MASSACHUSETTS DEPARTMENT OF TRANSPORTATION **HIGHWAY DIVISION**

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PLAN AND PROFILE OF

KEYES HILL ROAD OVER UMPACHENE RIVER (BRIDGE NO. N-08-020 (CN3))

IN THE TOWN OF

NEW MARLBOROUGH **BERKSHIRE COUNTY**

FEDERAL AID PROJECT NO. BFL(BR-OFF)-003S(798)X





LENGTH OF PROJECT = 475 FEET = 0.090 MILES

NEW MARLBOROUGH KEYES HILL ROAD OVER UMPACHENE RIVER

STATE FED. AID PROJ. NO. MA BFL(BR-OFF)-003S(798)X PROJECT FILE NO. 609078

TITLE SHEET & INDEX

THESE PLANS ARE SUPPLEMENTED BY THE OCTOBER 2017 CONSTRUCTION STANDARD DETAILS. THE 2015 OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, THE 1990 STANDARD DRAWINGS FOR SIGNS AND SUPPORTS, THE 1968 STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING AND THE LATEST EDITION OF THE AMERICAN STANDARD FOR NURSERY STOCK

DESIGN DESIGNATION (KEYES HILL ROAD)

DESIGN SPEED ADT (2023) ADT (2043) K D T (PEAK HOUR) T (AVERAGE DAY) DHV DDHV FUNCTIONAL CLASSIFICATION

20 MPH 213 260 10.8% 86.7% 4.3% 10.8% 23 20 RURAL LOCAL



CHIEF ENGINEER

Kin Chung Lam Digitally signed by Kin Chung Lar Date: 2024.11.04 09:03:37 -05'00

REV #

DATE

LAMSON ENGINEERING CORPORATION NEWTON, MASSACHUSETTS

GENERAL SYMBO	LS		TRAFFIC SYMBOLS		
EXISTING	PROPOSED	DESCRIPTION	EXISTING	PROPOSED	DESCRIPTION
		JERSEY BARRIER CATCH BASIN	Ø 1	Ø1	CONTROLLER PHASE ACTUATED
		CATCH BASIN CURB INLET			TRAFFIC SIGNAL HEAD (SIZE AS NOTED)
© FP	♦ FP	FLAG POLE GAS PUMP		Ŏ	
		MAIL BOX			WIRE LOOP DETECTOR (6' x 6' TYP UNLESS OTHERWISE SPECIFIED)
		POST SQUARE		T	VIDEO DETECTION CAMERA
⊕ WELL	⊕ WELL	WELL			MICROWAVE DETECTOR
□ EHH	□ EHH	ELECTRIC HANDHOLE	\oplus	•	PEDESTRIAN PUSH BUTTON, SIGN (DIRECTIONAL ARROW AS SHOWN) AND SADDLE
0 66	O O GG	FENCE GATE POST GAS GATE	*	*	EMERGENCY PREEMPTION CONFIRMATION STROBE LIGHT
● BHL #	 BHL # 	BORING HOLE	<──	-	VEHICULAR SIGNAL HEAD
↔ MW #	- ф МW# п тр#	MONITORING WELL	≪	←	VEHICULAR SIGNAL HEAD, OPTICALLY PROGRAMMED
\sim	• · · · <i>"</i>	HYDRANT	<		
*	*				PEDESTRIAN SIGNAL HEAD, (TYPE AS NOTED OR AS SPECIFIED)
$\bigcirc \diamond$		GPS POINT	RRSG	RRSG	
C	©	CABLE MANHOLE	-O- OR O	•	SIGNAL POST AND BASE (ALPHA-NUMERIC DESIGNATION NOTED)
D (E)	(D) (E)	DRAINAGE MANHOLE ELECTRIC, MANHOLE		• <u>20</u>	MASTARM, SHAFTAND BASE (ARM LENGTH AS NOTED)
G	©	GAS MANHOLE			HIGH MAST POLE OR TOWER
(M)	()	MISC MANHOLE	0	0	
(C) (T)	I I	TELEPHONE MANHOLE	$\overline{0}$	○ ○ ↓ ²⁰ →	
Ŵ	W				
MHB MON	■ MHB	MASSACHUSETTS HIGHWAY BOUND MONUMENT			
□ SB		STONE BOUND			
■ TB		TOWN OR CITY BOUND			
-• TPL or GUY	- TPL or GUY	TROLLEY POLE OR GUY POLE			
• HTP		TRANSMISSION POLE			
-o- UPDL	_&_ UFВ -∲- UPDL	UTILITY POLE WITH DOUBLE LIGHT			FUEL BOX 12 X12 (OR AS NOTED) ELECTRIC HANDHOLE 12"x24" (OR AS NOTED)
-δ- ULT	_& ULT	UTILITY POLE W / 1 LIGHT			- TRAFFIC SIGNAL CONDUIT
UPL	-o- UPL	UTILITY POLE BUSH			
•SIZE & TYPE		TREE			
O		STUMP SWAMP / MARSH			
• WG	• WG	WATER GATE			
• PM	• PM	PARKING METER			
		= CURBING			
<u> </u>		– CONTOURS (ON-THE-GROUND SURVEY DATA) – CONTOURS (PHOTOGRAMMETRIC DATA)			
		– UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER)			
		- UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER)			
		– UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER) – UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER)	EXISTING	PROPOSED	DESCRIPTION
		- UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER)		•	PAVEMENT ARROW - WHITE
		 UNDERGROUND WATER MAIN (DOUBLE LINE 24 INCH AND OVER) BALANCED STONE WALL 	ONLY	ONLY	LEGEND "ONLY" - WHITE
		- GUARD RAIL - STEEL POSTS			STOP LINE
		– GUARD RAIL - WOOD POSTS – GUARD RAIL - DOUBLE FACE - STEEL POSTS		<u>cw</u>	CROSSWALK
		- GUARD RAIL - DOUBLE FACE - WOOD POSTS		SWL	SOLID WHITE LINE
		- CHAIN LINK OR METAL FENCE		<u> </u>	SOLID YELLOW LINE
	u			BWL	
				BYL	
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		- SAWCUT LINE			
		- TOP OR BOTTOM OF SLOPE			
		- LIMIT OF EDGE OF PAVEMENT OR COLD PLANE AND OVERLAY BANK OF RIVER OR STREAM			
		BORDER OF WETLAND			
		100 FT WETLAND BUFFER			
		- STATE HIGHWAY LAYOUT			
		- COUNTY LAYOUT - RAILROAD SIDELINE			
		TOWN OR CITY BOUNDARY LINE			
		PROPERTY LINE OR APPROXIMATE PROPERTY LINE – EASEMENT			
			NOT ALL SYMBOLS	OR ABBREVIATIONS	ARE NEEDED ON THESE PLANS.

