PSF SURFACE SURCHARGE ADJACENT TO THE WALLS.
- 11. DO NOT BACKFILL AGAINST CONCRETE WALLS UNTIL WALLS HAVE REACHED THE SPECIFIED DESIGN STRENGTH.

CONCRETE REPAIR

- 1. TAKE MEASURES TO PREVENT DAMAGE TO STRUCTURES DURING REPAIR OPERATIONS. DAMAGE THAT DOES OCCUR SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT NO ADDITIONAL COST TO THE OWNER.
- 2. SHORE STRUCTURES TO UNLOAD MEMBERS BEFORE REPAIRING STRUCTURAL ELEMENTS.
- 3. DO NOT CUT EXISTING REINFORCING BARS AT INTERFACES BETWEEN REPAIR WORK AND EDGES OF EXISTING SURFACES.
- 4. DAMAGE DUE TO REPAIR OPERATIONS SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT THE CONTRACTOR'S EXPENSE. FRACTURED CONCRETE SHALL BE REPAIRED WITH A PRESSURE INJECTED EPOXY SYSTEM APPROVED BY THE ENGINEER.
- 5. PATCH AND FINISH EXPOSED SURFACES TO MATCH THE ADJACENT AREA TO THE SATISFACTION OF THE ENGINEER.

DEMOLITION

- TAKE MEASURES TO PREVENT DAMAGE TO EXISTING STRUCTURES TO REMAIN DURING DEMOLITION. DAMAGE THAT DOES OCCUR SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT NO ADDITIONAL COST TO THE OWNER.
- 2. SHORE EXISTING STRUCTURES TO UNLOAD MEMBERS BEFORE REMOVING STRUCTURAL ELEMENTS
- 3. DO NOT OVER CUT CORNERS WHEN USING A ROTARY SAW. CORE HOLES AT CORNERS PRIOR TO SAW CUTTING AND CHIP FLUSH AS INDICATED IN THE STANDARD DETAIL
- 4. DO NOT CUT EXISTING REINFORCING BARS AT INTERFACES WITH NEW WORK. INCORPORATE EXISTING BARS INTO NEW WORK INSOFAR AS PRACTICABLE. REMOVE THOSE REINFORCING BARS WHICH CANNOT BE BENT INTO AREAS OF NEW CONCRETE WORK.
- 5. INSURE ADJACENT CONCRETE WILL NOT BE FRACTURED WHEN THE CONCRETE TO BE REMOVED IS BROKEN OUT. REMOVE CONCRETE ANCHORS USED TO SECURE CORING EQUIPMENT, AND FILL HOLES WITH NON-SHRINK, NON-METALLIC GROUT.
- 6. DAMAGE DUE TO DEMOLITION OPERATIONS SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT THE CONTRACTOR'S EXPENSE. FRACTURED CONCRETE AND MASONRY SHALL BE REPAIRED WITH A PRESSURE INJECTED EPOXY SYSTEM APPROVED BY THE ENGINEER.
- 7. PATCH AND FINISH EXPOSED SURFACES TO MATCH THE ADJACENT AREA UNLESS OTHERWISE INDICATED OR SPECIFIED.

WATERSTOPS

- 1. PROVIDE WATERSTOPS WHERE SHOWN ON CONTRACT DOCUMENTS.
- 2. WATERSTOPS SHALL FORM A CONTINUOUS WATERTIGHT DIAPHRAGM TO PREVENT LEAKAGE.
- 3. TERMINATE VERTICAL WATERSTOPS THREE INCHES BELOW TOP OF CONCRETE
- 4. PROVIDE 9" BULBED WATERSTOPS IN CONTRACTION AND EXPANSION JOINTS AND 6" RIBBED WATERSTOPS IN CONSTRUCTION JOINTS.
- 5. USE FACTORY MADE CROSSES, TEES AND ELLS AT ALL CORNERS AND INTERSECTIONS.
- 6. INSTALL WATERSTOPS IN CONTINUOUS LENGTHS TO MINIMIZE FIELD SPLICES.
- 7. PROVIDE 1-INCH MINIMUM CLEARANCE BETWEEN WATERSTOPS AND REINFORCING STEEL OR OTHER EMBEDDED ITEMS.
- 8. SECURE WATERSTOPS IN POSITION WITH TIE WIRE FROM LOOPS OR GROMMETS TO ADJACENT REINFORCEMENT ON BOTH SIDES AT 12 INCHES MAXIMUM SPACING ALONG EACH EDGE.
- 9. PROTECT EXPOSED WATERSTOP FROM DAMAGE.
- 10. PROVIDE HYDROPHILIC WATERSTOPS ONLY WHERE INDICATED ON THE DRAWINGS.
- 11. PROTECT HYDROPHILIC WATERSTOPS FROM BECOMING WET PRIOR TO CONCRETE PLACEMENT. HYDROPHILIC WATERSTOPS THAT HAVE BECOME WET PRIOR TO CONCRETE PLACEMENT SHALL BE REMOVED AND DISCARDED.

<u>ALUMINUM</u>

INDICATED.

- 1. STRUCTURAL ALUMINUM TO CONFORM TO ALLOY 6061-T6. DETAILING. FABRICATION AND ERECTION OF STRUCTURAL ALUMINUM SHALL COMPLY WITH ALL APPLICABLE OSHA REGULATIONS. DETAIL AND FABRICATE IN CONFORMANCE HP WITH THE LATEST ALUMINUM ASSOCIATION DESIGN MANUAL AND THE SPECIFICATIONS AND GUIDELINES FOR STRUCTURES OF ALUMINUM.
- 2. WELDING SHALL CONFORM TO THE STRUCTURAL WELDING CODE, AWS D1.2.
- 3. WELDING SHALL BE BY THE METAL INERT GAS (MIG) OR THE TUNGSTEN INERT
- GAS (TIG) PROCESS USING 5356 FILLER ALLOY. MFR 4. FIELD CONNECTIONS SHALL BE BOLTED UNLESS WELDING IS INDICATED. FIELD NTS WELDING OF STRUCTURAL MEMBERS IS NOT PERMITTED UNLESS SPECIFICALLY
- 5. SHOP CONNECTIONS MAY BE BOLTED OR WELDED UNLESS THE CONNECTION METHOD IS INDICATED.
- 6. PROVIDE ASTM F 593 TYPE 316 STAINLESS STEEL BOLTS FOR BOLTED CONNECTIONS. PROVIDE DRILLED HOLES 1/16" LARGER THAN THE BOLT DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE ASTM F 594 STAINLESS STEEL NUTS WITH STAINLESS STEEL WASHERS IN BOTH OUTER PLIES.
- 7. SIMPLY SUPPORTED BEAM-TO-COLUMN AND BEAM-TO-BEAM CONNECTIONS SHALL BE MADE WITH DOUBLE ANGLES IN CONFORMANCE WITH THE ALUMINUM ASSOCIATION DESIGN MANUAL UNLESS OTHERWISE INDICATED.
- 8. DETAIL BRACING MEMBERS TO AVOID ECCENTRIC CONNECTIONS.
- 9. PROVIDE TEMPORARY BRACING AND STAYS DURING ERECTION TO RESIST VERTICAL AND LATERAL LOADS UNTIL MEMBERS ARE PERMANENTLY FASTENED.
- 10. PAINT ALUMINUM IN CONTACT WITH CONCRETE WITH ASPHALTIC PAINT OR PROVIDE SHEET NEOPRENE BARRIER.
- 11. PROVIDE DISSIMILAR METAL PROTECTION AT LOCATIONS WHERE DISSIMILAR METALS ARE IN CONTACT. PROTECT WITH A MINIMUM 4-MIL DRY THICKNESS COAT OF ZINC CHROMATE PRIMER ON THE ALUMINUM SURFACES AND A MINIMUM 2-MIL DRY THICKNESS COAT OF ALL-METAL PRIMER FOLLOWED BY ONE COAT OF MINIMUM 3-MIL DRY THICKNESS ALUMINUM PAINT TO THE DISSIMILAR METAL.

12. USE NON-SHRINK, NON-METALLIC GROUT UNDER BASE AND BEARING PLATES.

ANCHOR BOLT

ABBREVIATIONS:

ADDITIONAL

ALTERNATE

ALUMINUM

ENGINEERS

CENTERLINE

COLUMN

CONCRETE

DEGREE

DIAMETER DIRECTION

DRAWING

EACH FACE

ELEVATION

EACH WAY

FLOOR DRAIN

EXISTING

FAR FACE

FINISH

FLOOR

FOOTING

HORIZONTAL

HIGH POINT

INSIDE FACE

LOW POINT

MECHANICAL

MANUFACTURER

NOT TO SCALE

OUTSIDE FACE

ON CENTER

PROJECTION

SPECIFICATIONS

SECTION

SQUARE

COUNCIL

STEEL

TREADS

TYPICAL

VERTICAL

STANDARD

STRUCTURAL

SYMMETRICAL

TOP OF CONCRETE

TOP OF STEEL

MAXIMUM

MINIMUM

OPENING

PIECE

EXPANSION JOINT

GALVANIZE (HOT-DIP)

LONG LEG HORIZONTAL

POUNDS PER CUBIC FEET

PREMOLDED JOINT FILLER

POUNDS PER SQUARE FEET

POUNDS PER SQUARE INCH

STEEL STRUCTURES PAINTING

UNLESS NOTED OTHERWISE

WATERSTOP, WATER SURFACE

DIRECTION IN WHICH BARS EXTEND

WELDED WIRE FABRIC

LONG LEG VERTICAL

DETAIL

DOWN

DOWEL

EACH

CONTINUOUS

BFAM

BOTTOM

CLEAR

ARCHITECTURAL

ASTM INTERNATIONAL

CONSTRUCTION JOINT

AMERICAN SOCIETY OF CIVIL

ADDL

ALUM

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HORIZ,

ВМ

CJ

ALT

PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation 10 Park Plaza, Suite 6620

Boston, MA 02116 617.626.1250 tel 617.626.1351 fax www.mass.gov/orgs/department-of-conservation-recreation

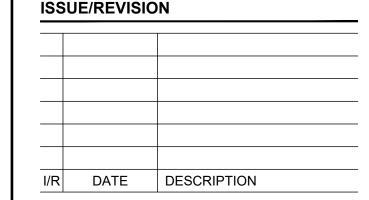
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REGISTRATION

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PROJECT NUMBER 60604936

Designed By:	J. PRIVITERA
Drawn By:	A. CATALANO
Dept Check:	M. MALENFANT
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