RAFFIC SYMBOLS			ABBREVIATIC	DNS		
EXISTING	PROPOSED	DESCRIPTION	GENERAL		- KFY	NEW MARLBOROUGH
<i>Ø</i> 1	<i>Ø</i> 1	CONTROLLER PHASE ACTUATED	AADT	ANNUAL AVERAGE DAILY TRAFFIC		
6			ABAN	ABANDON		STATE FED. AID PROJ. NO. NO. SHEETS
	Ŏ	TRAFFIC SIGNAL HEAD (SIZE AS NOTED)	ADJ APPROX.	APPROXIMATE		
			A.C.	ASPHALT CONCRETE		PROJECT FILE NO. 609078
		WIRE LOOP DETECTOR (6' x 6' TYP UNLESS OTHERWISE SPECIFIED)	ACCM PIPE	ASPHALT COATED CORRUGATED METAL PIPE		LEGEND AND ABBREVIATIONS
~	T	VIDEO DETECTION CAMERA	BIT.	BITUMINOUS		
		MICROWAVE DETECTOR	BC			
$\oplus$	•	PEDESTRIAN PUSH BUTTON, SIGN (DIRECTIONAL ARROW AS SHOWN) AND SADDLE	BD. BI	BOOND BASELINE		
	-		BLDG	BUILDING	ABBREVIAT	IONS (cont.)
*	*	EMERGENCY PREEMPTION CONFIRMATION STROBE LIGHT	BM	BENCHMARK	<u>GENERAL</u>	
<──	◄—	VEHICULAR SIGNAL HEAD	BO	BY OTHERS	PVC	POINT OF VERTICAL CURVATURE
≪	₩—	VEHICULAR SIGNAL HEAD, OPTICALLY PROGRAMMED	BOS	BOTTOM OF SLOPE	PVI	POINT OF VERTICAL INTERSECTION
<	<b>—</b>	FLASHING BEACON	BR.	BRIDGE	PVT	
	_		CBC			PAVEMENT PAVED WATER WAY
		PEDESTRIAN SIGNAL HEAD, (TYPE AS NOTED OR AS SPECIFIED)	CC	CEMENT CONCRETE	R	RADIUS OF CURVATURE
RRSG	X RRSG	RAILROAD SIGNAL	CCM	CEMENT CONCRETE MASONRY	R&D	REMOVE AND DISPOSE
	•	SIGNAL POST AND BASE (ALPHA-NUMERIC DESIGNATION NOTED)	CEM	CEMENT	RCP	REINFORCED CONCRETE PIPE
°O	20'	MAST ARM. SHAFT AND BASE (ARM LENGTH AS NOTED)	CI	CURB INLET	RD	ROAD
			CIP	CAST IRON PIPE	RDWY	ROADWAY
			CLF		KEM Ret	KEMUVE RETAIN
0		SIGN AND POST			RFT WΔ11	
$\overline{\mathbf{O}}$	00	SIGN AND POST (2 POSTS)	CIVIE		ROW	RIGHT OF WAY
	20'	MAST ARM WITH LUMINAIRE	CO	COUNTY	RR	RAILROAD
	<u>т</u> -		CONC	CONCRETE	R&R	REMOVE AND RESET
	■	OPTICAL PRE-EMPTION DETECTOR	CONT	CONTINUOUS	R&S	REMOVE AND STACK
	$\boxtimes$	CONTROL CABINET, GROUND MOUNTED	CONST	CONSTRUCTION	RT	RIGHT
		CONTROL CABINET, POLE MOUNTED	CR GR	CROWN GRADE	SB SB	
			DHV	DESIGN HOURLY VOLUME	SMH	STUULDER SEWER MANHOI F
Mari		FLASHING BEACON CONTROL AND METER PEDESTAL	DI		ST	STREET
$\bowtie$	$\boxtimes$	LOAD CENTER ASSEMBLY			STA	STATION
		PULL BOX 12"x12" (OR AS NOTED)	DW	STEADY DON'T WALK - PORTLAND ORANGE	SSD	STOPPING SIGHT DISTANCE
		ELECTRIC HANDHOLE 12"x24" (OR AS NOTED)	DWY	DRIVEWAY	SHLO	STATE HIGHWAY LAYOUT LINE
			ELEV (or EL.)	ELEVATION	SW	SIDEWALK
		= TRAFFIC SIGNAL CONDULT	EMB	EMBANKMENT		TANGENT DISTANCE OF CURVE/TRUCK
			EOP	EDGE OF PAVEMENT		
			EXIST (or EX)	EXISTING	TC	TOP OF CURB
			EXC		TOS	TOP OF SLOPE
			F&C		TYP	TYPICAL
			FDN	FOUNDATION	UP	UTILITY POLE
			FLDSTN	FIELDSTONE	VAR	VARIES
			GAR	GARAGE	VERT	
			GD	GROUND	VC WG	WATER GATE
			GG	GAS GATE	WIP	WROUGHT IRON PIPE
MENT MARKING	GS SYMBOLS		GI		WM	WATER METER/WATER MAIN
			GIP	GALVANIZED IRON PIPE	X-SECT	CROSS SECTION
<u>EXISTING</u>	PROPOSED		GRAN			
		DESCRIPTION	GRAV	GRAVEL		
	<b>•</b> ]	DESCRIPTION PAVEMENT ARROW - WHITE	GRAV GRD	GRAVEL GUARD		
ONLY	<b>€</b> ∩ ONLY	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE	GRAV GRD HDW	GRAVEL GUARD HEADWALL		
ONLY	<b>€</b> ∭Y sl	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE	GRAV GRD HDW HMA	GRAVEL GUARD HEADWALL HOT MIX ASPHALT		
ONLY	<b>↑</b> ∭Y S∟	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE	GRAV GRD HDW HMA HOR	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL	TRAFFIC SIG	GNAL ABBREVIATIONS
ONLY	<b>↑</b> ∭Y S∟         cw	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK	GRAV GRD HDW HMA HOR HYD	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT	TRAFFIC SIC	GNAL ABBREVIATIONS CABINET
ONLY	↑   NY  SL   SWL	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK SOLID WHITE LINE	GRAV GRD HDW HMA HOR HYD INV	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT	TRAFFIC SIC CAB CCVE	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT
ONLY		DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK SOLID WHITE LINE SOLID YELLOW LINE	GRAV GRD HDW HMA HOR HYD INV JCT	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION	TRAFFIC SIC CAB CCVE DW	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND
ONLY	↑	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK SOLID WHITE LINE SOLID YELLOW LINE REOKEN WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE	TRAFFIC SIC CAB CCVE DW FDW	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND
ONLY	↑   NUY   SL   SWL   SYL  BWL	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK SOLID WHITE LINE SOLID YELLOW LINE BROKEN WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE	TRAFFIC SIG CAB CCVE DW FDW FR FR	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEET ARROW/
ONLY		DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         BROKEN YELLOW LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRL FRR	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW
ONLY		DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRL FRR FY	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING CIRCULAR YELLOW
	↑       SL       SL       SVL       SYL       BWL       BYL       DWL       DYL	DESCRIPTION PAVEMENT ARROW - WHITE LEGEND "ONLY" - WHITE STOP LINE CROSSWALK SOLID WHITE LINE SOLID YELLOW LINE BROKEN WHITE LINE DOTTED WHITE LINE DOTTED WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRL FRR FY FYL	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING CIRCULAR YELLOW FLASHING YELLOW LEFT ARROW
		DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE         DOTTED WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FRL FRR FY FYL FYL	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING CIRCULAR YELLOW FLASHING YELLOW LEFT ARROW
	<ul> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>BWL</li> <li>BYL</li> <li>BYL</li> <li>DWL</li> <li>DVL</li> <li>DVL</li> <li>DVL</li> </ul>	DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE         DOTTED WHITE LINE         DOTTED WHITE LINE EXTENSION	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH MHB	GRAVEL GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FRL FRR FY FYL FYR G	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING CIRCULAR YELLOW FLASHING YELLOW LEFT ARROW STEADY CIRCULAR GREEN
	<ul> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>BWL</li> <li>BYL</li> <li>BYL</li> <li>BYL</li> <li>DWL</li> <li>DYL</li> <li>DYL</li> <li>DYLEx</li> </ul>	DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE EXTENSION	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH MHB MHB MIN	GRAVEL GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND MINIMUM MILL & OVERLAX	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FRL FRR FY FYL FYR G GL	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING RED RIGHT ARROW FLASHING YELLOW LEFT ARROW FLASHING YELLOW RIGHT ARROW STEADY CIRCULAR GREEN STEADY CREEN LEFT ARROW
	<ul> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>BWL</li> <li>▲</li> <li>BYL</li> <li>BYL</li></ul>	DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         BROKEN YELLOW LINE         DOTTED WHITE LINE         DOTTED WHITE LINE         DOTTED WHITE LINE EXTENSION         DOUBLE WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH MHB MH MHB MIN MNN M&O NIC	GRAVEL GRAVEL GUARD HEADWALL HOT MIX ASPHALT HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND MINIMUM MILL & OVERLAY NOT IN CONTRACT	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FRL FRR FYL FYL FYR G GL GR GSI	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING YELLOW LEFT ARROW FLASHING YELLOW RIGHT ARROW STEADY GREEN LEFT ARROW STEADY GREEN RIGHT ARROW
	<ul> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>BWL</li> <li>▲</li> <li>BYL</li> </ul>	DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE EXTENSION         DOUBLE WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH MHB MH MHB MIN MAN MIN MAN MIN MAN MIN MAN	GRAVEL GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND MINIMUM MILL & OVERLAY NOT IN CONTRACT NUMBER	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FYL FYR G GL GR GSL GSR	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING CIRCULAR YELLOW FLASHING YELLOW LEFT ARROW FLASHING YELLOW RIGHT ARROW STEADY GREEN LEFT ARROW STEADY GREEN RIGHT ARROW
ONLY	<ul> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>▲</li> <li>B</li> <li>B</li> <li>B</li> <li>▲</li> <li>B</li> <li>▲</li> <li>B</li> <li>▲</li> <li>B</li> <li>▲</li> <li>B</li> <li>▲</li> <li>B</li> <li>▲</li> <li>८</li> <li>८</li></ul>	DESCRIPTION         PAVEMENT ARROW - WHITE         LEGEND "ONLY" - WHITE         STOP LINE         CROSSWALK         SOLID WHITE LINE         SOLID YELLOW LINE         BROKEN WHITE LINE         DOTTED WHITE LINE         DOTTED WHITE LINE EXTENSION         DOTTED YELLOW LINE EXTENSION         DOUBLE WHITE LINE	GRAV GRD HDW HMA HOR HYD INV JCT L LB LP LT MAX MB MH MHB MH MHB MIN MHB MIN MA MHB MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME MIN ME ME MIN ME ME ME ME ME ME ME ME ME ME ME ME ME	GRAVEL GUARD HEADWALL HOT MIX ASPHALT HORIZONTAL HYDRANT INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND MINIMUM MILL & OVERLAY NOT IN CONTRACT NUMBER POINT OF CURVATURE	TRAFFIC SIG CAB CCVE DW FDW FR FRL FRR FY FYL FYR G GL GR GSL GSR GV	GNAL ABBREVIATIONS CABINET CLOSED CIRCUIT VIDEO EQUIPMENT STEADY UPRAISED HAND FLASHING UPRAISED HAND FLASHING CIRCULAR RED FLASHING RED LEFT ARROW FLASHING RED RIGHT ARROW FLASHING VELLOW LEFT ARROW FLASHING YELLOW LEFT ARROW STEADY GREEN LEFT ARROW STEADY GREEN RIGHT ARROW STEADY GREEN SLASH LEFT ARROW STEADY GREEN SLASH RIGHT ARROW
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		6.
1.	CONDUCTED BY WSP SURVEYORS FROM MAY 19, 2020 THROUGH JUNE 5, 2020.	
2.	NORTH ORIENTATION AND BEARING BASE PER GRID NORTH.	
÷.	THE LOCATION OF UNDERGROUND IMPROVEMENTS, UTILITIES OR ENCROACHMENTS, IF ANY EXIST, OR AS SHOWN HEREON, ARE NOT CERTIFIED. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF A UTILITY MARK OUT SERVICE. THE LOCATION OF UNDERGROUND UTILITIES DEPICTED ON THIS PLAN HAVE BEEN COMPILED FROM VARIOUS SOURCES. INCLUDING, BUT	7.
	NOT LIMITED TO INFORMATION AND RECORD PLANS OBTAINED FROM VARIOUS UTILITY PROVIDERS, AND LOCATION OF STRUCTURES VISUALLY IDENTIFIED AND LOCATED DURING THE COURSE OF THE FIELD SURVEY. THE LOCATION OF ALL UTILITIES DEPICTED ON THIS PLAN SHALL BE CONSIDERED APPROXIMATE. WSP MAKES NO WARRANTY NOR GUARANTEE	8.
	AS TO THE ACCURACY OF THE LOCATION OF THE UTILITY LINES DEPICTED ON THIS PLAT. FURTHERMORE, WSP MAKES NO WARRANTY NOR GUARANTEE THAT THE UTILITIES DEPICTED ON THIS MAP COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE ROUTE, SIZE AND LOCATION OF ALL UTILITIES MUST BE VERIFIED BY THE APPROPRIATE AUTHORITIES. THE PROPER UNDERGROUND FACILITIES PROTECTIVE	DRAII DRAIN THE C
	ORGANIZATION SHALL BE NOTIFIED, AND A UTILITY MARKOUT SERVICE DEPLOYED PRIOR TO CONDUCTING EXCAVATIONS AND CONSTRUCTION.	MAY C WILL E
4.	THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN UP TO DATE ABSTRACT OF TITLE.	STRU
5.	HORIZONTAL COORDINATES RELATIVE TO THE MASSACHUSETTS STATE PLANE COORDINATE SYSTEM, MASSACHUSETTS MAINLAND ZONE. BASED ON THE NORTH AMERICAN DATUM OF 1983, NAD83 (2011) EPOCH 2010.00.	PAID U
6.	ELEVATIONS RELATIVE TO THE NORTH AMERICAN VERTICAL DATUM OF 1988, NAVD 1988.	CONS
о. 7	HORIZONTAL COORDINATE AND ELEVATION UNITS ARE U.S. SURVEY FEET	1
י. 8	NO EXISTING RECORD LAYOUT FOR THE PORTION OF KEYES HILL ROAD DEPICTED HEREON	1.
	FIELD RECONNAISSANCE SURVEY WAS UNABLE TO RECOVER ANY EXISTING MONUMENTS TO ESTABLISH THE LOCATION LINES OF KEYES HILL ROAD. THE LOCATION LINES OF KEYES HILL ROAD DEPICTED HEREON HAVE BEEN ESTABLISHED PURSUANT TO G.M.L. TITLE XIV, CHAPTER 86, SECTION 2 AND IN ACCORDANCE WITH EXISTING OCCUPATION AS EVIDENCED BY PORTIONS OF ESTABLISHED FENCE LINES FRONTING UPON KEYES HILL ROAD IN CONJUNCTION WITH THE EXISTING LOCATION OF BRIDGE NO. N-08-020 OVER THE UMPACHENE	2.
	RIVER.	3.
9.	NO EXISTING RECORD LAYOUT FOR THE PORTION OF CANAAN-SOUTHFIELD ROAD DEPICTED HEREON. THE SOUTHERLY LOCATION LINE OF CANAAN-SOUTHFIELD ROAD DEPICTED HEREON ESTABLISHED PER MONUMENTATION FOUND IN ACCORDANCE WITH "SURVEY OF LAND IN NEW MARLBOROUGH, MASSACHUSETTS PREPARED FOR ROBIN RICHARDSON" FILED WITH THE REGISTRY OF DEEDS ON SEPTEMBER 9, 2011 IN PLAT FILE P, PAGE 183. THE NORTHERLY	
	LOCATION LINE OF CANAAN-SOUTHFIELD ROAD DEPICTED HEREON ESTABLISHED PER BEST FIT ALIGNMENT OF THE CENTERLINE OF THE EXISTING TRAVELED WAY OFFSET IN A NORTHEASTERLY DIRECTION 24.75 FEET (1.5 RODS).	4.
10.	BOUNDARIES OF PROPERTIES ABUTTING THE PUBLIC ROADS DEPICTED HEREON ESTABLISHED PER DEEDS AND/OR PLANS OF RECORD AND ARE NOT NECESSARILY THE RESULT OF A COMPLETE BOUNDARY OF SURVEY OF SAID ABUTTING PROPERTIES.	5.
11.	AN EXISTING PAIR OF GPS CONTROL POINTS NUMBERS 2555 AND 2556 WAS SET BY MASSDOT. WSP OCCUPIED THE EXISTING GPS PT ON KEYES HILL ROAD AND SET STATION 1 ON MILL STREET. A CLOSED LOOP TRAVERSE WAS RAN WITH STATION 1 AS THE INITIAL BACKSIGHT AND STATION 1993 AS THE INITIAL SETUP POINT. THE AZIMUTH BETWEEN STATIONS 1993 AND 1994 WAS HELD DURING THE TRAVERSE AD JUSTMENT TO POSITION THE MAIN CONTROL LOOP	6.
	ALL CONTROL WAS THREE-WIRE LEVELED AND ADJUSTED IN REFERENCE TO THE PROVIDED ELEVATION OF STATION 1993.	7.
12.	NOTE THAT THE SECTION OF THE UMPACHENE RIVER DEPICTED HEREON WAS SURVEYED PURSUANT TO CHAPTER 1.1.5 OF THE 2013 LFRD BRIDGE MANUAL (REVISED AUGUST 2020). PORTIONS OF THE THREE-DIMENSIONAL SURFACE ALONG THE RIVER HAVE BEEN DEVELOPED	TRAF
	USING CROSS-SECTIONAL SURVEY DATA COLLECTED IN THE FIELD AT INTERVALS AND/OR SUPPLEMENTED WITH STATE LIDAR DATA; AND NOT NECESSARILY FROM A COMPLETE DETAILED GROUND SURVEY OF THE ENTIRE AREA REQUIRED TO FACILITATE A HYDRAULIC	1.
	ANALYSIS. AS SUCH, PORTIONS OF THE TOPOGRAPHIC INFORMATION DEPICTED ON THIS PLAN SET AS DEMARCATED AND LABELED HEREON WERE DEVELOPED AT A LOWER VERTICAL ACCURACY THAN THE PORTIONS ALONG THE ROADS AND DIRECTLY ADJACENT TO BRIDGE	
	ACCURACY ARE INTENDED FOR THE EXPRESS PURPOSE OF CONDUCTING HYDRAULIC ANALYSIS ONLY.	TEMF
13.	THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPLACING ANY HIGHWAY BOUND OR PRIVATE PROPERTY PIN THAT MAY BE DAMAGED OR DESTROYED DURING CONSTRUCTION, TO ITS LOCATION JUST PRIOR TO CONSTRUCTION.	1.
	UTILITIES	2.
1.	ALL UNDERGROUND UTILITIES AS SHOWN WERE COMPILED USING FIELD SURVEY INFORMATION AND AVAILABLE RECORD INFORMATION.	
2.	RECORD UTILITY INFORMATION FROM THE VARIOUS UTILITY COMPANIES AND PUBLIC AGENCIES, ARE APPROXIMATELY ONLY AND ACTUAL LOCATIONS MUST BE DETERMINED IN THE FIELD.	3.
3.	WHERE AN EXISTING UTILITY IS FOUND TO CONFLICT WITH THE PROPOSED WORK, THE LOCATION, ELEVATION, AND SIZE OF THE UTILITY SHALL BE ACCURATELY DETERMINED WITHOUT DELAY BY THE CONTRACTOR AND THE INFORMATION FURNISHED TO THE ENGINEER FOR RESOLUTION OF THE CONFLICT.	
4.	CONTRACTOR SHALL MAKE ALL ARRANGEMENTS FOR THE ALTERATION AND ADJUSTMENT OF ELECTRIC, TELEPHONE, AND ANY OTHER PRIVATE UTILITIES BY THE UTILITY COMPANIES AT NO ADDITIONAL COST TO THE OWNER. IF THE CONTRACTOR ADJUSTS UTILITY COVERS, IT SHALL BE DEEMED PART OF THE WORK AND THERE WILL BE NO ADDITIONAL COMPENSATION.	
5.	ALL UTILITY COMPANIES, PUBLIC AND PRIVATE MUST BE NOTIFIED, INCLUDING THOSE IN CONTROL OF UTILITIES NOT SHOWN ON THIS PLAN, (SEE CHAPTER 370, ACTS OF 1963, MASSACHUSETTS) PRIOR TO DESIGNING, EXCAVATING, BLASTING, INSTALLING, BACKFILLING,	

### TIES (CONT')

- SUBSURFACE UTILITY LOCATIONS HAVE BEEN PLOTTED TO MEET UTILITY QUALITY LEVEL "C" AS DESCRIBED IN ASCE STANDARD 38-02 AND SUMMARIZED BELOW. THE UNDERGROUND UTILITIES ARE SHOWN IN APPROXIMATE LOCATIONS BASED ON ABOVE-GROUND FIELD OBSERVATION AND EXISTING RECORD INFORMATION RECEIVED FROM UTILITY STAKE-HOLDERS.
- THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE NECESSITY OF MAKING HIS/HER OWN INVESTIGATION IN ORDER TO ASSURE THAT NO DAMAGE TO EXISTING STRUCTURES. DRAINAGE LINES, TRAFFIC SIGNAL CONDUITS, ETCETERA, WILL OCCUR.
- NO EXISTING PUBLIC UTILITY STRUCTURES SHALL BE ABANDONED AND/OR DISMANTLED WITHOUT AUTHORIZATION FROM THE ENGINEER

#### INAGE:

NAGE ELEVATIONS ARE PROVIDED FOR DESIGN PURPOSES ONLY

CONTRACTOR SHALL VERIFY BY TEST PIT, THE LOCATIONS OF EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED DRAINAGE DESIGN. ANY FIELD ADJUSTMENTS REQUIRED BE MADE AS APPROVED OR DIRECTED BY THE ENGINEER. ONLY AFTER THE CONTRACTOR IES ELEVATIONS FOR THE CONSTRUCTABILITY OF THE DRAINAGE SYSTEM SHALL ANY ICTURES BE ORDERED. ANY FIELD ADJUSTMENTS TO LINE & GRADE UP TO A DEPTH OF 5' _ BE INCLUDED IN THE COST OF THE PIPE. PIPE EXCAVATION GREATER THAN 5' WILL BE JNDER CLASS B TRENCH EXCAVATION.

#### STRUCTION

- AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DAMAGED BY THE CONTRACTOR'S OPERATIONS, INCLUDING STAGING AREAS, SHALL BE RESTORED BY THE CONTRACTOR TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
- THE CONTRACTOR IS HEREBY NOTIFIED THAT ADDITIONAL WORK WITHIN THE PROJECT LIMITS MAY BE PERFORMED BY OTHERS. THE CONTRACTOR SHALL MAKE EVERY EFFORT TO COORDINATE WITH ANY SUCH WORK. NO ADDITIONAL COMPENSATION WILL BE MADE FOR EXTRA WORK DAYS, DELAYS, OR RESCHEDULING OF WORK TO ACCOMMODATE ANY OTHER CONSTRUCTION. PERMIT AND/OR MAINTENANCE OPERATIONS IN THE AREA.
- WHERE THE NEW CONSTRUCTION IS WITHIN THE EXISTING TRAVELED WAY, THE CONTRACTOR SHALL PERFORM WORK SO THAT INTERFERENCE TO BUSINESS CONCERNS AND ABUTTERS, ON ACCOUNT OF THE CONSTRUCTION WORK, IS KEPT TO A MINIMUM. THE CONTRACTOR WILL NOT BE ALLOWED TO PARK EQUIPMENT, OR STOCKPILE MATERIAL ON THE TRAVELED WAYS OVERNIGHT OR WHEN NOT IN USE. THE CONTRACTOR SHALL MAINTAIN SAFE AND REASONABLE ACCESS TO AND FROM ABUTTING PROPERTIES AT ALL TIMES AT NO ADDITIONAL COST.
- THE CONTRACTOR SHALL DISPOSE OF ALL WASTE MATERIAL IN ACCORDANCE WITH ALL FEDERAL, STATE AND LOCAL REGULATIONS AT HIS OWN EXPENSE, OUTSIDE OF THE PROJECT LIMITS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR INVESTIGATING AND CONFIRMING THAT ALL ITEMS TO BE REUSED ARE IN SERVICEABLE CONDITION. IF IT IS DEEMED THAT ANY ITEM IS NOT ABLE TO BE REUSED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING AND INCLUDE ESTIMATED COSTS TO INSTALL NEW.
- CONSTRUCTION MATERIALS AND EQUIPMENT SHOULD NOT BE STORED ON THE RIVERBED, AND THAT SEDIMENT, TURBIDITY, AND EROSION CONTROLS BE REQUIRED.
- IN-WATER CONSTRUCTION IS LIMITED TO THE SPRING AND FALL TIME OF YEAR FROM MARCH 15 TO JULY 15 AND SEPTEMBER 1 TO NOVEMBER 15, RESPECTIVELY.