SHEET TITLE STRUCTURAL NOTES AND

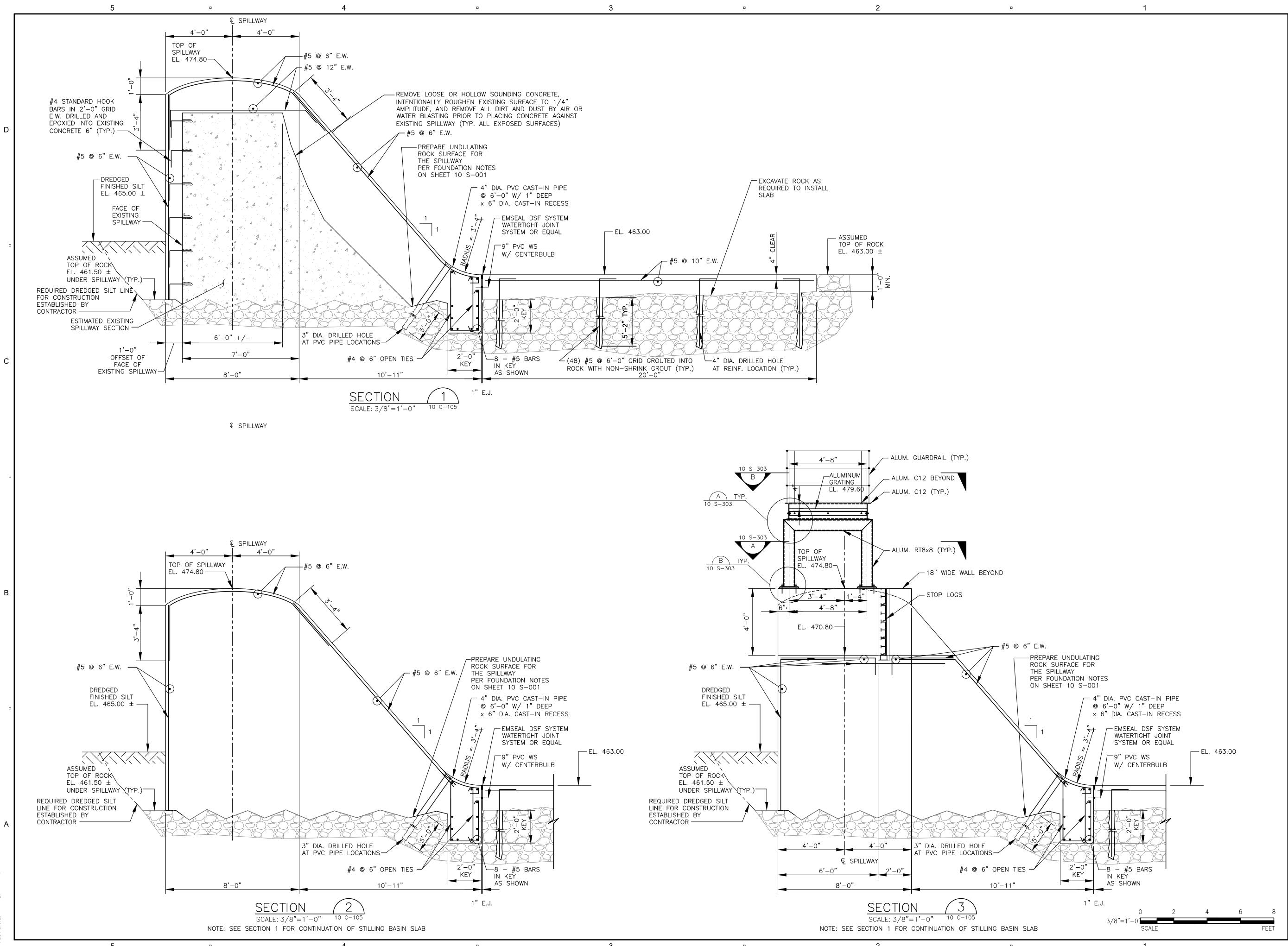
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STANDARD DETAILS

DISCIPLINE **STRUCTURAL**



PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620
Boston, MA 02116
617.626.1250 tel 617.626.1351 fax
www.mass.gov/orgs/department-of-conservation-recreation

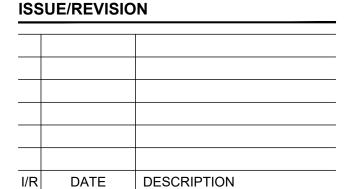
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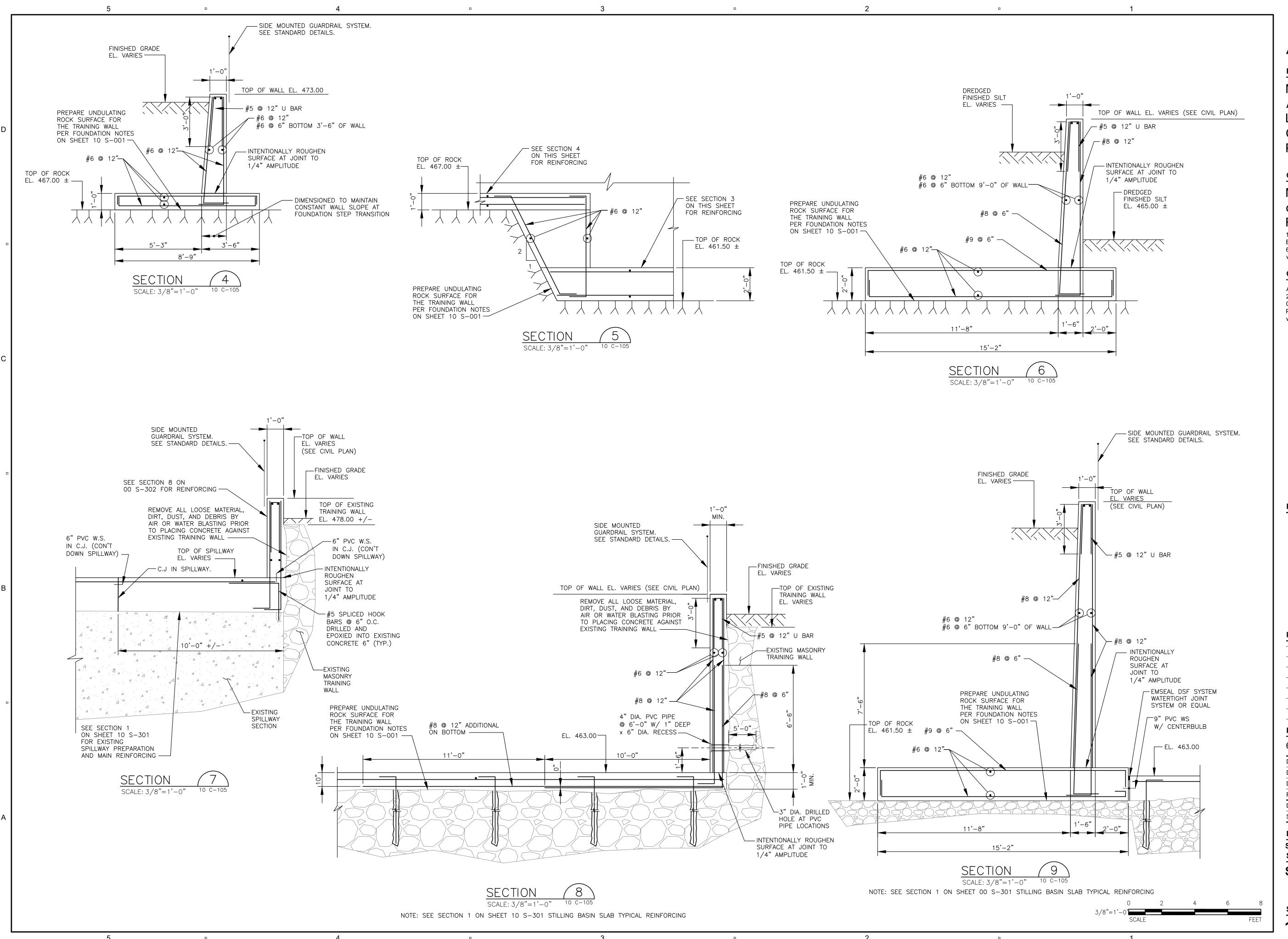
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DISCIPLINE