#### FFIC

THE MINIMUM MOUNTING HEIGHT OF POST-MOUNTED SIGNS. MEASURED VERTICALLY FROM THE BOTTOM OF THE SIGN TO THE TOP OF THE CURB OR SIDEWALK. OR TO THE ELEVATION OF THE NEAR EDGE OF THE TRAVELED WAY, SHALL BE 7 FEET UNLESS OTHERWISE SPECIFIED ON THE PLANS.

#### PORARY TRAFFIC CONTROL

- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR TRAFFIC MANAGEMENT AND TO COMPLY WITH CONDITIONS OUTLINED WITHIN THE SPECIFICATIONS AND MASSDOT HIGHWAY DIVISION STANDARD DETAILS AND DRAWINGS FOR THE DEVELOPMENT OF TRAFFIC MANAGEMENT PLANS MANUAL.
- THIS PLAN DEPICTS IN SCHEMATIC FORM, THE ELEMENTS OF AN APPROACH TO THE LAYOUT AND PLANNING OF THE WORK DURING THE PROGRESS OF THE CONSTRUCTION OPERATIONS.THE PREPARER OF THIS PLAN HAS NO ROLE IN THE OVERSIGHT OR OTHERWISE IN THE IMPLEMENTATION OF THIS PLAN.
- CONTRACTOR SHALL COORDINATE THE CONSTRUCTION EFFORT WITH OTHER PROJECTS IN THE VICINITY IN ORDER TO MINIMIZE POTENTIAL TRAFFIC AND PARKING IMPACTS.

#### **TEMPORARY TRAFFIC CONTROL (CONT')**

- AS APPROVED OR DIRECTED BY THE ENGINEER.
- REQUIREMENTS OF THE INDIVIDUAL AGENCIES AND ABUTTERS.
- LIGHTS AT ONLY THE ENGINEERS DIRECTION.
- COMMENCEMENT OF CONSTRUCTION.
- ALTERNATIVE PLANS, AT NO ADDITIONAL COST.
- ACTUAL FIELD CONDITIONS AT TIME OF CONSTRUCTION.
- ENGINEER.

- SPECIAL EVENTS WITHIN THE LIMITS OF WORK.

#### CONSTRUCTION SIGNING:

- NOTIFIED THE ENGINEER PRIOR TO INSTALLING SIGNS.
- THE END OF THE WORK.
- SPECIFIED ON THE PLANS.

#### PLAN REFERENCES

- FILE P. PAGE 167.
- PAGE 183.
- ON DECEMBER 15, 1975 IN MAP BOOK 5, PAGE 14.

4. THE TEMPORARY TRAFFIC CONTROL PLANS CONTAINED HEREIN ARE GIVEN AS A GUIDE FOR TYPICAL WORK ZONE TRAFFIC CONTROL APPLICATIONS FOR THE TYPES OF WORK ANTICIPATED FOR THIS PROJECT. THEY ARE NOT INTENDED TO COVER ALL POSSIBLE CONSTRUCTION OPERATIONS WHICH THE CONTRACTOR MAY CHOOSE TO EMPLOY. WORK ZONE TRAFFIC CONTROL FOR OTHER CONSTRUCTION OPERATIONS OR OTHER TRAFFIC SITUATIONS IF APPLICABLE SHALL BE IN ACCORDANCE WITH THE CURRENT M.U.T.C.D. AND

5. THESE PLANS ARE NOT INTENDED TO LIMIT THE CONTRACTORS RIGHT TO SCHEDULE THE WORK BUT TO OUTLINE ONE WAY OF PROGRESSING. THE CONTRACTOR IS EXPECTED TO USE KNOWLEDGE AND EXPERIENCE TO PERFORM THE WORK IN THE MOST EFFICIENT MANNER IN COMPLIANCE WITH THE DRAWING AND SPECIFICATIONS AND THE

6. LANE RESTRICTIONS (OTHER THAN ACTIVE WORK ZONES) MAY NOT REMAIN OVERNIGHT OR DURING NON-WORKING HOURS AND MUST BE REMOVED BY THE END OF EACH WORKING TIME RESTRICTION. AFTER EACH WORKING DAY, TRAFFIC CONTROL DEVICES THAT ARE NOT REQUIRED SHALL BE MOVED OFF THE ROADWAY OR FULL DEPTH CONSTRUCTION AREA AND PLACED SO AS NOT TO IMPEDE PEDESTRIAN AREAS, ABUTTER ACCESS OR CAUSE CONFUSION TO ROADWAY USERS. IN CERTAIN CIRCUMSTANCES, AND ONLY WITH THE APPROVAL OF THE ENGINEER, CAN LANE RESTRICTIONS REMAIN OVERNIGHT, REFLECTORIZED DRUMS MAY BE FITTED WITH STEADY BURN AND/OR FLASHING WARNING

7. PLACE ALL CONSTRUCTION SIGNING AND TRAFFIC CONTROL DEVICES PRIOR TO

8. CONTRACTOR SHALL SECURE WORK AREAS ACCORDING TO CURRENT CONDITIONS TO ENSURE PUBLIC SAFETY AND CONVENIENCE. THIS SHALL INCLUDE ENSURING THAT ALL EXCAVATIONS ARE PROTECTED AT ALL TIMES AND WHEN WORK SHIFT IS COMPLETED.

9. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND APPROVAL, TEMPORARY TRAFFIC CONTROL PLANS FOR ANY WORK OUTSIDE THE WORK ZONES INDICATED IN THESE DRAWINGS, INCLUDING ALTERNATIVE PHASING OR MODIFICATION OF ANY ASPECT OF THE TEMPORARY TRAFFIC CONTROL PLANS OR CONSTRUCTION STAGING. THE CONTRACTOR SHALL BEAR RESPONSIBILITY FOR THE SUBMISSION AND REVIEW OF

10. EXISTING CONDITIONS ARE FOR CONTRACTOR INFORMATION ONLY AND ARE EXISTING CONDITIONS AT THE TIME OF DESIGN. THE CONTRACTOR SHALL VERIFY, AS NECESSARY.

11. TYPICAL DAYTIME WORK HOURS ARE FROM 7:00 AM TO 3:30 PM ON WEEKDAYS, UNLESS OTHERWISE PERMITTED BY THE ENGINEER. WORK SHALL NOT BE PERFORMED THE DAY BEFORE, OR THE DAY AFTER, A HOLIDAY WEEKEND, UNLESS OTHERWISE PERMITTED BY THE ENGINEER. REFER TO TEMPORARY TRAFFIC CONTROL PLANS, SPECIFICATIONS, AND PERMITS FOR MODIFICATION TO ALLOWABLE WORK PERIODS. ALL WORK SCHEDULES, HOWEVER, SHALL BE PRE-APPROVED BY THE DEPARTMENT PRIOR TO BEGINNING WORK WORK NECESSARY OUTSIDE OF THESE NORMAL WORK HOURS BECAUSE OF TRAFFIC CONDITIONS, AS NOTED IN THE PLANS OR SPECIFICATIONS, SHALL BE APPROVED BY THE

12. CONTRACTOR SHALL PROVIDE DETAILS FOR TRAFFIC CONTROL AS DIRECTED BY THE ENGINEER AND IN ACCORDANCE WITH THE SPECIFICATIONS. CONTRACTOR SHALL BE GUIDED BY TEMPORARY TRAFFIC CONTROL LAYOUTS PROVIDED FOR SPECIFIC LOCATIONS AND BY TYPICAL LAYOUTS AT ALL OTHER LOCATIONS. TYPICAL LAYOUTS SHALL CONFORM TO PART 6 OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, LATEST EDITION.

13. WORK ZONES INDICATED ON THE TEMPORARY TRAFFIC CONTROL PLANS ARE INTENDED FOR THE DURATION OF THE WORK WITHIN THE ZONES ONLY AND SHALL BE RESTORED TO CONDITIONS ACCEPTABLE TO THE ENGINEER AT COMPLETION OF THE WORK INDICATED.

14. CONTRACTOR SHALL COORDINATE WITH THE ENGINEER CONCERNING ALL SCHEDULED

1. LOCATIONS OF SIGNS SHOWN ARE APPROXIMATE. EXACT LOCATION SHALL BE DETERMINED BY THE CONTRACTOR IN THE FIELD. THE CONTRACTOR SHALL ENSURE THAT SIGNS ARE PLACED IN ACCORDANCE WITH THE CURRENT M.U.T.C.D. THE CONTRACTOR SHALL

2. EXISTING SIGNING WHICH CONFLICTS WITH PROPOSED CONSTRUCTION TRAFFIC MANAGEMENT SIGNING SHALL BE REMOVED AND STACKED OR COVERED AND RESTORED AT

3. ALL SIGNS SHALL BE COVERED OR REMOVED WHEN CONDITION IS NOT IN EFFECT

4. THE MINIMUM MOUNTING HEIGHT OF POST-MOUNTED SIGNS, MEASURED VERTICALLY FROM THE BOTTOM OF THE SIGN TO THE TOP OF THE CURB OR SIDEWALK, OR TO THE ELEVATION OF THE NEAR EDGE OF THE TRAVELED WAY, SHALL BE 7 FEET UNLESS OTHERWISE

1. PLAN ENTITLED "SURVEY OF LAND IN NEW MARLBOROUGH, MASSACHUSETTS PREPARED FOR ROBIN RICHARDSON" FILED WITH THE REGISTRY OF DEEDS ON MAY 5, 2011 IN PLAT

2. "SURVEY OF LAND IN NEW MARLBOROUGH, MASSACHUSETTS PREPARED FOR ROBIN RICHARDSON" FILED WITH THE REGISTRY OF DEEDS ON SEPTEMBER 8, 2011 IN PLAT FILE P,

"PLAN OF PROPERTY OWNED BY FRANK J. & HELEN SANTELLI IN THE VILLAGE OF MILL RIVER IN THE TOWN OF NEW MARLBOROUGH, MASS." FILED WITH THE REGISTRY OF DEEDS

#### **NEW MARLBOROUGH** KEYES HILL ROAD OVER UMPACHENE RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	BFL(BR-OFF)-003S(798)X	3	42
	PROJECT FILE NO.	609078	

GENERAL NOTES



	KEYE	S HIL	NEW MARLBORO L ROAD OVER UMI	UGH PACH	ENE RI	
T NOTES:		STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
L ROAD		MA	BFL(BR-OFF)-003S(798)X	4	42	
			PROJECT FILE NO.	609078	5	
<u>DEPTH ROAI</u> TO 52+19 AN	<u>2WAY AND BRIDGE CONSTRUCTION</u> D 52+75 TO 55+25.00)		TYPICAL SECTION	ONS		
OURSE:	1.50" SUPERPAVE SURFACE COURSE 9.5 (SSC-9.5) OVER ASPHALT EMULSION FOR TACK COAT (RS-1H) OVER					
	1.75" SUPERPAVE INTERMEDIATE COURSE 12.5 (SIC FOR TACK COAT (RS-1H) OVER	;-12.5)	OVER ASPHALT EMU	LSION		
RSE:	3.25" SUPERPAVE BASE COURSE 25.0 (SBC-25.0) OV	ER				
COURSE:	4" DENSE GRADED CRUSHED STONE OVER 8" GRAVEL BORROW TYPE B					
	RIDGE:					
10 52+75)						
OURSE:	1.5" SUPERPAVE BRIDGE SURFACE COURSE 9.5 POLYMER (SSC-B-9.5-P) OVER ASPHALT EMULSION FOR TACK COAT (RS-1H) OVER					
E COURSE:	1.5" SUPERPAVE BRIDGE PROTECTIVE COURSE 9.5 F MEMBRANE WATERPROOFING FOR BRIDGE DECKS	POLYN	1ER (SPC-B-9.5-P) OV	ER		
2HALT DRIVE 50+85 LT. T ·42.5 RT. TO \$	<u>WAY:</u> O 50+97 LT. & 53+67.5 RT.)					
OURSE:	1.5" SURFACE COURSE OVER					
	2.5" INTERMEDIATE COURSE OVER					
	8" GRAVEL BORROW TYPE B					
VEWAY:						
OURSE:	4" DENSE GRADE CRUSHED STONE OVER					
	8" GRAVEL BORROW TYPE B					
ALL BE PER	SECTION 450 QUALITY ASSURANCE OF HMA AND SEC	TION 4	150 SUPERPAVE HMA	SPEC	IFICATIO	

1. TACK COAT SHALL BE SPRAY APPLIED TRIPLE OVERLAP FOR UNIFORM COVERAGE BETWEEN 0.06 TO 0.08 GAL/SY OVER

3. PROPOSED VERTICAL GRANITE CURB TYPE VA3 SHALL BE INSTALLED PER MASSDOT STANDARD DRAWING E106.3.0.



-			К	EYES HILL R	OAD CONSTRUCTION B	ASELINE DA	TA
-	NUMBER	STARTING STATION	NORTHING	EASTING	CURVE DATA	LINE DATA	ENDING STATIO
-	L1	50+00.00	2863824.8906	181365.9126		S81°01'40"E 165.00'	51+65.00
-	C1	51+65.00	2863799.1583	181528.8938	R=1500.00 [°] Δ=1°31'40" L=40.00' T=20.00'		52+05.00
-	L2	52+05.00	2863792.3941	181568.3165		S79°30'00"E 182.00'	53+87.00
-	C2	53+87.00	2863759.2272	181747.2689	$\begin{array}{ccc} R = 2000.00^{'} & \Delta = 3^{\circ}40'01'' \\ L = 128.00' & T = 64.02' \end{array}$		55+15.00
	L3	55+15.00	2863739.9430	181873.7858		S83°10'01"E 85.00'	56+00.0
N: 2863843.673' E: 181626.316' EL: 983.241' SPK DETAILED SURVEY 4 0 11 2555 (WSP 6) N: 2863802.653' E: 181576.716' EL: 989.919' REBAR 13.44' 52 PT +05.00 CSW CSW OHW	AJUR MOL	STA 52 12.63 N 286 E 181 OOD SPLIT AIL FENCE H85°21'01"W STA 52+71.31 27.37 RT N 2863753.39 E 181628.526	2+74.13 LT 3792.2105 638.5939	54"E 186.08' EG A 53+16.91 .70 RT 2863749.6794 181674.2191 BENCHM/ RR SPK UP3/ EL=10	ID: 410 1 79 KEYES H N/F LAND EDWARD B. G DEED BK 359 WOOD SPLIT RAIL FENCE PI 54 GUPL 3/30/4 SATE ARK C: (30/57) 000.78'	00 0 ILL ROAD S OF OODNOW PG 630 BORDER BORDER BORDER 6 C +87.00 N78°1 4 31: 57 OHW 6"DECIDU N80°26'43"W 0 28 5290 471 0 2	4'32"W 3.58' "20 MPH" JOUS • 
$\begin{array}{c} 988 \\ 988 \\ 998 \\ 5977 \\ 409 \\ 998 \\ 0 \\ 990.9 \\ \times \\ 990.7 \\ \times 991.4 \\ 990.9 \\ \times \\ 990.9 \\ \times \\ 990.9 \\ \times \\ 990.9 \\ \times \\ 990.7 \\ \times 991.4 \\ 990.7 \\ \times 991.4 \\ 3 \\ 990.9 \\ \times \\ 3 \\ 599 \\ 14 \\ L \end{array}$	579·29·27 ×	$\frac{1}{2}$	Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Chilt Ch	ND POST	2555 (WSP 6) N: 2863802.653' E: 181576.716' EL: 989.919' REBAR	N 6 802.653 6.716 919	×1023 ×1019.5 ×1019.5 ×1019.5 ×1020.3 .0 (