STRUCTURAL SHEET TITLE

SECTIONS I

SHEET NUMBER



PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

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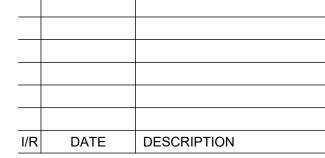
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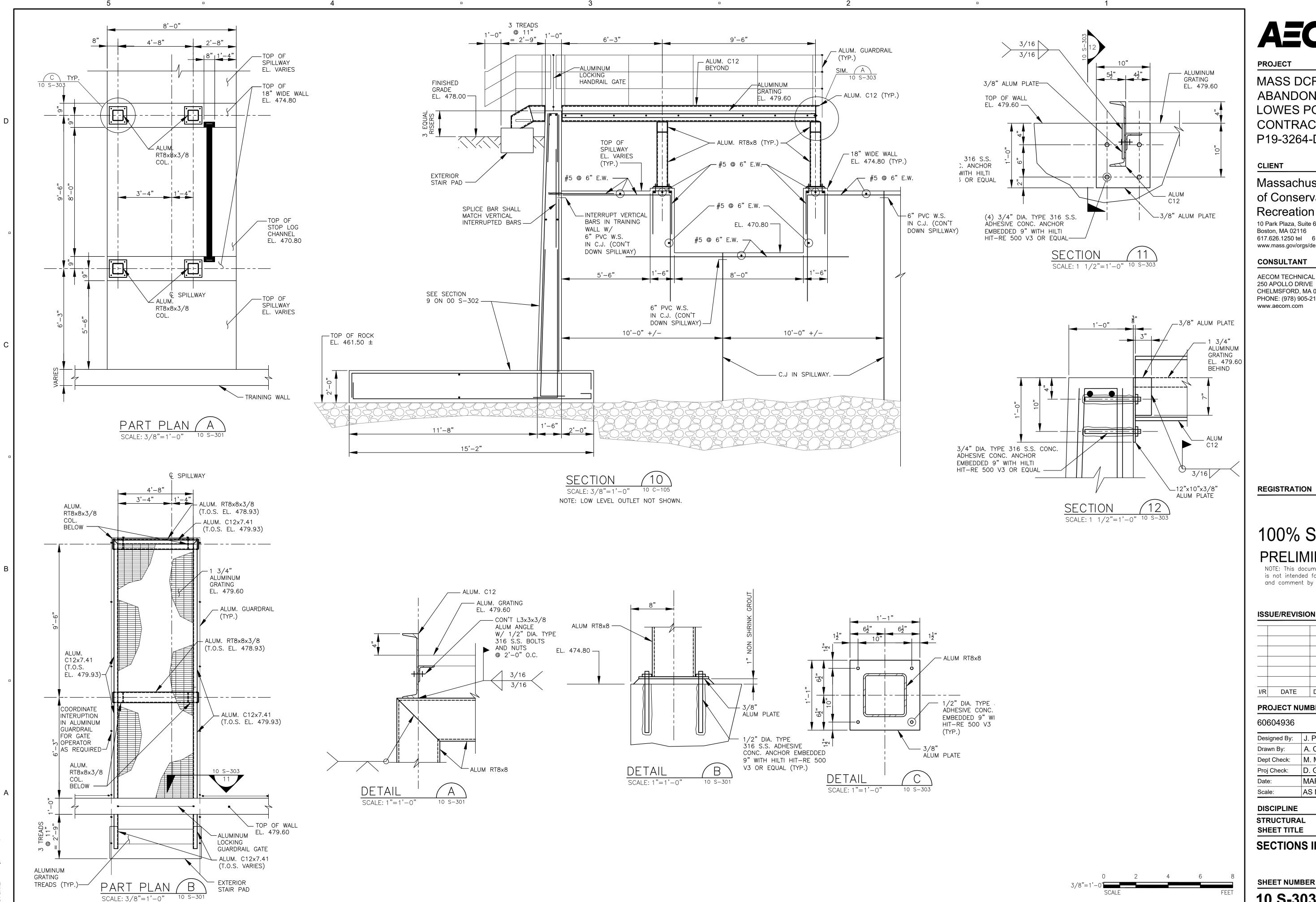
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DISCIPLINE

STRUCTURAL SHEET TITLE

SECTIONS II

SHEET NUMBER



PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620 Boston, MA 02116 617.626.1250 tel 617.626.1351 fax www.mass.gov/orgs/department-of-conservation-recreation