#### NEW MARLBOROUGH **KEYES HILL ROAD OVER UMPACHENE RIVER** SHEET TOTAL NO. SHEETS FED. AID PROJ. NO. STATE MA BFL(BR-OFF)-003S(798)X 8 42 PROJECT FILE NO. 609078 **CURB TIE & GRADING PLAN EDGE OF CURB - CURVE TABLE** START COORDINATES | START DIRECTION | END DIRECTION END COORDINATES N 2863783.5448 N 2863790.2684 S79° 30' 00"E 19.881 S81° 01' 40"E E 181527.4902 E 181566.6764 N 2863801.2434 N 2863808.0482 20.121 S81° 01' 40"E S79° 30' 00"E E 181530.2974 E 181569.9566 N 2863750.3779 N 2863731.0070 S83° 10' 01"E 64.310 S79° 30' 00"E E 181745.6288 E 181872.7150 N 2863768.0765 N 2863748.8791 63.734 S79° 30' 00"E S83° 10' 01"E E 181748.9090 E 181874.8566 PROJECT END STA. 55+25 N 2863738.7533 E 181883.7148 +28.96 - WOOD RAIL FENCE **∑**26.34' LT (TO RETAIN) +13.29 - EDGE OF EXIST. GRAVEL 28.06' LT RAIL FENCE S84°05'49"E 144.50 +15.00"" ൣ<u>൭൜൹ഁ൭൭ഀൎ൹</u>ഺൎഀ൹൝൝൛<u>ഀഀ൫൜</u> 9.00' LT – S83°10'01"E **KEYES HILL ROAD** 55 C4-- LIMIT OF PAVEMENT PT +15.00-C3-87.93' * * * * * N82°<u>37'5</u>0"W +15.00 192.64' 9.00' RT N80°<u>26'4</u>3"W _S62°19'45"W 8.39' - EDGE OF EXIST. GRAVEL - OFFSET: 21.60 └ DENSE GRADED CRUSHED STONE (TYP.) OFFSET: 9.00 OFFSET: 12.00 OFFSET: 73.08 FOR CONSTRUCTION PLAN: SEE SHEET NO. 6 100 SCALE: 1" = 20'

078_HD8 (CURB TIE PLAN).DWG Plotted on 29-June-2022 5:001











	TEMPORARY TRAFFIC CONTROL SIGN SUMMARY													
IDENTIFI- CATION	SIZE OF	SIGN (IN)	TEXT	TE DIMEN (INC	XT SIONS HES)	IS NUMBER OF SIGNS		COLOR					UNIT AREA II AREA SQUAR	AREA IN SQUARE
NUMBER	WIDTH	HEIGHT		LETTER	HEIGHT	REQUIRED	BAC GRO	CK- UND	LEGEND		BOR	DER	(S.F.)	FEET
M4-8A	24	18	END DETOUR	(	Ð	2	0	$\mathbf{)}$	Ċ	D	Ċ	Ð	3.00	6.00
M4-9	30	24				1							5.00	5.00
M4-9R	30	24				2							5.00	10.00
M4-10L	48	18	DETOUR			1							6.00	6.00
M4-10R	48	18	DETOUR			1							6.00	6.00
R11-2	48	30	ROAD CLOSED			2							10.00	20.00
R11-3B	60	30	BRIDGE OUT AHEAD LOCAL TRAFFIC ONLY			2							12.50	25.00
SP-1	60	30	KEYES HILL ROAD CLOSED AHEAD LOCAL TRAFFIC ONLY			3		1					12.50	37.50

SEE MUTCD 2022 EDITION, 1990 STD. DRAWINGS FOR SIGNS AND SUPPORTS SECTION M9.30.0. OF THE MASSDOT STANDARD SPECIFICATION FOR TEXT DIMENSIONS AND COLOR. BACKGROUND SHEETING FOR ALL CONSTRUCTION WARNING SIGNS SHALL BE OF A FLUORESCENT ORANGE COLOR PER 2024 MASSDOT SPECIFICATIONS SECTION 850.43.



#### NEW MARLBOROUGH KEYES HILL ROAD OVER UMPACHENE RIVER

STATEFED. AID PROJ. NO.SHEET<br/>NO.TOTAL<br/>SHEETSMABFL(BR-OFF)-003S(798)X1142PROJECT FILE NO.609078

DETOUR PLAN





![](_page_13_Figure_0.jpeg)

![](_page_13_Figure_1.jpeg)

![](_page_13_Figure_2.jpeg)

![](_page_13_Figure_4.jpeg)

![](_page_13_Picture_5.jpeg)

![](_page_14_Figure_0.jpeg)

![](_page_15_Figure_0.jpeg)

![](_page_15_Figure_3.jpeg)

![](_page_15_Figure_4.jpeg)

![](_page_15_Figure_5.jpeg)

![](_page_15_Figure_6.jpeg)

![](_page_15_Figure_7.jpeg)

![](_page_15_Figure_8.jpeg)

### **NEW MARLBOROUGH** /ER

Έ	S HIL	L ROAD OVER UMP	PACH	ENE F	۷I۶
	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
	MA	BFL(BR-OFF)-003S(798)X	16	42	
		PROJECT FILE NO.	609078		

UPSTREAM CHANNEL BANK IMPROVEMENT

![](_page_16_Figure_0.jpeg)

![](_page_16_Figure_1.jpeg)

#### NEW MARLBOROUGH **KEYES HILL ROAD OVER UMPACHENE RIVER**

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	BFL(BR-OFF)-003S(798)X	17	42
	PROJECT FILE NO.	609078	

### **KEY PLAN AND PROFILE**

### DRAWING INDEX

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			SEPT. 14, 2024	ISSUEE	D FOR CONSTRUCTION	C- 4
		KIN CHUNG LAM CIVII		Massadusetts De Highway Div	partment of Transportation Islon	SEPTEMBE
		No. 32881		PROPOSEI	D BRIDGE	~
		BATTERS ISTERED INFO	NE	W MARL	BOROUGH	(SF)
3.3 =		Kin Chung Lam Digitally signed by Kin Chung Lam Date: 2024.11.04 08:57:53 -05'00'	0	KEYES HI VER UMPAC	ILL ROAD Chene River	Submitta
3 = )		LAMSON ENGINEERING CORPORATION	MASSACHUS 10 Alexander K. Bardow, P.E.	ETTS DEPARTN HIGHWAY ) PARK PLAZA tally signed by Alexander K. tow, P.E. 9: 2024.11.07 17:15:55 -05'00'	IENT OF TRANSPORTATION DIVISION BOSTON, MASS Cance Jacellee Digitally signed by Carrie Lavallee, P.E. Date: 2024.11.15 09:57:59 -05'00'	8 Structural Plans
			STATE BRIDGE ENG	SINEER	CHIEF ENGINEER	60907
	SHEET	T 1 OF 22 SHE	ETS BRID	GE NO. N	N-08-020 (CN3)	

### **GENERAL NOTES:**

#### DESIGN:

IN ACCORDANCE WITH THE 2020 (9TH EDITION) LRFD SPECIFICATIONS OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) FOR HL-93 LOADING.

#### MASSDOT BENCH MARKS:

"2555" (WSP 6):	REBAR N 286802.6530, E 81576.7160, ELEV. 989.919' (NAVD 1988)
"61":	SPK N 2863755.018, E 181738.239 ELEV. 998.710' (NAVD 1988)
BENCH MARK C:	RR SPK UP3/30/57 Elev. 1000.78' (NAVD 1988)

ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

#### DATE:

TO BE PLACED ON THE INSIDE FACE OF THE SOUTHWESTERLY AND NORTHEASTERLY HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. ALL HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

### MASSDOT SURVEY NOTEBOOKS:

SURVEY NOTEBOOK NUMBER IS 40576.

#### SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS.

#### FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH APPROVAL OF THE ENGINEER.

#### UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIALS SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

#### ANCHOR BOLTS (FOR BRIDGE RAILING):

ALL ANCHOR BOLTS SHALL BE SET BY TEMPLATE BEFORE THE CONCRETE IS PLACED.

### EXISTING CONDITIONS:

INFORMATION SHOWN FOR EXISTING BRIDGE STRUCTURE IS TAKEN FROM THE EXISTING BRIDGE PLAN DATED OCTOBER 1938, AND APPROXIMATED FROM SURVEY DRAWINGS. ALL DIMENSIONS AND DETAILS SHOWN FOR EXISTING STRUCTURE ARE NOT GUARANTEED. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENT AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF AND SHALL NOT ORDER ANY MATERIAL OR BEGIN FABRICATION UNTIL THE ABOVE WORK IS COMPLETED AND THE EXTENT OF THE PROPOSED WORK IS APPROVED BY THE ENGINEER.

#### **GEOTECHNICAL DESIGN:**

SEE GEOTECHNICAL REPORT, DATED JANUARY 26, 2024.

NO CONSTRUCTION SHOULD BE CONDUCTED BELOW ELEV. 940.1 (APPROXIMATELY 52' FROM THE EXISTING ROADWAY SURFACE AT BRIDGE LOCATION) TO AVOID CONSTRUCTION ISSUES FROM POTENTIAL AQUIFER CONDITION.

#### CONCRETE:

ALL CONCRETE SHALL BE 5,000 PSI,  $\frac{3}{4}$  IN., 685 HP CEMENT CONCRETE.

TREMIE CONCRETE SHALL BE 4,000 PSI,  $\frac{3}{4}$  IN., 610 CEMENT CONCRETE. AN ADDITIONAL 10% OF CEMENT SHALL BE ADDED TO THIS CONCRETE DEPOSITED IN WATER.

ALL CIP AND PRECAST CONCRETE POUR SHOWN ON THESE CONSTRUCTION DRAWINGS WHERE ALL VOLUMETRIC DIMENSIONS ARE 4 FEET OR GREATER SHALL BE CONSIDERED TO BE MASS CONCRETE PLACEMENTS AND SHALL REQUIRE A HEAT OF HYDRATION ANALYSIS AND THERMAL CONTROL PLAN, AS SPECIFIED IN THE MASSDOT STANDARD SPECIFICATIONS.

### **REINFORCEMENT:**

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION 1. NONE	<u>#4 BARS</u> 16"	<u>#5 BARS</u> 17"	<u>#6 BARS</u> 21"
2.12" OF CONCRETE BELOW BAR	20	22	27
3. COATED BARS, COVER < 3d , OR	21"	26"	31"
CLEAR SPACING < 6d			
4. COATED BARS, ALL OTHER CASES	17"	21"	25"
5. CONDITION 2. AND 3.	23"	29"	35"
6. CONDITION 2. AND 4.	21"	27"	32"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

EPOXY COATED BARS:

ALL REINFORCING BARS AND SUPPORTING DEVICES IN THE ABUTMENTS, WINGWALLS, BACKWALLS, CURTAIN WALLS, RETAINING WALL CAP, CONCRETE DECK SLAB, AND SAFETY CURBS SHALL BE COATED.

#### MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECK SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS - SPLAY APPLIED.

	ESTIMATED QUANTITIES (NOT GUARANTEE	ED)	
ITEM NO.	DESCRIPTION	QUANTITY	UNIT
115.1	DEMOLITION OF BRIDGE NO. N-08-020	1	LS
127.	CONCRETE EXCAVATION	10	CY
140.	BRIDGE EXCAVATION	740	CY
140.1	BRIDGE EXCAVATION WITHIN COFFERDAM	280	CY
151.2	GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES	530	CY
156.	CRUSHED STONE	130	TON
160.3	CONTROLLED LOW-STRENGTH MATERIAL (> 300 PSI)	10	CY
191.11	CORE BORING	60	FT
450.601	SUPERPAVE BRIDGE SURFACE COURSE – 9.5 POLYMER (SSC–B – 9.5 – P)	10	TON
450.701	SUPERPAVE BRIDGE PROTECTIVE COURSE – 9.5 POLYMER (SPC–B – 9.5 – P)	10	TON
645.048	48 INCH CHAIN LINK FENCE (PIPE TOP RAIL)(LINE POST OPTION)	15	FT
698.4	GEOTEXTILE FABRIC FOR PERMANENT EROSION CONTROL	300	SY
950.31	EARTH SUPPORT SYSTEM – BRIDGE NO. N–08–020	1	LS
953.	STEEL SHEETING OBSTRUCTION	280	FT
983.1	RIPRAP	450	TON
991.1	CONTROL OF WATER - STRUCTURE NO. N-08-020	1	LS
994.01	TEMPORARY PROTECTIVE SHIELDING, BRIDGE NO. N-08-020	1	LS
995.01	BRIDGE STRUCTURE, BRIDGE NO. N-08-020	1	LS

SECTION MARK

![](_page_17_Picture_37.jpeg)

#### NEW MARLBOROUGH **KEYES HILL ROAD OVER UMPACHENE RIVER**

STATE FED. AID PROJ. NO. SHEET TOTAL NO. SHEETS MA BFL(BR-OFF)-003S(798)X 18 42 PROJECT FILE NO. 609078

### **GENERAL NOTES**

TRAFFIC DATA		
	ROADWAY OVER	ROADWAY/
DESIGN YEAR	2043	\YEAR/
AVERAGE DAILY TRAFFIC – PRESENT	213	\ADT/
AVERAGE DAILY TRAFFIC – DESIGN YEAR	260	à d⁄t
DESIGN HOURLY VOLUME	23	D₩V
DIRECTIONAL DISTRIBUTION	0.87	D/IST
TRUCK PERCENTAGE – AVERAGE DAY	10.8%	Ádtí
TRUCK PERCENTAGE – PEAK HOUR	4.3%	/PHTT\
DESIGN SPEED	20 MPH	/ DES \
DIRECTIONAL DESIGN HOURLY VOLUME	20	$/ DDHV \setminus$

SEISMIC DESIGN CRITERIA	
DESIGN RETURN PERIOD:	1000 TR.
DESIGN SPECTRA	
As	0.094
SDs	0.211
SD1	0.089
SITE CLASS	D
SEISMIC DESIGN CATEGORY (SDC)	A

HYDRAULIC DESIGN FLOOD (HDF)	DATA
DRAINAGE AREA (SQ. MILES)	9.24
HDF DISCHARGE (C.F.S.)	1,673
HDF FREQUENCY (YEARS)	10
HDF VELOCITY (F.P.S.)	7.54
HDF ELEVATION (FEET, NAVD, UPSTREAM)	986.24
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	3,615
BASE FLOOD ELEVATION (FEET, NAVD, UPSTREAM)	989.57
SCOUR DESIGN FLOOD (SDF) EVENT D	ATA
SDF EVENT FREQUENCY (YEARS)	25
SDF ELEVATION (FEET, NAVD, UNDER BRIDGE)	987.36
SDF TOTAL SCOUR DEPTH AT ABUTMENT (FEET)	9.50
SDF TOTAL SCOUR DEPTH AT PIER (FEET)	N/A
SCOUR CHECK FLOOD (SCF) EVENT D	ATA
SCF EVENT FREQUENCY (YEARS)	50
SCF ELEVATION (FEET, NAVD, UNDER BRIDGE)	988.38
SCF TOTAL SCOUR DEPTH AT ABUTMENT (FEET)	11.14
SCF TOTAL SCOUR DEPTH AT PIER (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	N/A
FREQUENCY (IF KNOWN, YEARS)	N/A
MAXIMUM ELEVATION (FEET, NAVD)	N/A
DATE (MM/YYYY)	N/A
HISTORY OF ICE FLOES	N/A
EVIDENCE OF SCOUR AND EROSION	N/A

![](_page_17_Figure_45.jpeg)

- SECTION NUMBER DESIGNATION		
— SHEET NO. <u>NOTE:</u> NO SHEET NO. IS GIVEN IF IT IS ON THE SAME SHEET. INSTEAD "—" IS DENOTED.		
	SEPT. 14, 2024	ISSUED FOR CONSTRUCTION
	DATE	DESCRIPTION
	THIS SHEET IS CONSTRUCTION AUTHORIZED USE	APPROVED FOR BY MASSDOT SIGNATORY: STATE BRIDGE ENGINEER ONLY PRINTS OF LATEST DATE
SHEET 2 OF 22 SHE	ETS BRID	DGE NO. N-08-020 (CN3)

					LAMS 437 Che	ON E	ENGIN	EERING	CORPORATION	Boring No. BB-1 (Bridge Boring)	
		Bo City/Tow	ring Log	orougł	Phone: (	617) 5 dge No	58-0101 b.: N-08-	E-Mail: 020 (06N)	Lamsoneng@msn.com Project File No.: 60907	Scale: 1" = 5           78         Contract N	0.: -
	005	Location Groundv	: Keyes Hill vater Depth (F	Road of eet):	over Umpa *0' Date	achene & Tim	e River e: 8/21/2	20 7:00 a.r	Date & Time Started: n. Date & Time Complete	8/19/20 11:00 a.m.	Total Hours: 19.5
	- 995	Coordina Drilling (	ates: N 2,863 Company: Nev	,789.3 v Engla	E 181,58 and Boring	85.4 Contr	Ground	d Elevatior Dri	(Feet): 991.0' Inspector	's Name: Weijie Dong o Helper's Name: Cody	Richards
	_ _ 990 _	Sample Number S-1	Depth Range (Feet) 0' - 2'	Blc Coring 15	g Times M 26	per 6 linute I 32	Inches Per Foot 19	Recovery (inches) 15"	Field Des Dry, very dense, brown, FINE to coarse gravel, trace inorga	TO COARSE SAND, some nic silt.	Strata Changes
	- - - - 985	S-2	4' - 6'	14	8	6	4	11"	Dry, medium dense, brown, F fine to coarse gravel, trace inc	INE TO COARSE SAND, so organic silt.	me
-	_										
-	- - - 980 -	S-3	10' - 12'	3	4	5	15	0"	No Recovery (Sand & Gravel	from Wash).	11'6"
	_ _ 975 _	S-4	15' - 17'	10	23	22	8	9"	Wet, dense, gray, FINE TO C coarse gravel, trace inorganic	OARSE SAND, some fine to silt.	17'
	- - - - 970	S-5	20' - 22'	17	15	17	20	8"	Wet, dense, brown, FINE TO coarse gravel, trace inorganic	COARSE SAND, some fine silt.	to
-	- - - - 965	S-6	25' - 27'	12	19	15	18	17"	Wet, dense, brown, FINE SAN fine to medium gravel.	ND, some inorganic silt, som	e
-	_								Pockets of Cobbles / Boulder	s.	27'6"
-	_ 960 _	S-7	30' - 32'	20	25	22	21	17"	Wet, dense, brown, FINE SA fine to medium gravel.	ND, some inorganic silt, som	30' ie
	- - - 955 -	S-8	35' - 37'	16	18	23	30	18"	Wet, dense, brown, FINE SA fine to medium gravel.	ND, some inorganic silt, som	ie
-	- - - - 950 -	S-9	40' - 42'	19	42	44	78	7"	Wet, very dense, brown, FIN some fine to medium gravel.	E SAND, some inorganic silt	,
-	- - - 945 -	S-10	45' - 47'	21	23	25	28	21"	Wet, dense, brown, FINE SAN fine to medium gravel.	ND, some inorganic silt, som	9
-	_										
	_ 940	S-11	50' - 50'3"	120/	/3"			3"	Wet, very dense, brown, FINE some fine to medium gravel.	E SAND, some inorganic silt,	50'3"
	_								Pockets of Cobbles / Boudlers	5	54'6"
	- 935 	S-12	55' - 57'	19	26	28	27	18"	Wet, very dense, brown, FINE some fine to coarse gravel.	E SAND, some inorganic silt,	
	_ _ 930	S-13	60' - 60'9"	21	100/3"			5"	Wet, very dense, brown, FINE some fine to coarse gravel.	E SAND, some inorganic silt,	60'9"
	_								Pockets of Cobbles / Boudlers	3	64'
-	- 925 	S-14	65' - 67'	21	57	29	35	15"	Wet, very dense, brown, FINE some fine to coarse gravel.	E SAND, some inorganic silt,	
	_ _ 920 _	S-15	70' - 72'	13	32	54	48	9"	Wet, very dense, brown, FINE some fine to coarse gravel.	E SAND, some inorganic silt,	
	- - - - 915	S-16	75' - 77'	16	19	23	22	17"	Wet, dense, brown, FINE SAN fine to coarse gravel.	ND, some inorganic silt, som	e
	 910	S-17	80' - 82'	26	42	72	75	6"	Wet, very dense, brown, FINE some fine to coarse gravel.	E SAND, some inorganic silt,	
	  905	S-18	85' - 85'9"	103	70/3"			8"	Wet, very dense, brown, FINE TO COARSE GRAVEL, trace	E TO COARSE SAND & FINI inorganic silt.	=
	-	S-19	90' - 90'3"	120/	/3"			3"	Wet, very dense, brown, FINE	E TO COARSE SAND & FINI	90'3"

### BORING NO. BB-1 (CONTINUE

![](_page_18_Figure_2.jpeg)

-NO CONSTRUCTION BELOW ELEV. 940.1± (SEE NOTE 4)

### BORING NOTES:

- 1. LOCATION OF A BORING IS SHOWN ON SHEET 1 OF 22 AS THUS: •
- 2. LOCATION OF A PROBE IS SHOWN ON SHEET 1 OF 22 AS THUS: +
- 3. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 4. WATER LEVELS SHOWN ON THE BORING LOGS COULD NOT BE MEASURI TIME OF TAKING BORINGS DUE TO ARTESIAN AQUIFER CONDITION.

ARTESIAN AQUIFER WAS ENCOUNTERED DURING THE BORING EXPLORATI AT BOTH BORINGS BB-1 AND BB-2. TOP OF THE ARTESIAN AQUIFER ESTIMATED BETWEEN DEPTH OF 52' TO 96'. NO CONSTRUCTION SHOU CONDUCTED BELOW ELEV. 940.1 (APPROXIMATELY 52' FROM THE EXIST ROADWAY SURFACE AT BRIDGE LOCATION) TO AVOID CONSTRUCTION ISS POTENTIAL AQUIFER CONDITION.

DUE TO THE PROXIMITY OF THE PROPOSED PROJECT TO THE UMPACHE THE GROUNDWATER LEVEL IS LIKELY TO BE INFLUENCED BY THE RIVER WHICH ALSO FLUCTUATES DUE TO SEASONAL CHANGES AND STORM EVE

- 5. FIGURES IN THE COLUMN INDICATE NUMBER OF BLOWS REQUIRED TO 1-3/8" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT F
- 6. BORING SAMPLES ARE STORED AT A STORAGE FACILITY LOCATED ON RO (219 WINTHROP AVENUE) IN LAWRENCE, MASSACHUSETTS. THE CONTRA EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING THE MASSACHI DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL SECTION AT 10 PARK ROOM 6500, BOSTON, MA 02116, AT (857)368-9182.
- 7. ALL BORINGS WERE MADE IN AUGUST 2020.
- 8. BORINGS WERE MADE BY NEW ENGLAND BORING CONTRACTORS LOCATE FORDWAY STREET, DERRY, NEW HAMPSHIRE 03038.
- 9. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THR

No. BB-1				KEYES H	NEW MA	KLBORO	OGH PACHI	ENE R	IVER
oring) Scale: 1" = 5'				STA	TE FED. AID	PROJ. NO.	SHEET NO.	TOTAL SHEETS	
	Strata Changes				PROJECT F	ILE NO.	19 609078	42	
raak @ 09'6"					BORING	LOGS (1	OF 3)		
ОСК (Д 98 6	98'6"								
	105'8"								
05'8"									
artesian.									
ve Device Stand: -	Box: -								
Pipe: - Screen F Drill Rig: Truck GT8	Pipe: -								
Weight: 300 lbs F Fypes: HW N	⁻ all: 24" W								
4" 3 25' 10	5" 20'								
ic Hammer Weight: 1 lammer Weight:	1 7 ₈ 1D 140 lbs								
ammer Weight: rrel Type: - Size:	Fall: 30" -								
OF THE									
D AT THE									
DATITL									
N PROGRAM									
LD BE NG									
JES FROM									
NE RIVER.									
WATER NTS.									
RIVE A									
$A \cup D \cap Z \cap$									
ALLING JU.									
UTE 114 CTOR MAY									
UTE 114 CTOR MAY ISETTS PLAZA,									
UTE 114 CTOR MAY ISETTS PLAZA,			SEPT 1	4 2024					NI
UTE 114 CTOR MAY USETTS PLAZA,			SEPT. 1	4, 2024 ATE	ISSUE	D FOR C	ONSTF PTION	RUCTIC	N
OUTE 114 CTOR MAY USETTS PLAZA,			SEPT. 1 D/ THIS S CONS	4, 2024 ATE HEET IS AF IRUCTION E	ISSUE PPROVED FO BY MASSDOT	D FOR C DESCRI			N Jale
OUTE 114 CTOR MAY SETTS PLAZA, DUGHOUT.			SEPT. 1 D/ THIS S CONS ⁻ AUT	4, 2024 ATE HEET IS AF IRUCTION F HORIZED S USE (	ISSUE PPROVED FO BY MASSDOT SIGNATORY: DNLY PRINTS	D FOR C DESCRI DR STATE S OF LAT	ONSTR PTION May M BRID EST D	RUCTIC GE EN ATE	IGINEER

					BOF	RING	<u>NO. BB-2</u>				BORING NO. I	BB-2 (CON	ITINUED)			
	Moving Massachus	Highway	<b>E L</b> 4	AMSON 37 Cherry	N ENGIN Street, #109	EERING 9, Newton, M	CORPORATION Massachusetts 02465Boring No. (Bridge Boring)BB-2		Г <b></b>						NEW MARLBOROUG KEYES HILL ROAD OVER UMPA	iH CHENE R!
	BOI City/Tow	n: New Ma	F borough	hone: (61 Bridge	7) 558-0101 e No.: N-08-0	E-Mail: L 020 (06N)	Lamsoneng@msn.com     Scale: 1" = 5'       Project File No.: 609078     Contract No.: -		900	ASSS DOT-	LAMSON ENGINEERI 437 Cherry Street, #109, New Phone: (617) 558, 0101 E-M	NING CORPORATIO	6 (Bridge Boring)		STATE FED. AID PROJ. NO.	HEET TOTAL NO. SHEETS
	Location Groundv	Keyes H ater Depth	I Road ove Feet): *0'	er Umpach Date & ⊺	ene River Time: 8/19/2	20 7:00 a.m.	Date & Time Started:         8/17/20         12:00 a.m.         Total Hour           n.         Date & Time Completed:         8/19/20         11:00 a.m.         19.5		Samp	Depth Range	Blow Counts per 6 Inches Recov	very hes)	eld Description	Strata Changes	MA BFL(BR-OFF)-003S(798)X	20 42
995	Coordina Drilling C	ites: N 2,86 ompany: N	3,778.3 [ w Englanc	E 181,648. Boring Co	.2 Ground	d Elevation ( Drille	(Feet):992.1'Inspector's Name:Weijie Dongler's Name:Mark D'AmbrosioHelper's Name:Cody Richards							96'	PROJECT FILE NO. 60	9078
	Sample Number	Depth Rang (Feet)	e Blow Coring T	Counts pe ïmes Minu	er 6 Inches ute Per Foot	Recovery (inches)	Field Description Strat	295	— 895 —			Bott	om of Exploration @ 96'		BORING LOGS (2 OF	3)
	S-1	0' - 2'	16	16 1	16 16	15" [	Dry, dense, brown, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.					- Boring terminated at	96' deep due to artesian.			
990									890							
— —	S-2	5' - 7'	12	6	5 7	5" [	Dry, medium dense, brown, FINE TO COARSE SAND, trace									
— — 985						f	fine gravel, trace inorganic silt.									
_  -																
-	S-3	10' - 12'	15	12	16 11	6" \ f	Wet, medium dense, brown, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.									
980							14'									
	S-4	15' - 17'	2	2	1 3	7"	Wet, soft, gray, INORGANIC SILT, pieces of wood, trace fine									
75						S	sand, trace fine gravel.	BOTTOM OF FOOTING								
							18'6	ELEV. 972.00								
070	S-5	20' - 22'	6	7 2	26 30	11"  \   f	Wet, dense, brown, FINE SAND, some inorganic silt, trace fine gravel.									
- 9/U -																
	S-6	25' - 27'	9	11 2	20 13	16"	Wet, dense, brown, FINE SAND, some inorganic silt, trace	BOTTOM OF TREMIE CONCRETE								
- 965						f	nne gravei.									
0.00	S-7	30' - 32'	14	30 2	26 22	12"	Wet, very dense, brown, FINE SAND, some inorganic silt, some fine to coarse gravel.									
- 960									Notes	<u>:</u> und water measure	d at ground level due to artesian.	Arrov Sign	v-Board: - Protective Dev :: - Well Depth: -	ice Stand: - Box: - Solid Pipe: -		
	S-8	35' - 37'	13	20 1	19 20	19"	Wet, dense, brown, FINE SAND, some inorganic silt, some				Penetration Resistance (N) C	Guide:	s: 4 Stick Up Pipe: Type of Drill Rig	- Screen Pipe: - g: Truck GT8		
955							fine to coarse gravel.	TIP OF STEEL SHEETING		Cohesionless Relative Density	Soils (Sands, Gravels)         Consistence           Penetration Resistance         Consistence	Cohesive Soils (Silts, Clays istency Penetration Re	) Hammer Weigh sistance Casing Types:	nt: 300 lbs Fall: 24" HW NW		
								OF 22, NOTE 4)		Very Loose Loose Medium Dense	0 - 4 Very 4 - 10 So 10 - 30 Modiu	ry Soft 0 - 2 Soft 2 - 4	Size: Depth:	4" 3" 25' 95'		
	S-9	40' - 42'	14	17 3	39 25	17"	Wet, very dense, brown, FINE SAND, some inorganic silt, some fine to coarse gravel.			Dense Very Dense	30 - 50 Si Over 50 Very	Stiff 8 - 15 v Stiff 15 - 30	Sampler Type: Automatic Ham	S/S Size: 1 ³ / ₈ " ID mer Weight: 140 lbs		
									N= Ter	=Sum of Second a ms Used for Second	and Third 6" Blow Counts Ha	ard Over 3 , some = 10-40% trace = 10	Donut Hammer or less Core Barrel Two	Weight: Fall: 30"		
	S-10	45' - 47'	10	12 1	16 18	21"	Wet, dense, brown, FINE SAND, some inorganic silt, some				,	, 13 13 /0, udoc – 10			1	
							ine to coarse gravel.									
								NO CONSTRUCTION BELOW								
10	S-11	50' - 52'	12	20 3	34 45	17"	vvet, very dense, brown, FINE SAND, some inorganic silt, some fine to coarse gravel.	ELEV. 940.1±								
<del>-</del> υ																
-	S-12	55' - 57'	16	35 5	54 58	12"	Wet, very dense, brown, FINE SAND, some inorganic silt,									
- 935																
_		<b>6</b> 55					Wat you dongo brown FINE CAND									
- 9.30	S-13	60' - 62'	21	30 2	22 31	5"	some fine to coarse gravel.									
500																
	S-14	65' - 67'	10	20 2	23 28	8"	Wet, dense, brown, FINE SAND, some inorganic silt, some fine to coarse gravel.									
925							~									
	Q 1E	70' 70'	20	18 ^	on oo	16"	Wet, dense, brown, FINF SAND, some inorganic silt, some									
920	61-0	10 - 12		10 Z	-0 20	1	fine to coarse gravel.									
	S-16	75' - 76'6"	17	27 10	08	13"	Wet, very dense, brown, FINE SAND, some inorganic silt, some fine to coarse gravel.									
915																
	S-17	80' - 82'	28	40 5	50 41	14"	Wet, very dense, brown, FINE SAND, some inorganic silt,									
10				J			some fine to coarse gravel.									
											BORING	<u>G N</u> OTES:				
	S-18	85' - 87'	63	57 4	12 35	17"	Wet, very dense, brown, FINE TO COARSE SAND, some fine gravel, trace inorganic silt.				FOR ROF	RING NOTES SE	E SHEET 3			
- 905												, LO, OL				
															DATE ISSUED FOR CON	ION
- 900															THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	. ht
															AUTHORIZED SIGNATORY: STATE E	RIDGE I
			1				I									