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I/R	DATE	DESCRIPTION

PROJECT NUMBER

60604936

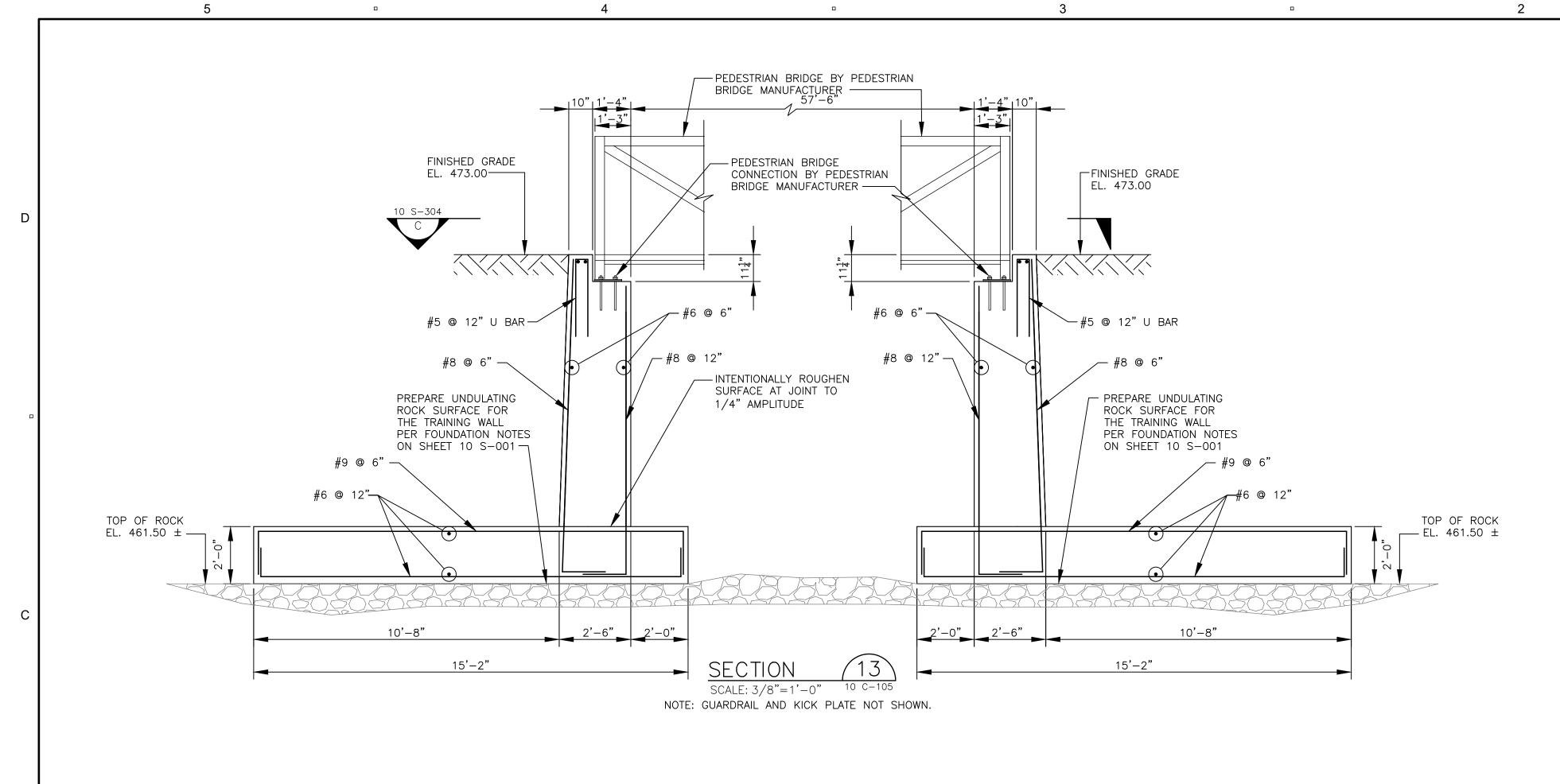
Designed By:	J. PRIVITERA
Drawn By:	A. CATALANO
Dept Check:	M. MALENFANT
Proj Check:	D. GOVE
Date:	MARCH 2024
Scale:	AS NOTED

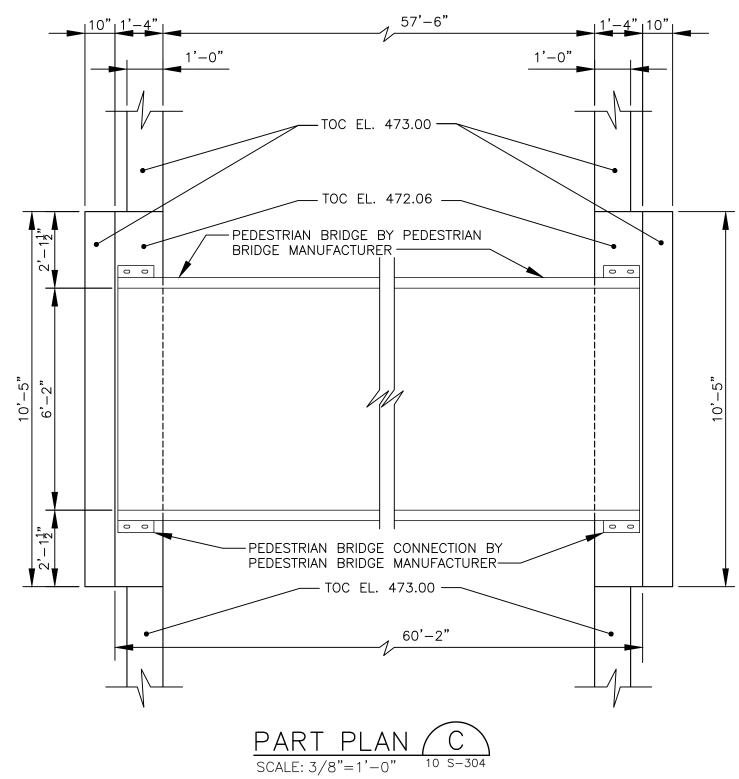
DISCIPLINE

STRUCTURAL SHEET TITLE

SECTIONS III

SHEET NUMBER





AECON

PROJECT

MASS DCR
ABANDONED DAMS
LOWES POND DAM
CONTRACT NO.
P19-3264-D4A

CLIENT

Massachusetts Department of Conservation and Recreation

10 Park Plaza, Suite 6620
Boston, MA 02116
617.626.1250 tel 617.626.1351 fax
www.mass.gov/orgs/department-of-conservation-recreation