		SEPT. 14		ISSUED FOR CONSTRUCTION							
		DA	ΓE				DESC	RIPTION		$\bigcirc$	
		THIS SH CONSTE	EET IS RUCTION	APPR BY	OVED F MASSD(	FOR DT	$\mathcal{C}$	Ang In	<i>l</i> X	Jalen	
		AUTH	IORIZED	SIGN	ATORY:	-	STA	re' brid	GE	ENGINEER	
			USE	ONL	Y PRIN	TS (	OF L/	ATEST D	ATE		
2	SHE	ETS	BRID	)GE	NO.	Ν	-08	3-020	C	(CN3)	

![](_page_20_Figure_0.jpeg)

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Moving Massachusetts Forw	DOT				RING	G COF		<b>DN</b>	Probe	No.	Page
Prob		· · · · · · · · · · · · · · · · · · ·	Phone: (617)	) 558-0101 I	E Mail:	Lamson	ieng@msn.co	om	P	2-2	1/1
City/Town:	New Marlb	orough	Bridge	e No.: N-08-02	20 (06	V)	Project File	No.: 609	078	Contract N	lo.:
Location: K	eyes Hill R	oad over	Umpachene	River			Date & Time	Started:	8/17/20	0:00 am	Total Hours:
Coordinates	s: N 2,863	,782.5 E	181,639.4	Ground Elev.	(Feet):	991.7'	Date & Time	Complet	ed: 8/17/20	11:30 am	1.5
Drilling Con	npany: Nev	/ Englanc	Boring Con	tractors	Dri	ler's Na	me: Mark D'A	mbrosio	Helper's	Name: Cod	y Richards
Inspector's	Name: We	ijie Dong	Type of Dr	ill Rig: Truck -	GT8	Drill Ro	ds/Solid Auge	rs: DR	Type: AW S	ize: 1-3/8" H	ammer: 140 lb
	INFORMATION LOG										
	Probe Number	Depth	Distance From Face	Refusal or Required Depth			Probe Number	Depth	Distance From Face	Refusal o Required Depth	or 1
	P-2A	2'6"	1'6"	Refusal							
*	P-2B	20'	2'6"	Required							
Remarks:	Coordina	tes and o	iround eleva	tion are for fir	st nrot	e P-2A					
* Probe		ated at by	nound cieva	$a$ to $\Lambda W$ rode $\lambda$	voro k	icked of	f the evicting	abutmer	at causing be	antrode D	secibly bit
refusal a	it 5'6" deep	. Unable	to continue	with the prob	e line.		i ule existility	abutifier	it causing be	fillious. F	
		_		•							
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			\			Edae	of Water				
						—Euge (Tvp.	)				
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				11	/			//			
				/'			4'	12" Ku			
◀				/				' 💐 Ρ-2	2B		
	кеу	es Hill Ro			Abutme	ent Back	wall	<u>P-2</u>	2A	/ ^{−Edge}	of Roadway
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				57							
		/									
Remarks:											

### NOTE:

THESE ARE THE PROBE RESULTS AT THE BACK OF EXISTING ABUTMENT.

#### NEW MARLBOROUGH KEYES HILL ROAD OVER UMPACHENE RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	BFL(BR-OFF)-003S(798)X	21	42
	PROJECT FILE NO.	609078	

### BORING LOGS (3 OF 3)

Probe No. LAMSON ENGINEERING CORPORATION Moving Massachusetts Forward Page 437 Cherry Street, #109, Newton, Massachusetts 02465 1/1 P-3 Phone: (617) 558-0101 E Mail: Lamsoneng@msn.com Probe Log City/Town: New Marlborough Bridge No.: N-08-020 (06N) Project File No.: 609078 Contract No.: Location: Keyes Hill Road over Umpachene River Date & Time Started: 8/24/20 11:00 am Total Hours: Coordinates: N 2,863,781.5 E 181,638.7 Ground Elev.(Feet): 991.7' Date & Time Completed: 8/24/20 12:30 pm 1.5 Drilling Company: New England Boring Contractors Driller's Name: Carl Downings Helper's Name: Mark D'Ambrosio Inspector's Name: Weijie Dong Type of Drill Rig: Geoprobe 6712DT Drill Rods/Solid Augers: 1.5" Drill Rods Type: Size: Hammer: INFORMATION LOG Distance Refusal or From Required Face Depth Distance Refusal or Probe Depth Probe Depth From Required Face Depth Number Number P-3F 8'6" P-3A 2'6" 1'6" Refusal 6'6" Refusal P-3B 6' 2'6" Refusal P-3G 11'6" 7'6" Refusal 3'6" Refusal P-3C 8' P-3D 8'6" 4'6" Refusal P-3E 8' 5'6" Refusal **Remarks:** Coordinates and ground elevation are for first probe P-3A. - Probe line P-3 created due to termination of P-2. Located at approximately 12" offset of P-2. SKETCH (PLAN VIEW) Edge of Water (Typ.) 5' 12" (TYP / Front Face of Existing Abutment Backwall──── Keyes Hill Road Edge of Roadway  $\rightarrow$ 1 P-3G \<u>P-3F</u> <u>P-3E</u> <u>P-3D</u> <u>P-3C</u> <u>P-3B</u> <u>P-3A</u> JER LON Remarks:

### BORING NOTES:

FOR BORING NOTES, SEE SHEET 3.

	SEPT. 14, 2024	ISSUE	D FOR CONSTRUC	TION
	DATE		DESCRIPTION	$\frown$
	THIS SHEET IS CONSTRUCTION AUTHORIZED	APPROVED FC I BY MASSDOT SIGNATORY:	DR T STATE BRIDGE	ENGINEER
	USE	ONLY PRINTS	S OF LATEST DATE	-
SHEET 5 OF 22 SHE	ETS BRID	GE NO.	N-08-020	(CN3)

![](_page_21_Figure_0.jpeg)

![](_page_22_Figure_0.jpeg)

![](_page_23_Figure_0.jpeg)

![](_page_24_Figure_0.jpeg)

![](_page_25_Figure_0.jpeg)

![](_page_25_Figure_1.jpeg)

![](_page_26_Figure_0.jpeg)

![](_page_27_Figure_0.jpeg)

![](_page_28_Figure_0.jpeg)

![](_page_29_Figure_0.jpeg)

![](_page_29_Figure_1.jpeg)

**KEYES HILL ROAD OVER UMPACHENE RIVER** SHEET TOTAL NO. SHEETS FED. AID PROJ. NO. STATE MA BFL(BR-OFF)-003S(798)X 30 42 PROJECT FILE NO. 609078 WINGWALL DETAIL 2 -CONST. JOINT 2'-10" RANSITION TOP (RAKE FINISH) / 2" CHAMFER TOP OF ROADWAY-16<u>1</u>" COPING WALL -SEE NOTE . (IAP 6'-6" NSITION Ϋ́́ -#5 @ 6" (TYP.) 2¹/₂" CL. CL. Ĺӡ" 22" NOTES: 1.  $1\frac{1}{2}$ " H x 1" D GROOVE. ALIGN WITH GROOVE AT TOP OF STRIATIONS. 2. REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY. SECTION 5 AT SAFETY CURB SCALE:  $\frac{1}{2}$ " = 1'-0" NOTES: 1. PRECAST GUARDRAIL TRANSITION SHALL BE 5000 PSI,  $\frac{3}{4}$  IN, 685 HP CEMENT CONCRETE. 2. GRAVEL BORROW SHALL BE PLACED AND THOROUGHLY COMPACTED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE PRECAST GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION. WHERE NO GRAVEL BORROW IS REQUIRED BELOW THE BASE, IT SHALL BE PLACED ON UNDISTURBED SOIL. 3. CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKILL PRECAST GUARDRAIL TRANSITION WITH CONTROLLED LOW-STRENGTH MATERIAL (> 300 PSI) TO THE ELEVATION SHOWN. 4. FOR SOUTHEAST HIGHWAY GUARDRAIL TRANSITION, AFTER CONTROLLED LOW-STRENGTH MATERIAL (> 300 PSI) HAS SET FILL THE GAPS BETWEEN GUARDRAIL TRANSITION AND BLOCK-OUT IN BACKWALL AND ABUTMENT WITH NON-SHRINK GROUT UP TO THE TOP OF BACKWALL. 5. THE REST OF REINFORCEMENT IS NOT SHOWN FOR CLARITY.

 SEPT. 14, 2024
 ISSUED FOR CONSTRUCTION

 DATE
 DESCRIPTION

 THIS SHEET IS APPROVED FOR
 Impute the second se

SHEET 14 OF 22 SHEETS BRIDGE NO. N-08-020 (CN3)

**NEW MARLBOROUGH** 

![](_page_30_Figure_0.jpeg)

#### STA _____

![](_page_30_Figure_2.jpeg)

### NOTES:

- 1. THIS BEARING IS DESIGNED
- 2. ELASTOMER SHALL HAVE A
- 3. STEEL LAMINATES SHALL CC EDGES OF STEEL LAMINATES
- 4. THE COMPRESSIVE DESIGN THE COMPRESSIVE DESIGN DESIGN LOAD BY THE AREA
- 5. THE 25 YEAR CREEP STRAI
- 6. TAPERED INTERNAL LOAD P OR GRADE 50. ALL EDGES GROUND SMOOTH.
- 7. ALL BEARINGS SHALL BE M INCLUDE THE BEARING LOCA ARROW THE POINTS UP-STA VISIBLE AFTER BEARING IS I
- 8. BEAMS SHALL BE ERECTED AND 90°F. IF BEAMS ARE WILL HAVE TO BE JACKED THE TEMPERATURE RETURNS

### ELASTON

![](_page_30_Figure_13.jpeg)

<u>INTERNAL</u>

	STATEFED. AID PROJ. NO.SHEET NO.TOTAL SHEETSMABFL(BR-OFF)-003S(798)X3142	
	PROJECT FILE NO. 609078 BRIDGE BEARING	
NUP		
OMFR LAYFR		
$- \frac{1}{4}$ (TYP.)		
NTERNAL LOAD PLATE		
STEEL LAMINATE		
DIAMETER		
ANG AASHIO MEIHOD B. EAR MODULUS OF 0.160 KSI.		
ORM TO ASTM A 1011 GRADE 36 OR HIGHER. ALL HALL BE GROUND.		
AD ON THE BEARING PAD IS 31 KIPS.		
THE PAD AND IS EQUAL TO 0.62 KSI.		
, HALL BE LIMITED TO 35%. E SHALL CONFORM TO AASHTO M 270 GRADE 36		
- TAPERED INTERNAL LOAD PLATE SHALL BE		
(ED PRIOR TO SHIPPING. THE MARKS SHALL IN ON THE BRIDGE, AND A $\frac{1}{32}$ " DEEP DIRECTION N. ALL MARKS SHALL BE PERMANENT AND BE TALLED.		
EN THE AMBIENT TEMPERATURE IS BETWEEN 30°F ECTED AT OTHER AMBIENT TEMPERATURES, THEY ) THE ELASTOMERIC BEARINGS RECENTERED WHEN O THAT RANGE.		
RIC BEARING PAD		
T TO SCALE		
NTERNAL LOAD PLATE		
σοκοΐ		
DIAMETER		
<u>OAD PLATE DETAIL</u>		
LE: 6" = 1'-0"		
SEPT. 14, 2024 DATF	ISSUED FOR CONSTRUCTION	)N
THIS SHEET IS CONSTRUCTIO	APPROVED FOR N BY MASSDOT	) ///
	J SIGNATURY: STATE' BRIDGE EN F ONLY PRINTS OF LATEST DATE	NGIN

![](_page_31_Figure_0.jpeg)

![](_page_32_Figure_0.jpeg)

### CONSTRUCTION SEQUENCE NOTES:

- 1. AFTER ALL BEAMS HAVE BEEN ERECTED, TENSION TRANSVERSE TIE TO 5 KIPS.
- 2. FILL ALL KEYWAYS WITH MORTAR (M4.04.5). IF THI KEYWAYS ARE NOT FILLED WITHIN FIVE (5) DAYS AF THE BEAMS ARE ERECTED, THE CONTRACTOR SHAL COVER AND PROTECT THE KEYWAYS FROM WEATHER DEBRIS UNTIL THEY ARE FILLED.
- 3. AFTER THE MORTAR HAS CURED (24 HOURS MINIM TENSION EACH TRANSVERSE TIE TO 44 KIPS.
- 4. CONCRETE FOR DECK SLAB SHALL BE PLACED AFTI THE TRANSVERSE TIES HAVE BEEN FULLY TENSIONE
- 5. NO TRAFFIC OR HEAVY EQUIPMENT WILL BE PERMIT ON THE BRIDGE UNTIL ALL TRANSVERSE TIES HAVE PROPERLY TENSIONED AND THE DECK HAS BEEN C AND CURED PER THE STANDARD SPECIFICATIONS.

![](_page_32_Figure_8.jpeg)

ACH -		NEW MARLBOROUGH         KEYES HILL ROAD OVER UMPACHENE RIVER         STATE       FED. AID PROJ. NO.       SHEET       TOTAL SHEETS         MA       BFL(BR-OFF)-003S(798)X       33       42
- TER		PROJECT FILE NO. 609078 PRESTRESSED CONCRETE BEAM 1
AND	NOTES:	
IM),	1. + DENOTES STRAIGHT STRANDS.	
R	2. SEE SHEAR KEY DETAIL ON THIS SH	IEET.
).	3. SEE END OF BEAM PLAN ON SHEET	18 OF 22 FOR STIRRUP SPACING.
ED BEEN	4. 1"ø drain, placed at both ends	OF EACH VOID.
ST	5. ALL PRETENSIONING ELEMENTS SHAL RELAXATION STEEL STRANDS AND SH	L BE 0.6"Ø, UNCOATED, SEVEN-WIRE, LOW IALL CONFORM TO AASHTO M 203.
	6. THE NOMINAL TENSILE STRENGTH OF KSI.	THE PRETENSIONING STRANDS SHALL BE 270
	7. THE INITIAL TENSION PER 0.6"Ø STR	AND SHALL BE 44 KIPS.
	8. THE MINIMUM 28 DAY COMPRESSIVE	STRENGTH OF CONCRETE SHALL BE 6,500 PSI
	9. NO PRESTRESS SHALL BE TRANSFER A COMPRESSIVE STRENGTH, AS SHO' PSI.	RED TO THE CONCRETE UNIT IT HAS ATTAINED WN BY CYLINDER TEST, OF AT LEAST 4,500
	10. THE TOP OF ALL BEAMS SHALL BE THE WIDTH PERPENDICULAR TO THE	GIVEN A RAKE FINISH ( $\frac{1}{4}$ " AMPLITUDE) ACROSS BEAM'S AXIS.
	11. THE FABRICATOR IS FULLY RESPONS AND BEAM STRESSES DURING LIFTIN FOR THE SAFETY FACTORS REQUIRED	BIBLE FOR THE DESIGN OF THE LIFTING DEVICES G AND HANDLING WHICH SHALL BE ADEQUATE D BY THE ERECTION PROCEDURE.
	12. TO CONTROL CRACKING AT THE END DEBOND APPROXIMATELY 50% OF TH OF THE BEAM.	OF THE BEAM, THE FABRICATOR SHALL IE STRANDS FOR THE FIRST 6" FROM THE END
		TOP OF BEAM
FOR		(RAKE FINISH)
		MORTAR (M4.04.5)
1 01	3" Ø SLEEVE	0.6" TRANSVERSE
01, 0		CLOSED CELL NON-ABSORBENT
		FOAM JOINT FILLER
L BENT FILLER		<u>3</u> "
AT	SI	ECTION 8
	SCALE	$: 1^{1}_{2}$ = 1'-0"
ITH 2 <mark>1</mark> " ø TAM	NOTES:	
	1. MORTAR FOR EXTERIOR P SHALL BE THE SAME COI	OCKETS SHALL CONFORM TO M4.02.15 AND OR AND TEXTURE AS THE BEAM CONCRETE.
ANSVERSE (SEE NOT	TE3)2.OTHER ANCHORAGE SYSTE	EMS MAY BE SUBSTITUTED WITH THE
Ø SLEEVE	APPROVAL OF THE ENGIN SHALL BE WATERTIGHT AN	EER. ALTERNATE ANCHORAGE SYSTEMS ID CORROSION PROOF.
	3. TRANSVERSE TIES SHALL	BE COVERED BY A SEAMLESS
WITH WAT AT ANCHOF	ERTIGHT POLYPROPYLENE SHEATH BETWEEN THE STRAND AN STRAND, EXCEPT AT THE	(WITH CORROSION INHIBITING GREASE ID SHEATH) FOR THE FULL LENGTH OF THE ANCHORAGE LOCATION.
	SEPT. 14,	2024 ISSUED FOR CONSTRUCTION DESCRIPTION
-	THIS SHE CONSTRU	ET IS APPROVED FOR JUNCTION BY MASSDOT
	AUTHC	DRIZED SIGNATORY: STATE BRIDGE ENGINEER USE ONLY PRINTS OF LATEST DATE

![](_page_33_Figure_0.jpeg)

![](_page_33_Figure_1.jpeg)

• \$\$" (SKENED)           • \$\$" (SKENED)           • \$\$" (SKENED)             • \$\$" (SKENED)           • \$\$" (SKENED)           • \$\$" (SKENED)             • \$\$           • \$\$           • \$\$           • \$\$           • \$\$         • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$           • \$\$                   • \$           • \$                   • \$             • \$           • \$           •			KEYES HIL	L ROAD O			
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHESS           STATE OF BEAM         DIRECTION BEAMS #1 & #7           BEAM         DIRECTION BEAMS #1 & #7           BEAM         DIRECTION BEAMS #1 & #7           STATE OF BEAM         DIRECTION BEAMS #1 & #7           CAMEER AT DRECTION BEAMS #1 & #7         BEAMS #2 TO #4           CAMEER AT DRECTION 40         D.70         0.58           CAMEER AT DRECTION 40         D.57         0.41           THAL INCL DEFLECTIONS 15         DIRECTION 40         0.69         0.52           NOTESI         1         APROVED TO SUMMUNAL DURINGER DATE         0.52         0.41           THAL INCL DEFLECTION 45         DOWN         0.69         0.52         0.52           NOTESI         1         CAMEER AT DEFLECTION 45         DOWN         0.69         0.52           NOTESI         1         CAMEER AT DEFLECTION 45         DOWN         0.69         0.52         NT           1         THE ARD DEFLECTION 5         N THE ARS NOT COMMUNED AND ARS DEFT         0.52         NT           1         DEFLECTION S ASSUMET TO DEFLECTION 5         DIRECTION 14         THE ARS DEFLECTION S ASSUMED TO DEFLECTION S ASSUMET TO DEFLECTION S ASSUMET TO THE ASSUMET AND ARS DEFT         DIRECTION 14         DIRECTION 14         DIRECTION 14 <td< th=""><th>9³" (SKEWED)</th><th></th><th>STATE MA</th><th>FED. AID P</th><th>ROJ. NO. 003S(798)X</th><th>NO. S</th><th>SHEETS 42</th></td<>	9 ³ " (SKEWED)		STATE MA	FED. AID P	ROJ. NO. 003S(798)X	NO. S	SHEETS 42
PRESTRESSED CONCRETE BEAM 2         Image: State of Beam State of State of Beam Direction Beams #1 & #7 Beams #2 TO #4         State of Beam Direction Beams #1 & #7 Beams #2 TO #4         CANGER AI TRANSTER *29 UP 0.70 C.35         CANGER AI TRANSTER *20 UP 0.70 C.35         CANGER AND DEFLECTIONS IN THE "ARE ARE NOT SUBBANIED AND ARE PLACEDON 0.69 C.62         NOTES:         • CANGER AND DEFLECTIONS IN THE TRANSTER USED N         • DEFN OND FOR INFORMATIONAL PLACESS DONLY.         • THE DEAM CONSETE MCDULUS OF ELESTICH VISIO IN THE AREAD BEAM DEFLECTION 'S ASSUMED TO BE 5.008 PS (AT 28 DAYS).         • THE BEAM CONSETE MCDULUS OF ELESTICH VISIO IN THE AREAD BEAM DEFLECTION 'S ASSUMED TO BE 5.008 PS (AT 28 DAYS).         • THE BEAM CONSETE MCDULUS OF ELESTICH VISIO IN THE AREAD BEAM DEFLECTION 'S ASSUMED TO BE 5.008 PS (AT 28 DAYS).         • DISCRIPTION         • DISCRIPTION PARES DE TANKED TANKED				PROJECT FI	LE NO.	609078	
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHESS         STATE OF BEAM       DIRECTION         BEAMS #1 4& #7       BEAMS #2 TO #3         CAMBER AT TRANSFER ⁽²⁾ UP       0.73       0.59         CAMBER AT TRANSFER ⁽²⁾ UP       0.73       0.69         CAMBER AT TRANSFER ⁽²⁾ UP       0.73       0.69         CAMBER AT TRANSFER ⁽²⁾ UP       0.73       0.69         FINAL COLL DEFLECTIONS IN THE TAJE AXE NOT CUARANTED AND AND AND AND AND AND AND AND AND AN	~~		PRES	TRESSED	CONCRE	TE BE	AM 2
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHESS STATE OF BEAM DIRECTION BEAMS #1 & #7 BEAMS #2 TO #4 CAMBER AT TRANSFER (2) JF 0.70 0.55 CAMBER AT TRANSFER (2) JF 0.57 0.41 FINAL CEL DEFLECTION (2) DOWN 0.59 0.52 INAL CEL DEFLECTION (2) DOWN 0.59 0.52 NOTES:         1. CAMBER AND DEF COLONS IN THE TABLE AFF NOT GUARANTEED AND ARE FROVEDE FOR INFORMATINE JURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM DEFLECTION S ASSUMED TO BE 5,000 PS (AT 26 DAVS).         IDP.)       STET. 14.202/ IESUED FOR CONSTRUCTION THE ABOVE BEAM CONCRETE MODULUS OF FLASTOR VIEST IN THE ABOVE BEAM DEFLECTION S ASSUMED TO BE 5,000 PS (AT 26 DAVS).         IDP.)       STET. 14.202/ IESUED FOR CONSTRUCTION THE SECON STATUS OF MARKED FOR CONSTRUCTION IN THE ABOVE DESCENTION STATUS OF MARKED AND ADDUCE SECAN DEFLECTION S ASSUMED TO BE 5,000 PS (AT 26 DAVS).							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES STATE OF BEAM     DIRECTION     BEAMS #1 & #7     BEAMS #2 TO #4 CAMBER AT FRANSER 20       CAMBER AT FRANSER 21     UP     0.70     0.58       INAL C3L DEFLECTION 15     DOWN     0.89     0.52       INAL C3L DEFLECTION 16     100 FUNCTION 100 AND ALL PURPOSES ONLY.     0.99       INDES:     1     CAMER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ALL PURPOSES ONLY.       2. THE BEAM CONCRTST NOTULING OF ELASTICITY AT TRANSFER USED IN THE ABOVE DEAM CAMERY IS ASSUMED TO BE 5,003 FISI (AT 28 DAYS).       STHE IS AND DEFLECTION IS ASSUMED TO BE 5,003 FISI (AT 28 DAYS).       STHE IS AND DEFLECTION IS ASSUMED TO BE 5,003 FISI (AT 28 DAYS).							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4 CAMBER AT TRANSFER (?)         UP       0.70       0.58         CAMBER AT TRANSFER (?)       UP       0.77       0.41         FINAL COLL DEFLECTION (?)       DOWN       0.96       0.98         ENAL COLL DEFLECTION (?)       DOWN       0.69       0.52         NOTES:       1.       CAMBER AND DEFLECTION (?)       DOWN       0.69       0.52         NOTES:       1.       CAMBER NODELECTION (?)       DOWN       0.69       0.52         NOTES:       1.       CAMBER NODELECTION (?)       DOWN       0.69       0.52         NOTES:       1.       CAMBER NODELECTION (?)       DOWN       0.69       0.52         NOTES:       1.       THE PRAY CONCRETE MODULUS OF FLASHICHTY AT TRANSFER USED IN THE ABOVE BOAM CAMBER IS ASSUMED TO BE 4.455 FSI.       1.       THE PRAY CONCRETE MODULUS OF FLASHICHTY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5.005 FSI (AT 28 DAYS).         (TVP.)       INFERTION IS ASSUMED TO BE 5.005 FSI (AT 28 DAYS).       DESCRIPTION ONE DAY THE DAY OF DASSIMED TO AUTORZED SIGNAROVER TRANSFER TO DASSIME DAYS							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES STATE OF BEAM     DIRECTION     BEAMS #1 & #7     BEAMS #2 TO #4       CAMBER AT TRANSFER (2)     JP     0.70     0.58       CAMBER AT ERECTION     JP     0.57     0.41       FINAL COL DEFLECTION (3)     JOWN     0.96     0.99       FINAL COL DEFLECTION (4)     JOWN     0.96     0.99       FINAL COL DEFLECTION (4)     JOWN     0.96     0.52       NOTES:     1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT CLARANTEED AND ARE PROVED FOR INFORMATIONAL PURPOSES ONLY.     1. HE BEAM CONCRETE MODILUS OF ELASTICITY AT TRANSFER USED IN THE AREVE MODILUS OF ELASTICITY AT TRANSFER USED IN THE AREVE POLY DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED IN THE AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND ARE FER USED IN THE AREVE ADDITION OF ELASTICITY AT TRANSFER USED IN THE AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND AREVE DEFLECTION IS ASSUMED TO BE 4.4430 FRUICED AND AREVE DEFLECTION IS ASSUMED TO BE 5.008 FPS (AT 28 DAYS).							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES STATE OF BEAM     DIRECTION BEAMS #1 & #7     BEAMS #2 TO #0       CAMBER A TRANSFER ³⁰ UP     0.70     0.38       CAMBER A TRANSFER ³⁰ UP     0.70     0.38       CAMBER A TRANSFER ³⁰ UP     0.70     0.38       CAMBER AT FRACTION     UP     0.57     0.41       THAL COL DEFLECTION ⁴ DOWN     0.96     0.39       THAL COL DEFLECTION ⁴ DOWN     0.69     0.52       NOTES:       1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT SURPANIEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES DNLY.       2. THE BEAM CONCRETE MODULUS OF PLASTICITY AT TRANSFER USED IN THE ASSUMED TO BE 1.4350 FML     THE ABOVE BOOK       3. THE BEAM CONCRETE MODULUS OF PLASTICITY AT TRANSFER USED IN THE SEME THE ASSUMED TO BE 3.000 PSI (AT 28 DAYS).       (IVP.)     IMPLICATION IS ASSUMED TO BE 3.000 PSI (AT 28 DAYS).							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES STATE OF BEAM DIRECTION BEAMS #1 & #7 BEAMS #2 TO #6 CAMBER AT TRANSFER ⁽²⁾ UP 0.70 0.58 CAMBER AT ERROTION UP 0.57 0.41 FINAL NOLL DEFLECTION ⁽³⁾ DOWN 0.98 0.99 FINAL COL DEFLECTION ⁽³⁾ DOWN 0.98 0.99 FINAL COL DEFLECTION ⁽³⁾ DOWN 0.69 0.52         NOTES:       1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES OVLY.         NOTES:       1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES OVLY.         1. THE BEAM CONCRETE MODULUS OF FLASTICITY WED IN THE ABOVE BEAM DEFLECTION IS ASSUVED TO BE 5,008 PSI (AT 28 GAYS).         STE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUVED TO BE 5,008 PSI (AT 28 GAYS).         INF.)       Image: Inflammation of the state of the store of the							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT TRANSFER ⁽²⁾ UP       0.73       0.53         CAMBER AT ERECTION       UP       0.57       0.41         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.69       0.52         NOTES:         1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT SUARANTEED AND ARE PROVIDED FOR INDRIVATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         3. THE REAV CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         (NP.)       SEPT 14, 2024       IESUED FOR CONSTRUCTION DEFORMED FOR CONSTRUCTION DEFORMED FOR CONSTRUCTION DEFENSION SIN AND AND AND AND AND AND AND AND AND AN							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHESS         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         TINAL NOLL DEFLECTION ⁽³⁾ DOWN       0.96       0.39         PINAL CDL DEFLECTION ⁽³⁾ DOWN       0.69       0.32         NOTES:       NOTES:       0.00000000000000000000000000000000000							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHESS         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMEER AT TRANSFER (2)       UP       0.70       0.58         CAMEER AT TRANSFER (2)       UP       0.57       0.41         FINAL COL DEFLECTION (3)       DOWN       0.96       0.99         FINAL COL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT TRANSFER ⁽²⁾ UP       0.57       0.41         TINAL NOL DEFLECTION UP       0.57       0.41         TINAL NOL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         PINAL CDL DEFLECTION ⁽³⁾ DOWN       0.69       0.52         NOTES:         *       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES CIN.Y.         2. THE BRAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION S ASSUMED TO BE 4.435 PSI.         3. THE BRAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION S ASSUMED TO BE 5.008 PSI (AT 28 DAYS).         (TYP.)       SEPT. '4, 2024 ISSUED FOR CONSTRUCTION THE SECONFILTION THE SECONFILTION S ASSUMED TO BE 5.008 PSI (AT 28 DAYS).							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT ERANSER (?)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL COL DEFLECTION (3)       DOWN       0.96       0.93         FINAL COL DEFLECTION (3)       DOWN       0.59       0.52         NOTES:       1       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ON X.         2. THE REAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 4.435 FSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5.008 PSI (AT 28 DAYS).         (TYP.)       ISSUED FOR CONSTRUCTION							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES         STATE OF BEAM       DIRECTION BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT TRANSFER ⁽²⁾ UP       0.57       0.41         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.69       0.52         NOTES:         1       CAMEER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,000 PSI (AT 25 DAYS).         (TYP.)       INTE       DESCRIPTION         SEPTIMENTIAL TO BE 5,000 PSI (AT 25 DAYS).							
SETIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.55         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.52         NOTES:         1. CAMPER AND DEFLECTIONS IN THE TABLE ARE NOT COLARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         IN BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         SEMEM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEMEM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEMEM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEMEM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEMEM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS). <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL NOL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         STATE IS APPRIVED FOR CONSTRUCTION DESCRIPTION         THIS SHEET IS APPRIVED FOR CONSTRUCTION DESCRIPTION         THIS SHEET IS APPRIVED FOR CONSTRUCTION THE ABOVE DESCRIPTION         THIS SHEET IS APPRIVED FOR CONSTRUCTION THE ABOVE DESCRIPTION         THIS SHEET IS APPRIVED FOR CONSTRUCTION THE ABOVE DESCRIPTION         THIS SHEET IS APPRIVED FOR CONSTRUCTION THE ABOVE DESCRIPTION         DATE TO APPRIVED FOR CONSTRUCTION THE ABOVE DESCRIPTION         DATE SHEET IS APPRIVED FOR CONSTRUCTION THE ABOVE DESCRI							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT ERECTION       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.69       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SUPILIES APPROVED FOR CONSTRUCTION DESCRIPTION         DATE         SUPILIES APPROVED FOR CONSTRUCTION DESCRIPTION         DESCRIPTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SUPILIES APPROVED FOR CONSTRUCTION         DATE         DESCRIPTION         SUPILIES APPROVED FOR CONSTRUCTION         DATE         DATE         CONSTRUCTION PM MASSOOT         DA							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL CDL DEFLECTION (3)       DGWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DGWN       0.689       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         STATE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         (TYP.)							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT TRANSFER ⁽²⁾ UP       0.77       0.41         FINAL NOL DEFLECTION       UP       0.57       0.41         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT CUARANTEED AND ARE PROVIDE FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CANBER IS ASSUMED TO BE 4.435 PSI.         3.         THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         TYP.)							
SETIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL CDL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:         1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024       ISSUED FOR CONSTRUCTION         DATE       DESCRIPTION         THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAY							
SETIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.411         FINAL Col DEFLECTION (3)       DOWN       0.96       0.999         FINAL Col DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         (TYP.)         SEPT. 14, 2024       ISSUED FOR CONSTRUCTION         DATE       DESCRIPTION         ISSUED FOR CONSTRUCTION       DATE         DATE       DESCRIPTION         DATE							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #0         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL NOLL DEFLECTION (3)       DOWN       0.96       0.99         FINAL COL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.       3.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE DESCRIPTION         THE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         (TYP.)							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES)         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #6         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.96       0.99         FINAL COL DEFLECTION ⁽³⁾ DOWN       0.69       0.52         NOTES:       .       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.       .         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).       (TYP.)         SEPT. 14, 2024       ISSUED FOR CONSTRUCTION THIS SHEET IS APPROVED FOR CONSTRUCTION DATE         DATE       DESCRIPTION       THIS SHEET IS APPROVED FOR CONSTRUCTION AUTHORIZED SIGNATORY: STATE & RIDGE ENGIN							
ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (INCHES         STATE OF BEAM       DIRECTION       BEAMS #1 & #7       BEAMS #2 TO #4         CAMBER AT TRANSFER ⁽²⁾ UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL NOLL DEFLECTION ⁽³⁾ DOWN       0.96       0.93         FINAL CDL DEFLECTION ⁽³⁾ DOWN       0.699       0.52         NOTES:       1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT CUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024         ISSUED FOR CONSTRUCTION         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         CONSTRUCTION BY MASSDOT         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         CONSTRUCTION BY MASSDOT         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         CONSTRUCTION BY MASSDOT         USE ONLY PRINTS OF LATEST DATE </th <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>							
STATE OF BEAM       DIRECTION       BEAMS       #1       & #7       BEAMS       #2       TO       #6         CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL CDL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:       1       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.       2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024         ISSUED FOR CONSTRUCTION BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).							
CAMBER AT TRANSFER (2)       UP       0.70       0.58         CAMBER AT ERECTION       UP       0.57       0.41         FINAL NCDL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE         DESCRIPTION         THIS APPROVED FOR CONSTRUCTION DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION DATE         USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE	ER AND DE	FLECTION	S AT M	IDSPA	.N (I	NCH
CAMBER AT ERECTION       UP       0.57       0.41         FINAL NCDL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION (3)       DOWN       0.69       0.52         NOTES:         1.       CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2.       THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3.       THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         ONSTRUCTION BY MASSDOT         AUTHORIZED SIGNATORY:         STATE BRIDGE ENGIN         USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE	ER AND DE	FLECTIONS BEAMS #1	S AT M & #7	IDSPA BEAM	.N (I S #2	NCH TO
FINAL NCDL DEFLECTION (3)       DOWN       0.96       0.99         FINAL CDL DEFLECTION       (3)       DOWN       0.69       0.52         NOTES:         1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION DATE         DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾	ER AND DE DIRECTION	FLECTIONS BEAMS #1 0.70	S AT M & #7	IDSPA BEAM	N (I S #2 0.5	NCH TO 8
FINAL CDL DEFLECTION       (3)       DOWN       0.69       0.52         NOTES:         1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         (TYP.)         SEPT. 14, 2024         ISSUED FOR CONSTRUCTION         DATE         OF INT S OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION	ER AND DE DIRECTION UP UP	<b>FLECTION</b> BEAMS <b>#</b> 1 0.70 0.57	S AT M & #7	IDSPA BEAM	N (I S #2 0.5 0.4	NCH TO 8
YP.)         NOTES:         1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         YP.)         SEPT. 14, 2024       ISSUED FOR CONSTRUCTION DATE         DATE       DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION DATE       CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:         STATE BRIDGE ENGIN       USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽⁴⁾	ER AND DE DIRECTION UP UP 3) DOWN	EFLECTIONS BEAMS #1 0.70 0.57 0.96	S AT M & #7	IDSPA BEAM	N (I S #2 0.5 0.4 0.9	NCH TO 8 1 9
1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY.         2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         STPP.)         SEPT. 14, 2024       ISSUED FOR CONSTRUCTION DATE         DATE       DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION THIS SHEET IS APPROVED FOR CONSTRUCTION DATE         DATE       DESCRIPTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         MUTHORIZED SIGNATORY:         STATE BRIDGE ENGIN	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽⁴⁾	ERANDDEDIRECTIONUPUP3)DOWN3)DOWN	EFLECTIONS BEAMS #1 0.70 0.57 0.96 0.69	S AT M & #7	IDSPA BEAM	N (I S #2 0.5 0.4 0.9 0.5	NCH TO 8 1 9 2
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ITHE ABOVE BEAM CAMBER IS ASSUMED TO BE 4,435 PSI.         3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,008 PSI (AT 28 DAYS).         SEPT. 14, 2024         ISSUED FOR CONSTRUCTION         DATE         DATE         THIS SHEET IS APPROVED FOR CONSTRUCTION         THIS SHEET IS APPROVED FOR CONSTRUCTION         AUTHORIZED SIGNATORY:         STATE BRIDGE ENGIN	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽⁴⁾ FINAL CDL DEFLECTION ⁽⁴⁾ NOTES: 1. CAMBER AND ARE PROVIDED	ER       AND       DE         DIRECTION       UP         UP       UP         3)       DOWN         3)       DOWN         DEFLECTIONS       IN         DEFLECTIONS       IN	FLECTIONS BEAMS #1 0.70 0.57 0.96 0.69	SAT M & #7	IDSPA BEAM	N (I S #2 0.5 0.4 0.9 0.5 ED AN	NCH TO 8 1 9 2
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(TYP.) SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGIN USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽⁴⁾ FINAL CDL DEFLECTION ⁽⁴⁾ NOTES: 1. CAMBER AND ARE PROVIDED 2. THE BEAM CO THE ABOVE B 3. THE BEAM CO BEAM DEFLECT	ER AND DE         DIRECTION         UP         UP         3)         DOWN         3)         DOWN         DEFLECTIONS IN         DEFLECTIONS IN         DOWN         DEFLECTIONS IN         DOWN         DOWN         DOWN         DOWN         DOWN         DEFLECTIONS IN         DOWN	FLECTIONS BEAMS #1 0.70 0.57 0.96 0.69 I THE TABLE A 10NAL PURPOS US OF ELASTIC ASSUMED TO US OF ELASTIC ASSUMED TO US OF ELASTIC	SAT M & #7	IDSPA BEAM UARANTE ANSFER PSI. IN THE 28 DAY	N (1 S #2 0.5 0.4 0.9 0.5 ED AN USED ABOVE S).	NCH TO 8 1 9 2 D IN
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(TYP.) SEPT. 14, 2024 ISSUED FOR CONSTRUCTION DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGIN USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽⁴⁾ FINAL CDL DEFLECTION ⁽⁴⁾ NOTES: 1. CAMBER AND ARE PROVIDED 2. THE BEAM CO THE ABOVE B 3. THE BEAM CO BEAM DEFLECT	ER AND DE         DIRECTION         UP         UP         3)         DOWN         3)         DOWN         DEFLECTIONS IN         DOWN         DEFLECTIONS IN         DOWN         DOWN         DOWN         DOWN         DOWN         DEFLECTIONS IN         DNCRETE MODULU         DNCRETE MODULU         DNCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU	FLECTIONS BEAMS #1 0.70 0.57 0.96 0.69 I THE TABLE A TIONAL PURPOS US OF ELASTIC ASSUMED TO US OF ELASTIC ASSUMED TO US OF ELASTIC	SAT M & #7 , , , , , , , , , , , , , , , , , , ,	IDSPA BEAM UARANTE ANSFER PSI. IN THE 28 DAY	N (1 S #2 0.5 0.4 0.9 0.5 ED AN USED ABOVE S).	NCH TO 8 1 9 2 D IN
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DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGIN USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽²⁾ FINAL CDL DEFLECTION ⁽²⁾ NOTES: 1. CAMBER AND ARE PROVIDED 2. THE BEAM CO THE ABOVE B 3. THE BEAM CO BEAM DEFLECT	ER AND DE         DIRECTION         UP         UP         3)         DOWN         DEFLECTIONS IN         DOWN         DEFLECTIONS IN         DOWN         DOWN         DOWN         DOWN         DOWN         DEFLECTIONS IN         DOWN         DNCRETE MODULU         DNCRETE MODULU         DNCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU         DINCRETE MODULU	FLECTIONS BEAMS #1 0.70 0.57 0.96 0.69 I THE TABLE A TIONAL PURPOS US OF ELASTIO ASSUMED TO US OF ELASTIO D TO BE 5,00	SAT M & #7	IDSPA BEAM UARANTE ANSFER PSI. IN THE 28 DAY	N (1 S #2 0.5 0.4 0.9 0.5 ED AN USED ABOVE S).	NCH TO 8 1 9 2
CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGIN USE ONLY PRINTS OF LATEST DATE	ESTIMATED CAMBE STATE OF BEAM CAMBER AT TRANSFER ⁽²⁾ CAMBER AT ERECTION FINAL NCDL DEFLECTION ⁽²⁾ FINAL CDL DEFLECTION ⁽²⁾ NOTES: 1. CAMBER AND ARE PROVIDED 2. THE BEAM CC THE ABOVE B 3. THE BEAM CC BEAM DEFLECT	ER AND DE         DIRECTION         UP         UP         3)         DOWN         3)         DOWN         DEFLECTIONS IN         DEFLECTIONS IN         DNCRETE MODULU         EAM CAMBER IS         ONCRETE MODULU         ION IS ASSUME	FLECTIONS BEAMS #1 0.70 0.57 0.96 0.69 I THE TABLE A TIONAL PURPOS US OF ELASTIO ASSUMED TO US OF ELASTIO D TO BE 5,00	SAT M & #7 & #7 ARE NOT G SES ONLY. CITY AT TR BE 4,435 CITY USED 08 PSI (AT ISSUEI	D FOR C	N (1 S #2 0.5 0.4 0.9 0.5 ED AN USED ABOVE S).	NCH TO 8 1 9 2 D IN
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![](_page_34_Figure_0.jpeg)