CONSULTANT

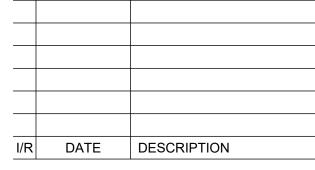
AECOM TECHNICAL SERVICES, INC. 250 APOLLO DRIVE CHELMSFORD, MA 01824 PHONE: (978) 905-2100 www.aecom.com

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STRUCTURAL SHEET TITLE

SECTIONS IV

SHEET NUMBER

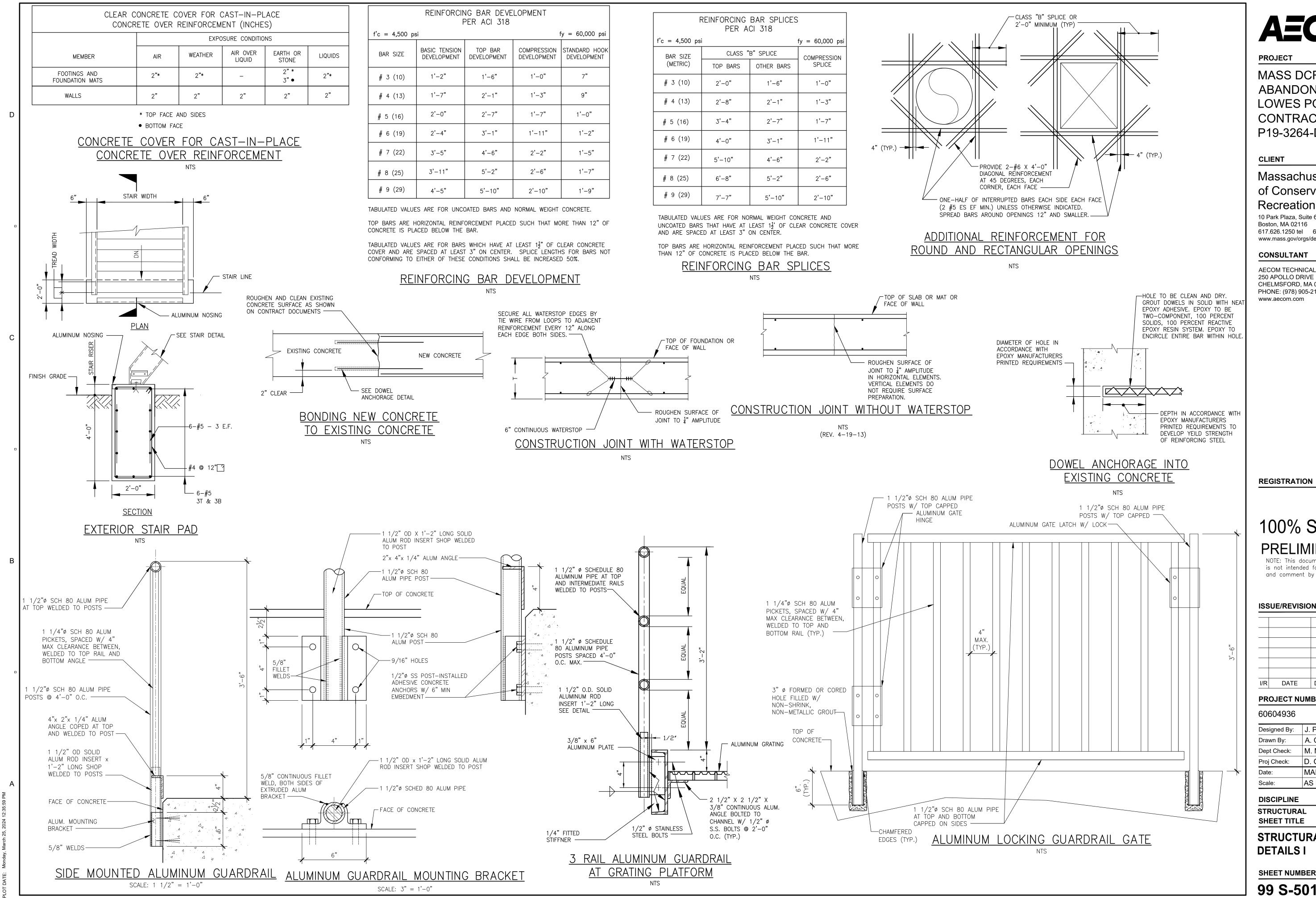
10 S-304

0 2 4 6 8 3/8"=1'-0" SCALE FEET

ATE: Monday, March 25, E: Monday, March 25, 20

SCALE SCALE

2



PROJECT

MASS DCR ABANDONED DAMS LOWES POND DAM CONTRACT NO. P19-3264-D4A

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STRUCTURAL SHEET TITLE

STRUCTURAL STANDARD **DETAILS I**

SHEET NUMBER

	SLIDE GATE SCHEDULE															
TAG NO	NUMBER OF UNITS	LOCATION	GATE SIZE (CLEAR OPENING) W(IN) x H(IN)	GATE TYPE	CONFIGURATION	HEAD TYPE	GATE DESIGN HEAD (FT)	INSTALLATION TYPE	THIMBLE TYPE	CLOSURE TYPE	OPERATOR TYPE	HP	RPM (MAX)	ENCL	SPEC SECTION	REMARKS
10-RW.SG-1	1	LOWES POND DAM	24 x 24	FABRICATED	STANDARD	SEATING	13	WALL MOUNTED	N/A	FLUSH BOTTOM	MANUAL CRANK WITH SHAFT EXTENSION TO CLEAR RAILING				15103	DESIGN HEAD: MAX WATER SURFACE TO GATE INVERT.

	STOP LOG SCHEDULE										
TAG NO	NUMBER OF UNITS	LOCATION	STOP LOG SIZE (CLEAR OPENING) W(IN) x H(IN)	SECTION HEIGHT(IN)	HEAD TYPE	DESIGN HEAD (FT) (MAX WATER TO BOT LOG INV)	INSTALLATION TYPE	CLOSURE TYPE	NUMBER OF LOGS	SPEC SECTION	REMARKS
10-RW.SL-1	1	LOWES POND DAM	96 x 108	6	SEATING	8	EMBEDDED	FLUSH BOTTOM	8	15112	GROOVES EMBEDDED 4 FT

PROCESS EQUIPMENT ABBREVIATIONS

SLIDE GATE STOP LOG

PROCESS GENERAL ABBREVIATIONS

ACFM	ACTUAL CUBIC FEET PER MINUTE		(FIXED x FIXED)	NO	NORMALLY OPEN	STR	STRUCTURAL
AFF	ABOVE FINISHED FLOOR	FE	FLOW ELEMENT	NO	NUMBER	TEFC	TOTALLY ENCLOSED FAN COOLED
ANCH	ANCHOR	FF	FLAT FACE	NOM	NOMINAL	TEMP	TEMPERATURE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	FHMB	FLAT HEAD MACHINE BOLT	NPSH	NET POSITIVE SUCTION HEAD	TENV	TOTALLY ENCLOSED NON VENTILATED
APPROX	APPROXIMATE	FIN FL	FINISHED FLOOR	NPSHR	NET POSITIVE HEAD REQUIRED	THK	THICK
ARCH	ARCHITECTURAL	FIN GR	FINISHED GRADE	NPSH3	NPSH RESULTING IN 3% DROP IN HEAD	TOC	TOP OF CONCRETE
ARV	AIR RELEASE VALVE	FIT	FLOW INDICATING TRANSMITTER	NPT	NATIONAL PIPE THREAD	TOD	TOP OF DUCT
AVV	AIR VACUUM VALVE	FLEX	FLEXIBLE	NRS	NON-RISING STEM	TOS	TOP OF STEEL
BOD	BOTTOM OF DUCT	FLG	FLANGE	NTS	NOT TO SCALE	TSH	TEMPERATURE SWITCH HIGH
CFM	CUBIC FEET PER MINUTE	FCV	FLOW CONTROL VALVE	OC	ON CENTER	TYP	TYPICAL
CFS	CUBIC FEET PER SECOND	FOB	FLAT ON BOTTOM	OD	OUTSIDE DIAMETER	VD	VOLUME DAMPER
CHEM	CHEMICAL	FOS	FLAT ON SIDE	ODP	OPEN DRIP PROOF	W/	WITH
CI	CAST IRON	FOT	FLAT ON TOP	OPER	OPERATING	WxH	WIDTH x HEIGHT
Ę.	CENTERLINE	FPD	FLOWER POT DISCHARGE	Р	PRESSURE GAUGE	WC	WATER COLUMN
CL	CLASS	FT/MIN	FEET PER MINUTE	PE	PLAIN END	WORK	WORKING
CMU	CONCRETE MASONARY UNIT	FT/S	FEET PER SECOND	PI	PRESSURE INDICATOR	WP	WEATHER PROTECTED
CO	CLEANOUT	FS	FLOW SWITCH	PIT	PRESSURE INDICATOR TRANSMITTER	WPI	WEATHER PROTECTED TYPE I
CONC	CONCRETE	FSI	FORMED SUCTION INTAKE	PLMB	PLUMBING	WPII	WEATHER PROTECTED TYPE II
CONN	CONNECTION	FT	FEET	PO	PUSH-ON	WS	WATER SURFACE
CONT	CONTINUATION	FW	FLUSHING WATER	PRI	PRIMARY	XPROOF	EXPLOSION PROOF
CPVC	CHLORINATED POLYVINYLCHLORIDE	GAL	GALLONS	PS	PRESSURE SWITCH	YD	YARD
CS	CARBON STEEL	GALV	GALVANIZED	PSH	PRESSURE SWITCH HIGH	GPM	GALLONS PER MINUTE
DEG C	DEGREES CELSIUS	LG	LONG	PSI	POUNDS PER SQUARE INCH	GCIC	GRIT CONCENTRATOR INFLUENT CHANNEL
DEG F	DEGREES FAHRENHEIT	LP	LOW POINT	PSIA	POUNDS PER SQUARE INCH ABSOLUTE	HDPE	HIGH DENSITY POLYETHYLEYNE
DET	DETAIL	LR	LONG RADIUS	PSIG	POUNDS PER SQUARE INCH GAGE	HG	HOSE GATE
DI	DUCTILE IRON	L/S	LITERS PER SECOND	PSV	PRESSURE SAFETY SWITCH	HHMB	HEXAGONAL HEAD MACHINE BOLT
DIA	DIAMETER	LSH	LEVEL SWITCH LOW	PVC	POLYVINYLCHLORIDE	Hg	MERCURY
DISCH	DISCHARGE	LSP	LIQUID SAMPLE PORT	PVDF	POLYVINYLIDENE FLOURIDE	HP	HIGH POINT
DN	DOWN	LxW	LENGTH X WIDTH	R	RADIUS	HP	HORSE POWER
DWG'S	DRAWINGS	LWL	LOW WATER LEVEL	RED	REDUCER / REDUCING	HPT	HOSE PIPE THREAD
ExE	FLEXIBLE COUPLING	LLWL	LOW-LOW WATER LEVEL	RED FLG	REDUCING FLANGE	HVAC	HEATING VENTILATION
LXL	(EXPANSION x EXPANSION)	M	METER	REF	REFERENCE	HWL	HIGH WATER LEVEL
ECC RED	ECCENTRIC REDUCER	MAX	MAXIMUM	REINF	REINFORCING	ID	INSIDE DIAMETER
EFF	EFFLUENT	MATL	MATERIAL	REQ'D	REQUIRED	IN	INCHES
EFF %	% EFFICIENCY	MCC	MOTOR CONTROL CENTER	RFG	RISING FLAP GATE	INSUL	INSULATION
ELEC	ELECTRICAL	MECH	MECHANICAL MECHANICAL	RPM	REVOLUTIONS PER MINUTE	INV	INVERT
EL	ELEVATION	MGD	MILLION GALLONS PER DAY	SCH	SCHEDULE	IPT	IRON PIPE THREAD
ELL	ELBOW	MH	MANHOLE	SH	SHEET	KG	KILOGRAM
ENCL	ENCLOSURE	MIN	MINIMUM	SPD	SUMP PUMP DISCHARGE	KW	KILOWATT
EQUIP	EQUIPMENT	MJ	MECHANICAL JOINT	SPECS	SPECIFICATIONS	LB	POUND
EXIST	EXISTING	MM	MILLIMETERS	SQ	SQUARE	LG	LONG
FxE	EXPANSION COUPLING	NAVD	NORTH AMERICAN VERTICAL DATUM	SR	SHORT RADIUS	20	LONG
1 AL	(FIXED x EXPANSION)	NC	NORMALLY CLOSED	SS	STAINLESS STEEL		
FxF	RESTRAINED COUPLING	NGVD	NATIONAL GEODETIC VERTICAL DATUM	STD	STANDARD		
1 /1	KEG IKANILD GOOF LING	NOVD	WATIONAL GEODETIO VERTICAL DATONI	310	CIANDAND		