SECTIONS WITH NOT LESS THAN 3 DAYS BETWEEN POURS. 2. DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS. END THE REINFORCEMENT 2" CLEAR OF JOINT.

![](_page_34_Picture_5.jpeg)

N LINE	RIGHT EDGE OF DECK SLAB
	8"
	5"
	8"

	THEORETICAL TOP OF DECK ELEVATIONS ALONG THE SPAN LENGTH										
	€ BRG.	0.1L	0.2L	0.3L	0.4L	0.5L	0.6L	0.7L	0.8L	0.9L	€ BRG.
LEFT CURBLINE	991.21	991.34	991.48	991.63	991.80	991.97	992.16	992.36	992.57	992.79	993.03
CROWN	991.33	991.44	991.57	991.71	991.87	992.03	992.21	992.40	992.60	992.81	993.04
RIGHT CURBLINE	991.09	991.20	991.32	991.45	991.59	991.75	991.91	992.09	992.28	992.48	992.70

NOTES:

1. REFERENCE FOR RIGHT CURBLINE, CROWN, AND LEFT CURBLINE LOOKING UP STATION.

2. THE THEORETICAL TOP OF DECK ELEVATIONS ARE PROVIDED FOR SETTING THE SCREED ELEVATIONS FOR DECK FINISHING. THESE VALUES ARE CALCULATED BY TAKING THE PROPOSED TOP OF HMA ELEVATION ALONG THE SPAN AT THE POINTS INDICATED IN THE TABLE, SUBTRACTING THE THICKNESS OF THE HMA AND ADDING THE DEFLECTIONS DUE TO THE DECK AND OTHER SUPERIMPOSED DEAD LOADS.

SUPERPAVE BRIDGE COURSES MEMBRANE WATERPROOFING -

![](_page_34_Figure_16.jpeg)

### NOTES:

- 1. LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE  $\mathbb{E}$  OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE  $\beta$  OF CONSTRUCTION.
- 2. ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE COATED.
- 3. THE FINISHED SURFACE OF THE BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.

![](_page_34_Figure_21.jpeg)

- 2. SAFETY CURB CONCRETE SHALL BE 5000 PSI,  $\frac{3}{4}$  IN, 685 HP CEMENT CONCRETE.

SAFETY CL

SCALE:

1. DECK SLAB SHALL BE 5000 PSI,  $\frac{3}{4}$  IN, 685 HP CEMENT CONCRETE.

BE 5000 PSI, ¥ IN, 685 HP CEMENI	SEPT. 14, 2024	ISSUED FOR CONSTRUCTION
	DATE	
IRB SECTION	THIS SHEET IS CONSTRUCTION AUTHORIZED	APPROVED FOR BY MASSDOT SIGNATORY: STATE BRIDGE ENGINEER
1" = 1' - 0"	USE	ONLY PRINTS OF LATEST DATE
SHEET 19 OF 22 SH	EETS BRI	DGE NO. N-08-020 (CN3)

### NEW MARLBOROUGH **KEYES HILL ROAD OVER UMPACHENE RIVER**

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	BFL(BR-OFF)-003S(798)X	35	42
	PROJECT FILE NO.	609078	

**BRIDGE TRANSVERSE SECTION** 

![](_page_35_Figure_0.jpeg)

![](_page_36_Figure_0.jpeg)

![](_page_37_Figure_0.jpeg)

![](_page_38_Figure_0.jpeg)

![](_page_39_Figure_0.jpeg)

CROSS SECTION.DWG Plotted on 29-June-2022 5:00

![](_page_40_Figure_0.jpeg)

				1000ך
T. TEMPO	RARY EASEME	NT		996
				992
				988
				984
				980
				976
				972
36	40	44	48	968

![](_page_40_Figure_2.jpeg)

**CROSS SECTION 3** 

CUT: 19.35 SF FILL: 12.07 SF

![](_page_40_Figure_5.jpeg)

HOR. SCALE IN FEET 0 4 VER. SCALE IN FEET

![](_page_41_Figure_0.jpeg)

![](_page_41_Figure_2.jpeg)

![](_page_41_Figure_3.jpeg)

![](_page_41_Figure_5.jpeg)

### CUT: 11.70 SF FILL: 4.60 SF

HOR. SCALE IN FEET 0 4 4 VER. SCALE IN FEET

![](_page_42_Figure_0.jpeg)

M	
E1. 97,5	
48 Elm WailtRod ClothEl. 10	0,00 Wire Tence Gravel Roadway Wire Fence 4"Elm yeamore Wire Sence
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	<i>FEI. 9/4.08</i>
Bed Of MIVER El. 184	FOR CONSTRUCTION DATE 11/10/38
CROSS SECTI SCALE 1"-4" OTES HIS MOUTHLY. MES OF QUANTITIES	ON BRIDGE NO. 1 ON TOPO SHEET 22 THE COMMONWEALTH OF MASSACHUSETTS PROPOSED BRIDGE NEW MARLBORD KEYES BRIDGE KEYES HULL BOAD
70 cu, yds, 80 cu, yds, 33,200 165. 70 cu, yds, 33,200 165. 7000 165. 90 110, ft, 27000 165. 30 cu, yds, 30 cu, yds, 25 cu yds,	OVER UMPACHENE RIVER SCALES AS NOTED DEPARTMENT OF PUBLIC WORKS 100 NASHUA ST. BOSTON, MASS, OCT. 12 1936 BRIDGE ENGINEER CLEVERDON, VARNEY & PIKE, ENGINEERS, BOSTON
1DGE	BRIDGE Nº N-8-20