PROJECT

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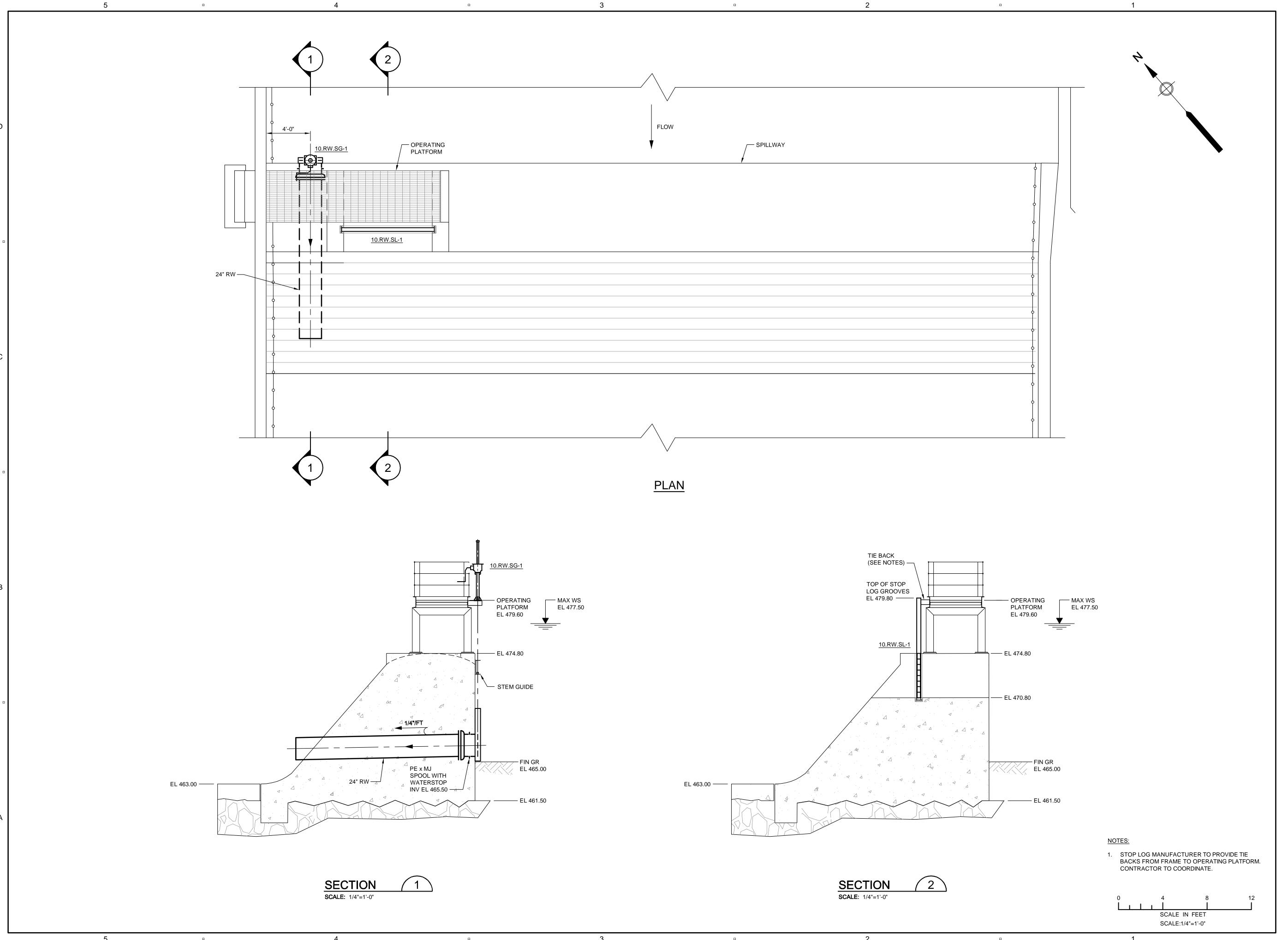
DISCIPLINE

MECHANICAL PROCESS SHEET TITLE

PROCESS SCHEDULES **AND ABBREVIATIONS**

SHEET NUMBER

00 D-001



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MASS DCR
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LOWES POND DAM
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MECHANICAL PROCESS
SHEET TITLE

PLAN AND SECTION

SHEET NUMBER

10 D-101